

Detailed Member Calculations

Units: N&mm

Regulation: ASCE 41-17

Calculation No. 1

column C1, Floor 1

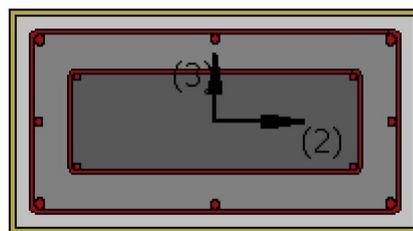
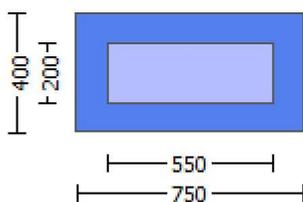
Limit State: Immediate Occupancy (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Shear capacity V_{Rd}

Edge: Start

Local Axis: (2)



Start Of Calculation of Shear Capacity for element: column JC1 of floor 1

At local axis: 2

Integration Section: (a)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$

Member Shear Force is generally considered as Force-Controlled Action according to Table C7-1, ASCE 41-17.

Lower-bound strengths are used for Force-Controlled Actions according to 7.5.1.3, ASCE 41-17

Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Primary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$
 Steel Elasticity, $E_s = 200000.00$
 Existing Column
 New material of Primary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$
 New material of Primary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$
 Concrete Elasticity, $E_c = 26999.444$
 Steel Elasticity, $E_s = 200000.00$
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 Note: Especially for the calculation of γ for displacement ductility demand,
 the expected (mean value) strengths are used (7.5.1.3, ASCE 41-17) because bending is considered as
 Deformation-Controlled Action (Table C7-1, ASCE 41-17).
 Jacket
 New material: Concrete Strength, $f_c = f_{cm} = 33.00$
 New material: Steel Strength, $f_s = f_{sm} = 555.56$
 Existing Column
 New material: Concrete Strength, $f_c = f_{cm} = 33.00$
 New material: Steel Strength, $f_s = f_{sm} = 555.56$
 #####
 External Height, $H = 400.00$
 External Width, $W = 750.00$
 Internal Height, $H = 200.00$
 Internal Width, $W = 550.00$
 Cover Thickness, $c = 25.00$
 Element Length, $L = 3000.00$
 Primary Member
 Ribbed Bars
 Ductile Steel
 Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
 Longitudinal Bars With Ends Lapped Starting at the End Sections
 Inadequate Lap Length with $l_o/l_{o,min} = l_b/l_d = 0.30$
 FRP Wrapping Data
 Type: Carbon
 Dry properties (design values)
 Thickness, $t = 1.00$
 Tensile Strength, $f_{fu} = 840.00$
 Tensile Modulus, $E_f = 82000.00$
 Elongation, $e_{fu} = 0.009$
 Number of directions, $NoDir = 1$
 Fiber orientations, $bi: 0.00^\circ$
 Number of layers, $NL = 1$
 Radius of rounding corners, $R = 40.00$

 Stepwise Properties

EDGE -A-
 Bending Moment, $M_a = -7.0400E+006$
 Shear Force, $V_a = -1995.365$
 EDGE -B-
 Bending Moment, $M_b = 1.0488E+006$
 Shear Force, $V_b = 1995.365$
 BOTH EDGES
 Axial Force, $F = -14177.47$
 Longitudinal Reinforcement Area Distribution (in 2 divisions)
 -Tension: $As_t = 0.00$
 -Compression: $As_c = 2676.637$
 Longitudinal Reinforcement Area Distribution (in 3 divisions)
 -Tension: $As_{t,ten} = 1137.257$
 -Compression: $As_{l,com} = 1137.257$
 -Middle: $As_{l,mid} = 402.1239$
 Mean Diameter of Tension Reinforcement, $Db_{L,ten} = 16.80$

New component: From table 7-7, ASCE 41_17: Final Shear Capacity $V_R = 1.0 * V_n = 948575.705$
 $V_n ((10.3), ASCE 41-17) = knl * V_{CoIO} = 948575.705$

VCol = 948575.705

knl = 1.00

displacement_ductility_demand = 0.01372547

NOTE: In expression (10-3) ' $V_s = A_v \cdot f_y \cdot d / s$ ' is replaced by ' $V_s + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: $f_c' = (f_c'_{\text{jacket}} \cdot \text{Area}_{\text{jacket}} + f_c'_{\text{core}} \cdot \text{Area}_{\text{core}}) / \text{Area}_{\text{section}} = 25.00$, but $f_c'^{0.5} \leq 8.3$
MPa (22.5.3.1, ACI 318-14)

M/Vd = 4.00

$\mu_u = 7.0400E+006$

$V_u = 1995.365$

$d = 0.8 \cdot h = 600.00$

$N_u = 14177.47$

$A_g = 300000.00$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 559706.147$

where:

$V_{s1} = 471238.898$ is calculated for jacket, with:

$d = 600.00$

$A_v = 157079.633$

$f_y = 500.00$

$s = 100.00$

V_{s1} is multiplied by $\text{Col1} = 1.00$

$s/d = 0.16666667$

$V_{s2} = 88467.249$ is calculated for core, with:

$d = 440.00$

$A_v = 100530.965$

$f_y = 500.00$

$s = 250.00$

V_{s2} is multiplied by $\text{Col2} = 1.00$

$s/d = 0.56818182$

V_f ((11-3)-(11.4), ACI 440) = 463792.00

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,
where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, a)$, is implemented for every different fiber orientation a_i ,
as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, a1)|)$, with:

total thickness per orientation, $t_{f1} = N_L \cdot t / \text{NoDir} = 1.00$

$d_{fv} = d$ (figure 11.2, ACI 440) = 707.00

f_{fe} ((11-5), ACI 440) = 328.00

$E_f = 82000.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.009$

From (11-11), ACI 440: $V_s + V_f \leq 797164.595$

$b_w = 400.00$

displacement_ductility_demand is calculated as δ / y

- Calculation of δ / y for END A -

for rotation axis 3 and integ. section (a)

From analysis, chord rotation $\theta = 3.7135020E-005$

$y = (M_y \cdot L_s / 3) / E_{\text{eff}} = 0.00270555$ ((4.29), Biskinis Phd)

$M_y = 2.6204E+008$

$L_s = M/V$ (with $L_s > 0.1 \cdot L$ and $L_s < 2 \cdot L$) = 3528.173

From table 10.5, ASCE 41_17: $E_{\text{eff}} = \text{factor} \cdot E_c \cdot I_g = 1.1390E+014$

factor = 0.30

$A_g = 300000.00$

Mean concrete strength: $f_c' = (f_c'_{\text{jacket}} \cdot \text{Area}_{\text{jacket}} + f_c'_{\text{core}} \cdot \text{Area}_{\text{core}}) / \text{Area}_{\text{section}} = 33.00$

$N = 14177.47$

$E_c \cdot I_g = E_{c_{\text{jacket}}} \cdot I_{g_{\text{jacket}}} + E_{c_{\text{core}}} \cdot I_{g_{\text{core}}} = 3.7968E+014$

Calculation of Yielding Moment M_y

Calculation of y and M_y according to Annex 7 -

$$y = \text{Min}(y_{\text{ten}}, y_{\text{com}})$$

$$y_{\text{ten}} = 2.8029545\text{E-}006$$

$$\text{with } ((10.1), \text{ASCE 41-17}) f_y = \text{Min}(f_y, 1.25 \cdot f_y \cdot (l_b/d)^{2/3}) = 311.2112$$

$$d = 707.00$$

$$y = 0.214783$$

$$A = 0.00962586$$

$$B = 0.0051813$$

$$\text{with } p_t = 0.00402142$$

$$p_c = 0.00402142$$

$$p_v = 0.00142194$$

$$N = 14177.47$$

$$b = 400.00$$

$$" = 0.06082037$$

$$y_{\text{comp}} = 1.4829079\text{E-}005$$

$$\text{with } f_c^* (12.3, \text{ACI 440}) = 33.35516$$

$$f_l = 33.00$$

$$f_l = 0.77176471$$

$$b = 400.00$$

$$h = 750.00$$

$$A_g = 300000.00$$

$$\text{From } (12.9), \text{ACI 440: } k_a = 0.14679369$$

$$g = p_t + p_c + p_v = 0.00946477$$

$$r_c = 40.00$$

$$A_e/A_c = 0.51607158$$

$$\text{Effective FRP thickness, } t_f = N L^* t \cdot \text{Cos}(b_1) = 1.00$$

$$\text{effective strain from } (12.5) \text{ and } (12.12), e_{fe} = 0.004$$

$$f_u = 0.009$$

$$E_f = 82000.00$$

$$E_c = 26999.444$$

$$y = 0.21210318$$

$$A = 0.00935205$$

$$B = 0.00502021$$

$$\text{with } E_s = 200000.00$$

Calculation of ratio l_b/d

Inadequate Lap Length with $l_b/d = 0.30$

End Of Calculation of Shear Capacity for element: column JC1 of floor 1

At local axis: 2

Integration Section: (a)

Calculation No. 2

column C1, Floor 1

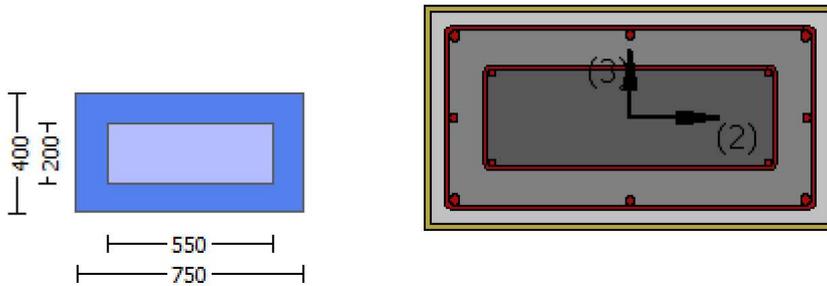
Limit State: Immediate Occupancy (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Chord rotation capacity (θ_u)

Edge: Start

Local Axis: (2)



Start Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At Shear local axis: 3

(Bending local axis: 2)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$

Mean strength values are used for both shear and moment calculations.

Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Existing Column

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

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Note: Especially for the calculation of moment strengths,

the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

Jacket

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

Existing Column

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

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External Height, $H = 400.00$

External Width, $W = 750.00$

Internal Height, $H = 200.00$

Internal Width, $W = 550.00$

Cover Thickness, $c = 25.00$

Mean Confinement Factor overall section = 1.0435

Element Length, $L = 3000.00$

Primary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Inadequate Lap Length with $l_o/l_{ou,min} = 0.30$

FRP Wrapping Data

Type: Carbon

Dry properties (design values)

Thickness, $t = 1.00$

Tensile Strength, $f_{fu} = 840.00$

Tensile Modulus, $E_f = 82000.00$

Elongation, $e_{fu} = 0.009$

Number of directions, $NoDir = 1$

Fiber orientations, $bi = 0.00^\circ$

Number of layers, $NL = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 3

EDGE -A-

Shear Force, $V_a = 3.4586346E-031$

EDGE -B-

Shear Force, $V_b = -3.4586346E-031$

BOTH EDGES

Axial Force, $F = -11016.808$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $As_c = 2676.637$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{t,ten} = 1137.257$

-Compression: $As_{c,com} = 1137.257$

-Middle: $As_{mid} = 402.1239$

Calculation of Shear Capacity ratio , $V_e/V_r = 0.14265244$

Member Controlled by Flexure ($V_e/V_r < 1$)

Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 121055.918$

with

$M_{pr1} = \text{Max}(Mu_{1+} , Mu_{1-}) = 1.8158E+008$

$Mu_{1+} = 1.8158E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination

$Mu_{1-} = 1.8158E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination

$M_{pr2} = \text{Max}(Mu_{2+} , Mu_{2-}) = 1.8158E+008$

$Mu_{2+} = 1.8158E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the the static loading combination

$Mu_{2-} = 1.8158E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the the static loading combination

Calculation of Mu_{1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$\phi_u = 1.7255512E-005$

$M_u = 1.8158E+008$

with full section properties:

$b = 750.00$

$d = 357.00$

$d' = 43.00$

$v = 0.00124684$

$N = 11016.808$

$f_c = 33.00$

ϕ_0 (5A.5, TBDY) = 0.002

Final value of c_u : $c_u^* = \text{shear_factor} * \text{Max}(c_u, c_c) = 0.01006403$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $c_u = 0.01006403$

w_e ((5.4c), TBDY) = $ase^* sh_{\min} * fy_{we} / f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$

where $f = af * pf * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

 $f_x = 0.02504965$

$af = 0.38744444$

$b = 750.00$

$h = 400.00$

From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.00266667$

$bw = 750.00$

effective stress from (A.35), $ff_e = 800.0873$

 $f_y = 0.04491794$

$af = 0.38744444$

$b = 400.00$

$h = 750.00$

From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.005$

$bw = 400.00$

effective stress from (A.35), $ff_e = 765.1636$

 $R = 40.00$

Effective FRP thickness, $tf = NL * t * \text{Cos}(b1) = 1.00$

$f_{u,f} = 840.00$

$E_f = 82000.00$

$u_{,f} = 0.015$

ase ((5.4d), TBDY) = $(ase1 * A_{ext} + ase2 * A_{int}) / A_{sec} = 0.12601038$

$ase1 = 0.12601038$

$bo_1 = 690.00$

$ho_1 = 340.00$

$bi2_1 = 1.1834E+006$

$ase2 = \text{Max}(ase1, ase2) = 0.12601038$

$bo_2 = 542.00$

$ho_2 = 192.00$

$bi2_2 = 661256.00$

$psh_{\min} * fy_{we} = \text{Min}(psh_x * fy_{we}, psh_y * fy_{we}) = 1.64062$

Expression ((5.4d), TBDY) for $psh_{\min} * fy_{we}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

 $psh_x * fy_{we} = psh1 * fy_{we1} + ps2 * fy_{we2} = 1.64062$

$ps1$ (external) = $(Ash1 * h1 / s1) / A_{sec} = 0.0020944$

$Ash1 = Astir_1 * ns_1 = 157.0796$

No stirrups, $ns_1 = 2.00$

$h1 = 400.00$

$ps2$ (internal) = $(Ash2 * h2 / s2) / A_{sec} = 0.00026808$

$Ash2 = Astir_2 * ns_2 = 100.531$

No stirrups, $ns_2 = 2.00$

$h2 = 200.00$

 $psh_y * fy_{we} = psh1 * fy_{we1} + ps2 * fy_{we2} = 3.23907$

$ps1$ (external) = $(Ash1 * h1 / s1) / A_{sec} = 0.00392699$

$Ash1 = Astir_1 * ns_1 = 157.0796$

No stirrups, $ns_1 = 2.00$

$h1 = 750.00$

$ps2$ (internal) = $(Ash2 * h2 / s2) / A_{sec} = 0.00073723$

$Ash2 = Astir_2 * ns_2 = 100.531$

No stirrups, $ns_2 = 2.00$

$h2 = 550.00$

 $A_{sec} = 300000.00$

$s1 = 100.00$

$s2 = 250.00$

$fy_{we1} = 694.45$

$fy_{we2} = 694.45$

$f_{ce} = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00243499$

$c = \text{confinement factor} = 1.0435$
 $y1 = 0.00140044$
 $sh1 = 0.0044814$
 $ft1 = 466.8167$
 $fy1 = 389.0139$
 $su1 = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou,min = lb/ld = 0.30$
 $su1 = 0.4 * esu1_nominal ((5.5), \text{TBDY}) = 0.032$
 From table 5A.1, TBDY: $esu1_nominal = 0.08$,
 For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs1 = (fs,jacket * Asl,ten,jacket + fs,core * Asl,ten,core) / Asl,ten = 389.0139$
 with $Es1 = (Es,jacket * Asl,ten,jacket + Es,core * Asl,ten,core) / Asl,ten = 200000.00$
 $y2 = 0.00140044$
 $sh2 = 0.0044814$
 $ft2 = 466.8167$
 $fy2 = 389.0139$
 $su2 = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou,min = lb/lb,min = 0.30$
 $su2 = 0.4 * esu2_nominal ((5.5), \text{TBDY}) = 0.032$
 From table 5A.1, TBDY: $esu2_nominal = 0.08$,
 For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs2 = (fs,jacket * Asl,com,jacket + fs,core * Asl,com,core) / Asl,com = 389.0139$
 with $Es2 = (Es,jacket * Asl,com,jacket + Es,core * Asl,com,core) / Asl,com = 200000.00$
 $yv = 0.00140044$
 $shv = 0.0044814$
 $ftv = 466.8167$
 $fyv = 389.0139$
 $suv = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou,min = lb/ld = 0.30$
 $suv = 0.4 * esuv_nominal ((5.5), \text{TBDY}) = 0.032$
 From table 5A.1, TBDY: $esuv_nominal = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = (fs,jacket * Asl,mid,jacket + fs,mid * Asl,mid,core) / Asl,mid = 389.0139$
 with $Es_v = (Es,jacket * Asl,mid,jacket + Es,mid * Asl,mid,core) / Asl,mid = 200000.00$
 $1 = Asl,ten / (b * d) * (fs1 / fc) = 0.0500703$
 $2 = Asl,com / (b * d) * (fs2 / fc) = 0.0500703$
 $v = Asl,mid / (b * d) * (fsv / fc) = 0.01770442$
 and confined core properties:
 $b = 690.00$
 $d = 327.00$
 $d' = 13.00$
 $fcc (5A.2, \text{TBDY}) = 34.43547$
 $cc (5A.5, \text{TBDY}) = 0.00243499$
 $c = \text{confinement factor} = 1.0435$
 $1 = Asl,ten / (b * d) * (fs1 / fc) = 0.05941729$
 $2 = Asl,com / (b * d) * (fs2 / fc) = 0.05941729$
 $v = Asl,mid / (b * d) * (fsv / fc) = 0.02100943$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)
 --->
 $v < vs,y2$ - LHS eq.(4.5) is satisfied

--->

$$s_u(4.9) = 0.16886055$$

$$M_u = M_{Rc}(4.14) = 1.8158E+008$$

$$u = s_u(4.1) = 1.7255512E-005$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of M_{u1} -

Calculation of ultimate curvature κ_u according to 4.1, Biskinis/Fardis 2013:

$$\kappa_u = 1.7255512E-005$$

$$M_u = 1.8158E+008$$

with full section properties:

$$b = 750.00$$

$$d = 357.00$$

$$d' = 43.00$$

$$v = 0.00124684$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$\alpha_{co}(5A.5, TBDY) = 0.002$$

$$\text{Final value of } \kappa_u: \kappa_u^* = \text{shear_factor} * \text{Max}(\kappa_u, \kappa_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \kappa_u = 0.01006403$$

$$\omega_{ve}((5.4c), TBDY) = \alpha_{se} * \text{sh}_{,\min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$$

where $f = \alpha_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.02504965$$

$$\alpha_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.00266667$$

$$b_w = 750.00$$

$$\text{effective stress from (A.35), } f_{fe} = 800.0873$$

$$f_y = 0.04491794$$

$$\alpha_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.005$$

$$b_w = 400.00$$

$$\text{effective stress from (A.35), } f_{fe} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL * t * \text{Cos}(b_1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{,f} = 0.015$$

$$\alpha_{se}((5.4d), TBDY) = (\alpha_{se1} * A_{ext} + \alpha_{se2} * A_{int}) / A_{sec} = 0.12601038$$

$$\alpha_{se1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i2,1} = 1.1834E+006$$

$$\alpha_{se2} = \text{Max}(\alpha_{se1}, \alpha_{se2}) = 0.12601038$$

$$b_{o,2} = 542.00$$

$$h_{o,2} = 192.00$$

$$b_{i2,2} = 661256.00$$

$$p_{sh,\min} * F_{ywe} = \text{Min}(p_{sh,x} * F_{ywe}, p_{sh,y} * F_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $p_{sh,\min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh_x*Fywe = psh1*Fywe1+ps2*Fywe2 = 1.64062
ps1 (external) = (Ash1*h1/s1)/Asec = 0.0020944
Ash1 = Astir_1*ns_1 = 157.0796
No stirups, ns_1 = 2.00
h1 = 400.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00026808
Ash2 = Astir_2*ns_2 = 100.531
No stirups, ns_2 = 2.00
h2 = 200.00

psh_y*Fywe = psh1*Fywe1+ps2*Fywe2 = 3.23907
ps1 (external) = (Ash1*h1/s1)/Asec = 0.00392699
Ash1 = Astir_1*ns_1 = 157.0796
No stirups, ns_1 = 2.00
h1 = 750.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00073723
Ash2 = Astir_2*ns_2 = 100.531
No stirups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044
sh2 = 0.0044814
ft2 = 466.8167
fy2 = 389.0139
su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044
shv = 0.0044814
ftv = 466.8167
fyv = 389.0139
suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

lo/lo_{u,min} = lb/d = 0.30

su_v = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fs_{yv} = fsv/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and y_v, sh_v,ft_v,fy_v, it is considered characteristic value fs_{yv} = fsv/1.2, from table 5.1, TBDY.

y₁, sh₁,ft₁,fy₁, are also multiplied by Min(1,1.25*(lb/d)^{2/3}), from 10.3.5, ASCE 41-17.

with fsv = (fs_{jacket}*Asl_{mid,jacket} + fs_{mid}*Asl_{mid,core})/Asl_{mid} = 389.0139

with Es_v = (Es_{jacket}*Asl_{mid,jacket} + Es_{mid}*Asl_{mid,core})/Asl_{mid} = 200000.00

1 = Asl_{ten}/(b*d)*(fs₁/fc) = 0.0500703

2 = Asl_{com}/(b*d)*(fs₂/fc) = 0.0500703

v = Asl_{mid}/(b*d)*(fsv/fc) = 0.01770442

and confined core properties:

b = 690.00

d = 327.00

d' = 13.00

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

1 = Asl_{ten}/(b*d)*(fs₁/fc) = 0.05941729

2 = Asl_{com}/(b*d)*(fs₂/fc) = 0.05941729

v = Asl_{mid}/(b*d)*(fsv/fc) = 0.02100943

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

v < v_{s,y2} - LHS eq.(4.5) is satisfied

--->

su (4.9) = 0.16886055

Mu = MRc (4.14) = 1.8158E+008

u = su (4.1) = 1.7255512E-005

Calculation of ratio lb/d

Inadequate Lap Length with lb/d = 0.30

Calculation of Mu₂₊

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

u = 1.7255512E-005

Mu = 1.8158E+008

with full section properties:

b = 750.00

d = 357.00

d' = 43.00

v = 0.00124684

N = 11016.808

fc = 33.00

co (5A.5, TBDY) = 0.002

Final value of cu: cu* = shear_factor * Max(cu, cc) = 0.01006403

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: cu = 0.01006403

we ((5.4c), TBDY) = ase* sh_{min}*fy_{we}/f_{ce}+Min(fx, fy) = 0.02692906

where f = af*pf*ffe/f_{ce} is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

fx = 0.02504965

af = 0.38744444

b = 750.00

h = 400.00

From EC8 A.4.4.3(6), pf = 2tf/bw = 0.00266667

bw = 750.00
effective stress from (A.35), $f_{f,e} = 800.0873$

fy = 0.04491794
af = 0.38744444
b = 400.00
h = 750.00
From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.005$
bw = 400.00
effective stress from (A.35), $f_{f,e} = 765.1636$

R = 40.00
Effective FRP thickness, $t_f = NL*t*Cos(b1) = 1.00$
fu,f = 840.00
Ef = 82000.00
u,f = 0.015
ase ((5.4d), TBDY) = $(ase1*A_{ext} + ase2*A_{int})/A_{sec} = 0.12601038$
ase1 = 0.12601038
bo_1 = 690.00
ho_1 = 340.00
bi2_1 = 1.1834E+006
ase2 = $Max(ase1, ase2) = 0.12601038$
bo_2 = 542.00
ho_2 = 192.00
bi2_2 = 661256.00

$p_{sh,min}*F_{ywe} = Min(p_{sh,x}*F_{ywe}, p_{sh,y}*F_{ywe}) = 1.64062$

Expression ((5.4d), TBDY) for $p_{sh,min}*F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$p_{sh,x}*F_{ywe} = p_{sh1}*F_{ywe1} + p_{sh2}*F_{ywe2} = 1.64062$
ps1 (external) = $(A_{sh1}*h1/s1)/A_{sec} = 0.0020944$
Ash1 = $A_{stir_1}*n_{s_1} = 157.0796$
No stirrups, $n_{s_1} = 2.00$
h1 = 400.00
ps2 (internal) = $(A_{sh2}*h2/s2)/A_{sec} = 0.00026808$
Ash2 = $A_{stir_2}*n_{s_2} = 100.531$
No stirrups, $n_{s_2} = 2.00$
h2 = 200.00

$p_{sh,y}*F_{ywe} = p_{sh1}*F_{ywe1} + p_{sh2}*F_{ywe2} = 3.23907$
ps1 (external) = $(A_{sh1}*h1/s1)/A_{sec} = 0.00392699$
Ash1 = $A_{stir_1}*n_{s_1} = 157.0796$
No stirrups, $n_{s_1} = 2.00$
h1 = 750.00
ps2 (internal) = $(A_{sh2}*h2/s2)/A_{sec} = 0.00073723$
Ash2 = $A_{stir_2}*n_{s_2} = 100.531$
No stirrups, $n_{s_2} = 2.00$
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: $cc = 0.00243499$
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = $l_b/l_d = 0.30$
su1 = $0.4*es_{u1,nominal} ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = (fs_{jacket} \cdot Asl_{ten,jacket} + fs_{core} \cdot Asl_{ten,core}) / Asl_{ten} = 389.0139$

with $Es1 = (Es_{jacket} \cdot Asl_{ten,jacket} + Es_{core} \cdot Asl_{ten,core}) / Asl_{ten} = 200000.00$

$y2 = 0.00140044$

$sh2 = 0.0044814$

$ft2 = 466.8167$

$fy2 = 389.0139$

$su2 = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with $shear_factor$

and also multiplied by the $shear_factor$ according to 15.7.1.4, with

$Shear_factor = 1.00$

$lo/lou, min = lb/lb, min = 0.30$

$su2 = 0.4 \cdot esu2_nominal$ ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = (fs_{jacket} \cdot Asl_{com,jacket} + fs_{core} \cdot Asl_{com,core}) / Asl_{com} = 389.0139$

with $Es2 = (Es_{jacket} \cdot Asl_{com,jacket} + Es_{core} \cdot Asl_{com,core}) / Asl_{com} = 200000.00$

$yv = 0.00140044$

$shv = 0.0044814$

$ftv = 466.8167$

$fyv = 389.0139$

$suv = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with $shear_factor$

and also multiplied by the $shear_factor$ according to 15.7.1.4, with

$Shear_factor = 1.00$

$lo/lou, min = lb/ld = 0.30$

$suv = 0.4 \cdot esuv_nominal$ ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: $esuv_nominal = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY

For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fsv = (fs_{jacket} \cdot Asl_{mid,jacket} + fs_{mid} \cdot Asl_{mid,core}) / Asl_{mid} = 389.0139$

with $Es_v = (Es_{jacket} \cdot Asl_{mid,jacket} + Es_{mid} \cdot Asl_{mid,core}) / Asl_{mid} = 200000.00$

$1 = Asl_{ten} / (b \cdot d) \cdot (fs1 / fc) = 0.0500703$

$2 = Asl_{com} / (b \cdot d) \cdot (fs2 / fc) = 0.0500703$

$v = Asl_{mid} / (b \cdot d) \cdot (fsv / fc) = 0.01770442$

and confined core properties:

$b = 690.00$

$d = 327.00$

$d' = 13.00$

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

$c =$ confinement factor = 1.0435

$1 = Asl_{ten} / (b \cdot d) \cdot (fs1 / fc) = 0.05941729$

$2 = Asl_{com} / (b \cdot d) \cdot (fs2 / fc) = 0.05941729$

$v = Asl_{mid} / (b \cdot d) \cdot (fsv / fc) = 0.02100943$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < vs, y2$ - LHS eq.(4.5) is satisfied

--->

su (4.9) = 0.16886055

$Mu = MRc$ (4.14) = 1.8158E+008

$u = su$ (4.1) = 1.7255512E-005

Calculation of ratio lb/ld

Inadequate Lap Length with $lb/ld = 0.30$

Calculation of Mu2-

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:

$$\mu = 1.7255512E-005$$

$$\text{Mu} = 1.8158E+008$$

with full section properties:

$$b = 750.00$$

$$d = 357.00$$

$$d' = 43.00$$

$$v = 0.00124684$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$c_o (5A.5, \text{TBDY}) = 0.002$$

$$\text{Final value of } c_u: c_u^* = \text{shear_factor} * \text{Max}(c_u, c_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } c_u = 0.01006403$$

$$w_e ((5.4c), \text{TBDY}) = a_{se} * s_{h,\min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$$

where $f = a_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.02504965$$

$$a_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.00266667$$

$$b_w = 750.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 800.0873$$

$$f_y = 0.04491794$$

$$a_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.005$$

$$b_w = 400.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L * t * \text{Cos}(b_1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{,f} = 0.015$$

$$a_{se} ((5.4d), \text{TBDY}) = (a_{se1} * A_{ext} + a_{se2} * A_{int}) / A_{sec} = 0.12601038$$

$$a_{se1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i2,1} = 1.1834E+006$$

$$a_{se2} = \text{Max}(a_{se1}, a_{se2}) = 0.12601038$$

$$b_{o,2} = 542.00$$

$$h_{o,2} = 192.00$$

$$b_{i2,2} = 661256.00$$

$$p_{sh,\min} * F_{ywe} = \text{Min}(p_{sh,x} * F_{ywe}, p_{sh,y} * F_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $p_{sh,\min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$p_{sh,x} * F_{ywe} = p_{s1} * F_{ywe1} + p_{s2} * F_{ywe2} = 1.64062$$

$$p_{s1} (\text{external}) = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$$

$$A_{sh1} = A_{stir,1} * n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

$$h_1 = 400.00$$

$$p_{s2} (\text{internal}) = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00026808$$

$$A_{sh2} = A_{stir,2} * n_{s,2} = 100.531$$

$$\text{No stirrups, } n_{s,2} = 2.00$$

$$h_2 = 200.00$$

psh_y*Fywe = psh₁*Fywe₁+ps₂*Fywe₂ = 3.23907
ps₁ (external) = (Ash₁*h₁/s₁)/Asec = 0.00392699
Ash₁ = Astir₁*ns₁ = 157.0796
No stirups, ns₁ = 2.00
h₁ = 750.00
ps₂ (internal) = (Ash₂*h₂/s₂)/Asec = 0.00073723
Ash₂ = Astir₂*ns₂ = 100.531
No stirups, ns₂ = 2.00
h₂ = 550.00

Asec = 300000.00

s₁ = 100.00

s₂ = 250.00

fywe₁ = 694.45

fywe₂ = 694.45

fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499

c = confinement factor = 1.0435

y₁ = 0.00140044

sh₁ = 0.0044814

ft₁ = 466.8167

fy₁ = 389.0139

su₁ = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

su₁ = 0.4*esu_{1_nominal} ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu_{1_nominal} = 0.08,

For calculation of esu_{1_nominal} and y₁, sh₁,ft₁,fy₁, it is considered
characteristic value fsy₁ = fs₁/1.2, from table 5.1, TBDY.

y₁, sh₁,ft₁,fy₁, are also multiplied by Min(1,1.25*(lb/ld)^{2/3}), from 10.3.5, ASCE 41-17.

with fs₁ = (fs_{jacket}*Asl_{ten,jacket} + fs_{core}*Asl_{ten,core})/Asl_{ten} = 389.0139

with Es₁ = (Es_{jacket}*Asl_{ten,jacket} + Es_{core}*Asl_{ten,core})/Asl_{ten} = 200000.00

y₂ = 0.00140044

sh₂ = 0.0044814

ft₂ = 466.8167

fy₂ = 389.0139

su₂ = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su₂ = 0.4*esu_{2_nominal} ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu_{2_nominal} = 0.08,

For calculation of esu_{2_nominal} and y₂, sh₂,ft₂,fy₂, it is considered
characteristic value fsy₂ = fs₂/1.2, from table 5.1, TBDY.

y₂, sh₂,ft₂,fy₂, are also multiplied by Min(1,1.25*(lb/ld)^{2/3}), from 10.3.5, ASCE 41-17.

with fs₂ = (fs_{jacket}*Asl_{com,jacket} + fs_{core}*Asl_{com,core})/Asl_{com} = 389.0139

with Es₂ = (Es_{jacket}*Asl_{com,jacket} + Es_{core}*Asl_{com,core})/Asl_{com} = 200000.00

y_v = 0.00140044

sh_v = 0.0044814

ft_v = 466.8167

fy_v = 389.0139

suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

suv = 0.4*esuv_{nominal} ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_{nominal} = 0.08,

considering characteristic value fsy_v = fs_v/1.2, from table 5.1, TBDY

For calculation of esuv_{nominal} and y_v, sh_v,ft_v,fy_v, it is considered
characteristic value fsy_v = fs_v/1.2, from table 5.1, TBDY.

y₁, sh₁,ft₁,fy₁, are also multiplied by Min(1,1.25*(lb/ld)^{2/3}), from 10.3.5, ASCE 41-17.

with fs_v = (fs_{jacket}*Asl_{mid,jacket} + fs_{mid}*Asl_{mid,core})/Asl_{mid} = 389.0139

with Es_v = (Es_{jacket}*Asl_{mid,jacket} + Es_{mid}*Asl_{mid,core})/Asl_{mid} = 200000.00

$$1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.0500703$$

$$2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.0500703$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.01770442$$

and confined core properties:

$$b = 690.00$$

$$d = 327.00$$

$$d' = 13.00$$

$$f_{cc} (5A.2, TBDY) = 34.43547$$

$$c_c (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.05941729$$

$$2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.05941729$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.02100943$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$s_u (4.9) = 0.16886055$$

$$\mu_u = M/R_c (4.14) = 1.8158E+008$$

$$u = s_u (4.1) = 1.7255512E-005$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 848607.437$

Calculation of Shear Strength at edge 1, $V_{r1} = 848607.437$

$V_{r1} = V_{Col} ((10.3), ASCE 41-17) = k_{nl} * V_{Col0}$

$$V_{Col0} = 848607.437$$

$k_{nl} = 1$ (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v * f_y * d / s$ ' is replaced by ' $V_{s+} = f * V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: $f'_c = (f'_{c,jacket} * Area_{jacket} + f'_{c,core} * Area_{core}) / Area_{section} = 33.00$, but $f'_c^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$$M/Vd = 2.00$$

$$\mu_u = 3.5345895E-012$$

$$V_u = 3.4586346E-031$$

$$d = 0.8 * h = 320.00$$

$$N_u = 11016.808$$

$$A_g = 300000.00$$

$$\text{From (11.5.4.8), ACI 318-14: } V_s = V_{s1} + V_{s2} = 279254.914$$

where:

$V_{s1} = 279254.914$ is calculated for jacket, with:

$$d = 320.00$$

$$A_v = 157079.633$$

$$f_y = 555.56$$

$$s = 100.00$$

V_{s1} is multiplied by $Col1 = 1.00$

$$s/d = 0.3125$$

$V_{s2} = 0.00$ is calculated for core, with:

$$d = 160.00$$

$$A_v = 100530.965$$

$$f_y = 555.56$$

$$s = 250.00$$

V_{s2} is multiplied by $Col2 = 0.00$

$$s/d = 1.5625$$

$V_f ((11-3)-(11.4), ACI 440) = 234192.00$

$f = 0.95$, for fully-wrapped sections

$wf/sf = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin \theta + \cos \theta$ is replaced with $(\cot \theta + \csc \theta) \sin \alpha$ which is more a generalised expression, where θ is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, \alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, \theta)|)$, with:

total thickness per orientation, $tf1 = NL * t / \text{NoDir} = 1.00$

$dfv = d$ (figure 11.2, ACI 440) = 357.00

ffe ((11-5), ACI 440) = 328.00

$E_f = 82000.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.009$

From (11-11), ACI 440: $V_s + V_f \leq 915872.391$

$bw = 750.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 848607.437$

$V_{r2} = V_{Co1}$ ((10.3), ASCE 41-17) = $k_n l * V_{Co10}$

$V_{Co10} = 848607.437$

$k_n l = 1$ (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v * f_y * d / s$ ' is replaced by ' $V_s + f * V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

$\gamma_c = 1$ (normal-weight concrete)

Mean concrete strength: $f'_c = (f'_c \text{ jacket} * \text{Area jacket} + f'_c \text{ core} * \text{Area core}) / \text{Area section} = 33.00$, but $f'_c^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$M / Vd = 2.00$

$\mu_u = 3.5345895E-012$

$\nu_u = 3.4586346E-031$

$d = 0.8 * h = 320.00$

$N_u = 11016.808$

$A_g = 300000.00$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 279254.914$

where:

$V_{s1} = 279254.914$ is calculated for jacket, with:

$d = 320.00$

$A_v = 157079.633$

$f_y = 555.56$

$s = 100.00$

V_{s1} is multiplied by $Col1 = 1.00$

$s/d = 0.3125$

$V_{s2} = 0.00$ is calculated for core, with:

$d = 160.00$

$A_v = 100530.965$

$f_y = 555.56$

$s = 250.00$

V_{s2} is multiplied by $Col2 = 0.00$

$s/d = 1.5625$

V_f ((11-3)-(11.4), ACI 440) = 234192.00

$f = 0.95$, for fully-wrapped sections

$wf/sf = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin \theta + \cos \theta$ is replaced with $(\cot \theta + \csc \theta) \sin \alpha$ which is more a generalised expression, where θ is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, \alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, \theta)|)$, with:

total thickness per orientation, $tf1 = NL * t / \text{NoDir} = 1.00$

$dfv = d$ (figure 11.2, ACI 440) = 357.00

ffe ((11-5), ACI 440) = 328.00

$E_f = 82000.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.009$

From (11-11), ACI 440: $V_s + V_f \leq 915872.391$

$bw = 750.00$

End Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1
At local axis: 3

Start Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1
At Shear local axis: 2
(Bending local axis: 3)
Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$
Mean strength values are used for both shear and moment calculations.
Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$
New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$
Concrete Elasticity, $E_c = 26999.444$
Steel Elasticity, $E_s = 200000.00$

Existing Column

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$
New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$
Concrete Elasticity, $E_c = 26999.444$
Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of moment strengths,
the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

Jacket

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

Existing Column

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

#####

External Height, $H = 400.00$

External Width, $W = 750.00$

Internal Height, $H = 200.00$

Internal Width, $W = 550.00$

Cover Thickness, $c = 25.00$

Mean Confinement Factor overall section = 1.0435

Element Length, $L = 3000.00$

Primary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Inadequate Lap Length with $l_o/l_{ou, \min} = 0.30$

FRP Wrapping Data

Type: Carbon

Dry properties (design values)

Thickness, $t = 1.00$

Tensile Strength, $f_{fu} = 840.00$

Tensile Modulus, $E_f = 82000.00$

Elongation, $e_{fu} = 0.009$

Number of directions, $NoDir = 1$

Fiber orientations, $bi: 0.00^\circ$

Number of layers, $NL = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 2

EDGE -A-

Shear Force, $V_a = 2.9582284E-031$

EDGE -B-

Shear Force, $V_b = -2.9582284E-031$

BOTH EDGES

Axial Force, $F = -11016.808$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $A_{st} = 0.00$

-Compression: $A_{sc} = 2676.637$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $A_{st,ten} = 1137.257$

-Compression: $A_{sc,com} = 1137.257$

-Middle: $A_{sc,mid} = 402.1239$

Calculation of Shear Capacity ratio, $V_e/V_r = 0.19671905$

Member Controlled by Flexure ($V_e/V_r < 1$)

Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 248405.514$

with

$M_{pr1} = \text{Max}(Mu_{1+}, Mu_{1-}) = 3.7261E+008$

$Mu_{1+} = 3.7261E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination

$Mu_{1-} = 3.7261E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination

$M_{pr2} = \text{Max}(Mu_{2+}, Mu_{2-}) = 3.7261E+008$

$Mu_{2+} = 3.7261E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the the static loading combination

$Mu_{2-} = 3.7261E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the the static loading combination

Calculation of Mu_{1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$\phi_u = 8.5888248E-006$

$M_u = 3.7261E+008$

with full section properties:

$b = 400.00$

$d = 707.00$

$d' = 43.00$

$v = 0.00118049$

$N = 11016.808$

$f_c = 33.00$

$\alpha_{co} (5A.5, TBDY) = 0.002$

Final value of ϕ_u : $\phi_u^* = \text{shear_factor} * \text{Max}(\phi_u, \phi_c) = 0.01006403$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $\phi_u = 0.01006403$

where $\phi_u (5.4c, TBDY) = \alpha_{se} * \text{sh}_{,min} * f_{ywe}/f_{ce} + \text{Min}(\phi_x, \phi_y) = 0.02692906$

where $\phi_x = \alpha_{se} * \phi_{f,ff} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$\phi_x = 0.02504965$

$\alpha_{se} = 0.38744444$

$b = 750.00$

$h = 400.00$

From EC8 A.4.4.3(6), $\phi_{f,ff} = 2t_f/b_w = 0.00266667$

$b_w = 750.00$

effective stress from (A.35), $f_{f,e} = 800.0873$

$\phi_y = 0.04491794$

$\alpha_{se} = 0.38744444$

$b = 400.00$

$h = 750.00$

From EC8 A.4.4.3(6), $\phi_{f,ff} = 2t_f/b_w = 0.005$

$b_w = 400.00$

effective stress from (A.35), $f_{f,e} = 765.1636$

R = 40.00

Effective FRP thickness, $t_f = NL * t * \cos(b_1) = 1.00$

$f_{u,f} = 840.00$

$E_f = 82000.00$

$u_{,f} = 0.015$

$ase((5.4d), TBDY) = (ase_1 * A_{ext} + ase_2 * A_{int}) / A_{sec} = 0.12601038$

$ase_1 = 0.12601038$

$bo_1 = 690.00$

$ho_1 = 340.00$

$bi_{2,1} = 1.1834E+006$

$ase_2 = \max(ase_1, ase_2) = 0.12601038$

$bo_2 = 542.00$

$ho_2 = 192.00$

$bi_{2,2} = 661256.00$

$p_{sh,min} * F_{ywe} = \min(p_{sh,x} * F_{ywe}, p_{sh,y} * F_{ywe}) = 1.64062$

Expression ((5.4d), TBDY) for $p_{sh,min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$p_{sh,x} * F_{ywe} = p_{sh1} * F_{ywe1} + ps_2 * F_{ywe2} = 1.64062$

ps_1 (external) = $(A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$

$A_{sh1} = A_{stir,1} * ns_1 = 157.0796$

No stirrups, $ns_1 = 2.00$

$h_1 = 400.00$

ps_2 (internal) = $(A_{sh2} * h_2 / s_2) / A_{sec} = 0.00026808$

$A_{sh2} = A_{stir,2} * ns_2 = 100.531$

No stirrups, $ns_2 = 2.00$

$h_2 = 200.00$

$p_{sh,y} * F_{ywe} = p_{sh1} * F_{ywe1} + ps_2 * F_{ywe2} = 3.23907$

ps_1 (external) = $(A_{sh1} * h_1 / s_1) / A_{sec} = 0.00392699$

$A_{sh1} = A_{stir,1} * ns_1 = 157.0796$

No stirrups, $ns_1 = 2.00$

$h_1 = 750.00$

ps_2 (internal) = $(A_{sh2} * h_2 / s_2) / A_{sec} = 0.00073723$

$A_{sh2} = A_{stir,2} * ns_2 = 100.531$

No stirrups, $ns_2 = 2.00$

$h_2 = 550.00$

$A_{sec} = 300000.00$

$s_1 = 100.00$

$s_2 = 250.00$

$f_{ywe1} = 694.45$

$f_{ywe2} = 694.45$

$f_{ce} = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00243499$

c = confinement factor = 1.0435

$y_1 = 0.00140044$

$sh_1 = 0.0044814$

$ft_1 = 466.8167$

$fy_1 = 389.0139$

$su_1 = 0.00512$

using (30) in Bisikinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$lo/lo_{u,min} = lb/ld = 0.30$

$su_1 = 0.4 * esu_{1,nominal}((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu_{1,nominal} = 0.08$,

For calculation of $esu_{1,nominal}$ and y_1, sh_1, ft_1, fy_1 , it is considered characteristic value $fs_1 = fs_1 / 1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\min(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs_1 = (f_{s,jacket} * A_{s1,ten,jacket} + f_{s,core} * A_{s1,ten,core}) / A_{s1,ten} = 389.0139$

with $Es_1 = (E_{s,jacket} * A_{s1,ten,jacket} + E_{s,core} * A_{s1,ten,core}) / A_{s1,ten} = 200000.00$

$y_2 = 0.00140044$

$sh_2 = 0.0044814$

$ft_2 = 466.8167$

$$f_y2 = 389.0139$$

$$s_u2 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$l_o/l_{o,u,min} = l_b/l_{b,min} = 0.30$$

$$s_u2 = 0.4 * e_{su2,nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $e_{su2,nominal} = 0.08$,

For calculation of $e_{su2,nominal}$ and y_2 , sh_2, ft_2, f_y2 , it is considered
characteristic value $f_{sy2} = f_s2/1.2$, from table 5.1, TBDY.

y_1 , sh_1, ft_1, f_y1 , are also multiplied by $\text{Min}(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } f_{s2} = (f_{s,jacket} * A_{s1,com,jacket} + f_{s,core} * A_{s1,com,core}) / A_{s1,com} = 389.0139$$

$$\text{with } E_{s2} = (E_{s,jacket} * A_{s1,com,jacket} + E_{s,core} * A_{s1,com,core}) / A_{s1,com} = 200000.00$$

$$y_v = 0.00140044$$

$$sh_v = 0.0044814$$

$$f_{tv} = 466.8167$$

$$f_{yv} = 389.0139$$

$$s_{uv} = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$l_o/l_{o,u,min} = l_b/l_d = 0.30$$

$$s_{uv} = 0.4 * e_{suv,nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $e_{suv,nominal} = 0.08$,

considering characteristic value $f_{syv} = f_{sv}/1.2$, from table 5.1, TBDY

For calculation of $e_{suv,nominal}$ and y_v , sh_v, f_{tv}, f_{yv} , it is considered
characteristic value $f_{syv} = f_{sv}/1.2$, from table 5.1, TBDY.

y_1 , sh_1, ft_1, f_y1 , are also multiplied by $\text{Min}(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } f_{sv} = (f_{s,jacket} * A_{s1,mid,jacket} + f_{s,mid} * A_{s1,mid,core}) / A_{s1,mid} = 389.0139$$

$$\text{with } E_{sv} = (E_{s,jacket} * A_{s1,mid,jacket} + E_{s,mid} * A_{s1,mid,core}) / A_{s1,mid} = 200000.00$$

$$1 = A_{s1,ten} / (b * d) * (f_{s1} / f_c) = 0.04740567$$

$$2 = A_{s1,com} / (b * d) * (f_{s2} / f_c) = 0.04740567$$

$$v = A_{s1,mid} / (b * d) * (f_{sv} / f_c) = 0.01676222$$

and confined core properties:

$$b = 340.00$$

$$d = 677.00$$

$$d' = 13.00$$

$$f_{cc} (5A.2, TBDY) = 34.43547$$

$$c_c (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = A_{s1,ten} / (b * d) * (f_{s1} / f_c) = 0.05824278$$

$$2 = A_{s1,com} / (b * d) * (f_{s2} / f_c) = 0.05824278$$

$$v = A_{s1,mid} / (b * d) * (f_{sv} / f_c) = 0.02059413$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$s_u (4.9) = 0.15682678$$

$$M_u = M_{Rc} (4.14) = 3.7261E+008$$

$$u = s_u (4.1) = 8.5888248E-006$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of M_{u1} -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$$u = 8.5888248E-006$$

$$M_u = 3.7261E+008$$

with full section properties:

$$b = 400.00$$

$$d = 707.00$$

$$d' = 43.00$$

$$v = 0.00118049$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$\alpha (5A.5, TBDY) = 0.002$$

$$\text{Final value of } c_u: c_u^* = \text{shear_factor} * \text{Max}(c_u, c_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } c_u = 0.01006403$$

$$w_e ((5.4c), TBDY) = \alpha s_e * \text{sh}_{\min} * f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$$

where $f = \alpha * p_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.02504965$$

$$\alpha f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f / b_w = 0.00266667$$

$$b_w = 750.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 800.0873$$

$$f_y = 0.04491794$$

$$\alpha f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f / b_w = 0.005$$

$$b_w = 400.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L * t * \text{Cos}(b_1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{,f} = 0.015$$

$$\alpha s_e ((5.4d), TBDY) = (\alpha s_{e1} * A_{ext} + \alpha s_{e2} * A_{int}) / A_{sec} = 0.12601038$$

$$\alpha s_{e1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i2,1} = 1.1834E+006$$

$$\alpha s_{e2} = \text{Max}(\alpha s_{e1}, \alpha s_{e2}) = 0.12601038$$

$$b_{o,2} = 542.00$$

$$h_{o,2} = 192.00$$

$$b_{i2,2} = 661256.00$$

$$p_{sh,\min} * F_{ywe} = \text{Min}(p_{sh,x} * F_{ywe}, p_{sh,y} * F_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $p_{sh,\min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$p_{sh,x} * F_{ywe} = p_{sh1} * F_{ywe1} + p_{sh2} * F_{ywe2} = 1.64062$$

$$p_{s1} (\text{external}) = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$$

$$A_{sh1} = A_{stir,1} * n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

$$h_1 = 400.00$$

$$p_{s2} (\text{internal}) = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00026808$$

$$A_{sh2} = A_{stir,2} * n_{s,2} = 100.531$$

$$\text{No stirrups, } n_{s,2} = 2.00$$

$$h_2 = 200.00$$

$$p_{sh,y} * F_{ywe} = p_{sh1} * F_{ywe1} + p_{sh2} * F_{ywe2} = 3.23907$$

$$p_{s1} (\text{external}) = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.00392699$$

$$A_{sh1} = A_{stir,1} * n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

$$h_1 = 750.00$$

$$p_{s2} (\text{internal}) = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00073723$$

$$A_{sh2} = A_{stir,2} * n_{s,2} = 100.531$$

$$\text{No stirrups, } n_{s,2} = 2.00$$

$$h_2 = 550.00$$

Asec = 300000.00

s1 = 100.00

s2 = 250.00

fywe1 = 694.45

fywe2 = 694.45

fce = 33.00

From ((5.A.5), TBDY), TBDY: cc = 0.00243499

c = confinement factor = 1.0435

y1 = 0.00140044

sh1 = 0.0044814

ft1 = 466.8167

fy1 = 389.0139

su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = (fsjacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044

sh2 = 0.0044814

ft2 = 466.8167

fy2 = 389.0139

su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044

shv = 0.0044814

ftv = 466.8167

fyv = 389.0139

suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.04740567

2 = Asl,com/(b*d)*(fs2/fc) = 0.04740567

v = Asl,mid/(b*d)*(fsv/fc) = 0.01676222

and confined core properties:

b = 340.00

d = 677.00

d' = 13.00

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

1 = $Asl_{ten}/(b*d)*(fs1/fc) = 0.05824278$

2 = $Asl_{com}/(b*d)*(fs2/fc) = 0.05824278$

v = $Asl_{mid}/(b*d)*(fsv/fc) = 0.02059413$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

v < $v_{s,y2}$ - LHS eq.(4.5) is satisfied

su (4.9) = 0.15682678

Mu = MRc (4.14) = 3.7261E+008

u = su (4.1) = 8.5888248E-006

Calculation of ratio lb/d

Inadequate Lap Length with lb/d = 0.30

Calculation of Mu2+

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

u = 8.5888248E-006

Mu = 3.7261E+008

with full section properties:

b = 400.00

d = 707.00

d' = 43.00

v = 0.00118049

N = 11016.808

fc = 33.00

co (5A.5, TBDY) = 0.002

Final value of cu: $cu^* = shear_factor * Max(cu, cc) = 0.01006403$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $cu = 0.01006403$

we ((5.4c), TBDY) = $ase * sh_{,min} * fy_{we}/f_{ce} + Min(f_x, f_y) = 0.02692906$

where $f = af * pf * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

fx = 0.02504965

af = 0.38744444

b = 750.00

h = 400.00

From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.00266667$

bw = 750.00

effective stress from (A.35), $ff_e = 800.0873$

fy = 0.04491794

af = 0.38744444

b = 400.00

h = 750.00

From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.005$

bw = 400.00

effective stress from (A.35), $ff_e = 765.1636$

R = 40.00

Effective FRP thickness, $tf = NL * t * Cos(b1) = 1.00$

fu,f = 840.00

Ef = 82000.00

u,f = 0.015

ase ((5.4d), TBDY) = $(ase1 * A_{ext} + ase2 * A_{int}) / A_{sec} = 0.12601038$

ase1 = 0.12601038

bo_1 = 690.00

ho_1 = 340.00

$$bi2_1 = 1.1834E+006$$

$$ase2 = \text{Max}(ase1, ase2) = 0.12601038$$

$$bo_2 = 542.00$$

$$ho_2 = 192.00$$

$$bi2_2 = 661256.00$$

$$psh, \text{min} * Fywe = \text{Min}(psh, x * Fywe, psh, y * Fywe) = 1.64062$$

Expression ((5.4d), TBDY) for psh, min * Fywe has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$psh_x * Fywe = psh1 * Fywe1 + ps2 * Fywe2 = 1.64062$$

$$ps1 \text{ (external)} = (Ash1 * h1 / s1) / Asec = 0.0020944$$

$$Ash1 = Astir_1 * ns_1 = 157.0796$$

$$\text{No stirrups, } ns_1 = 2.00$$

$$h1 = 400.00$$

$$ps2 \text{ (internal)} = (Ash2 * h2 / s2) / Asec = 0.00026808$$

$$Ash2 = Astir_2 * ns_2 = 100.531$$

$$\text{No stirrups, } ns_2 = 2.00$$

$$h2 = 200.00$$

$$psh_y * Fywe = psh1 * Fywe1 + ps2 * Fywe2 = 3.23907$$

$$ps1 \text{ (external)} = (Ash1 * h1 / s1) / Asec = 0.00392699$$

$$Ash1 = Astir_1 * ns_1 = 157.0796$$

$$\text{No stirrups, } ns_1 = 2.00$$

$$h1 = 750.00$$

$$ps2 \text{ (internal)} = (Ash2 * h2 / s2) / Asec = 0.00073723$$

$$Ash2 = Astir_2 * ns_2 = 100.531$$

$$\text{No stirrups, } ns_2 = 2.00$$

$$h2 = 550.00$$

$$Asec = 300000.00$$

$$s1 = 100.00$$

$$s2 = 250.00$$

$$fywe1 = 694.45$$

$$fywe2 = 694.45$$

$$fce = 33.00$$

$$\text{From ((5.A5), TBDY), TBDY: } cc = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$y1 = 0.00140044$$

$$sh1 = 0.0044814$$

$$ft1 = 466.8167$$

$$fy1 = 389.0139$$

$$su1 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$$lo/lou, \text{min} = lb/ld = 0.30$$

$$su1 = 0.4 * esu1_nominal \text{ ((5.5), TBDY)} = 0.032$$

$$\text{From table 5A.1, TBDY: } esu1_nominal = 0.08,$$

For calculation of esu1_nominal and y1, sh1, ft1, fy1, it is considered characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

$$y1, sh1, ft1, fy1, \text{ are also multiplied by } \text{Min}(1, 1.25 * (lb/ld)^{2/3}), \text{ from 10.3.5, ASCE 41-17.}$$

$$\text{with } fs1 = (fs, \text{jacket} * Asl, \text{ten, jacket} + fs, \text{core} * Asl, \text{ten, core}) / Asl, \text{ten} = 389.0139$$

$$\text{with } Es1 = (Es, \text{jacket} * Asl, \text{ten, jacket} + Es, \text{core} * Asl, \text{ten, core}) / Asl, \text{ten} = 200000.00$$

$$y2 = 0.00140044$$

$$sh2 = 0.0044814$$

$$ft2 = 466.8167$$

$$fy2 = 389.0139$$

$$su2 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$$lo/lou, \text{min} = lb/lb, \text{min} = 0.30$$

$$su2 = 0.4 * esu2_nominal \text{ ((5.5), TBDY)} = 0.032$$

$$\text{From table 5A.1, TBDY: } esu2_nominal = 0.08,$$

For calculation of esu2_nominal and y2, sh2, ft2, fy2, it is considered characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

$$y1, sh1, ft1, fy1, \text{ are also multiplied by } \text{Min}(1, 1.25 * (lb/ld)^{2/3}), \text{ from 10.3.5, ASCE 41-17.}$$

with $f_{s2} = (f_{s,jacket} \cdot A_{s,com,jacket} + f_{s,core} \cdot A_{s,com,core}) / A_{s,com} = 389.0139$
 with $E_{s2} = (E_{s,jacket} \cdot A_{s,com,jacket} + E_{s,core} \cdot A_{s,com,core}) / A_{s,com} = 200000.00$
 $y_v = 0.00140044$
 $sh_v = 0.0044814$
 $ft_v = 466.8167$
 $fy_v = 389.0139$
 $suv = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_o/l_{ou,min} = l_b/l_d = 0.30$
 $suv = 0.4 \cdot esuv_{nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_{nominal} = 0.08$,
 considering characteristic value $f_{syv} = f_{sv}/1.2$, from table 5.1, TBDY
 For calculation of $esuv_{nominal}$ and y_v, sh_v, ft_v, fy_v , it is considered
 characteristic value $f_{syv} = f_{sv}/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $f_{sv} = (f_{s,jacket} \cdot A_{s,mid,jacket} + f_{s,mid} \cdot A_{s,mid,core}) / A_{s,mid} = 389.0139$
 with $E_{sv} = (E_{s,jacket} \cdot A_{s,mid,jacket} + E_{s,mid} \cdot A_{s,mid,core}) / A_{s,mid} = 200000.00$
 $1 = A_{s,ten} / (b \cdot d) \cdot (f_{s1} / f_c) = 0.04740567$
 $2 = A_{s,com} / (b \cdot d) \cdot (f_{s2} / f_c) = 0.04740567$
 $v = A_{s,mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.01676222$

and confined core properties:

$b = 340.00$
 $d = 677.00$
 $d' = 13.00$
 $f_{cc} (5A.2, TBDY) = 34.43547$
 $cc (5A.5, TBDY) = 0.00243499$
 $c = \text{confinement factor} = 1.0435$
 $1 = A_{s,ten} / (b \cdot d) \cdot (f_{s1} / f_c) = 0.05824278$
 $2 = A_{s,com} / (b \cdot d) \cdot (f_{s2} / f_c) = 0.05824278$
 $v = A_{s,mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.02059413$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

$su (4.9) = 0.15682678$

$Mu = MRc (4.14) = 3.7261E+008$

$u = su (4.1) = 8.5888248E-006$

 Calculation of ratio l_b/l_d

 Inadequate Lap Length with $l_b/l_d = 0.30$

 Calculation of Mu_2 -

 Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 8.5888248E-006$

$Mu = 3.7261E+008$

 with full section properties:

$b = 400.00$

$d = 707.00$

$d' = 43.00$

$v = 0.00118049$

$N = 11016.808$

$f_c = 33.00$

$co (5A.5, TBDY) = 0.002$

Final value of cu : $cu^* = \text{shear_factor} \cdot \text{Max}(cu, cc) = 0.01006403$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $cu = 0.01006403$

$w_e ((5.4c), TBDY) = a_{se} \cdot s_{h,min} \cdot f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$
where $f = a_f \cdot p_f \cdot f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.02504965$
 $a_f = 0.38744444$
 $b = 750.00$
 $h = 400.00$
From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.00266667$
 $bw = 750.00$
effective stress from (A.35), $f_{f,e} = 800.0873$

$f_y = 0.04491794$
 $a_f = 0.38744444$
 $b = 400.00$
 $h = 750.00$
From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.005$
 $bw = 400.00$
effective stress from (A.35), $f_{f,e} = 765.1636$

$R = 40.00$
Effective FRP thickness, $t_f = NL \cdot t \cdot \text{Cos}(b_1) = 1.00$
 $f_{u,f} = 840.00$
 $E_f = 82000.00$
 $u_{,f} = 0.015$
 $a_{se} ((5.4d), TBDY) = (a_{se1} \cdot A_{ext} + a_{se2} \cdot A_{int})/A_{sec} = 0.12601038$
 $a_{se1} = 0.12601038$
 $bo_{,1} = 690.00$
 $ho_{,1} = 340.00$
 $bi_{2,1} = 1.1834E+006$
 $a_{se2} = \text{Max}(a_{se1}, a_{se2}) = 0.12601038$
 $bo_{,2} = 542.00$
 $ho_{,2} = 192.00$
 $bi_{2,2} = 661256.00$

$p_{sh,min} \cdot F_{ywe} = \text{Min}(p_{sh,x} \cdot F_{ywe}, p_{sh,y} \cdot F_{ywe}) = 1.64062$

Expression ((5.4d), TBDY) for $p_{sh,min} \cdot F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$p_{sh,x} \cdot F_{ywe} = p_{sh1} \cdot F_{ywe1} + p_{sh2} \cdot F_{ywe2} = 1.64062$
 $p_{s1} \text{ (external)} = (A_{sh1} \cdot h_1/s_1)/A_{sec} = 0.0020944$
 $A_{sh1} = A_{stir,1} \cdot n_{s,1} = 157.0796$
No stirrups, $n_{s,1} = 2.00$
 $h_1 = 400.00$
 $p_{s2} \text{ (internal)} = (A_{sh2} \cdot h_2/s_2)/A_{sec} = 0.00026808$
 $A_{sh2} = A_{stir,2} \cdot n_{s,2} = 100.531$
No stirrups, $n_{s,2} = 2.00$
 $h_2 = 200.00$

$p_{sh,y} \cdot F_{ywe} = p_{sh1} \cdot F_{ywe1} + p_{sh2} \cdot F_{ywe2} = 3.23907$
 $p_{s1} \text{ (external)} = (A_{sh1} \cdot h_1/s_1)/A_{sec} = 0.00392699$
 $A_{sh1} = A_{stir,1} \cdot n_{s,1} = 157.0796$
No stirrups, $n_{s,1} = 2.00$
 $h_1 = 750.00$
 $p_{s2} \text{ (internal)} = (A_{sh2} \cdot h_2/s_2)/A_{sec} = 0.00073723$
 $A_{sh2} = A_{stir,2} \cdot n_{s,2} = 100.531$
No stirrups, $n_{s,2} = 2.00$
 $h_2 = 550.00$

$A_{sec} = 300000.00$

$s_1 = 100.00$

$s_2 = 250.00$

$f_{ywe1} = 694.45$

$f_{ywe2} = 694.45$

$f_{ce} = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00243499$

$c = \text{confinement factor} = 1.0435$

$y_1 = 0.00140044$

$sh_1 = 0.0044814$

$$ft1 = 466.8167$$

$$fy1 = 389.0139$$

$$su1 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lou,min = lb/d = 0.30$$

$$su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25*(lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139$$

$$\text{with } Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00$$

$$y2 = 0.00140044$$

$$sh2 = 0.0044814$$

$$ft2 = 466.8167$$

$$fy2 = 389.0139$$

$$su2 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lou,min = lb/lb,min = 0.30$$

$$su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25*(lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139$$

$$\text{with } Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00$$

$$yv = 0.00140044$$

$$shv = 0.0044814$$

$$ftv = 466.8167$$

$$fyv = 389.0139$$

$$suv = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lou,min = lb/d = 0.30$$

$$suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25*(lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139$$

$$\text{with } Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00$$

$$1 = Asl,ten/(b*d)*(fs1/fc) = 0.04740567$$

$$2 = Asl,com/(b*d)*(fs2/fc) = 0.04740567$$

$$v = Asl,mid/(b*d)*(fsv/fc) = 0.01676222$$

and confined core properties:

$$b = 340.00$$

$$d = 677.00$$

$$d' = 13.00$$

$$fcc (5A.2, TBDY) = 34.43547$$

$$cc (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = Asl,ten/(b*d)*(fs1/fc) = 0.05824278$$

$$2 = Asl,com/(b*d)*(fs2/fc) = 0.05824278$$

$$v = Asl,mid/(b*d)*(fsv/fc) = 0.02059413$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

v < vs,y2 - LHS eq.(4.5) is satisfied

--->

$$su (4.9) = 0.15682678$$

$$Mu = MRc (4.14) = 3.7261E+008$$

$$u = s_u(4.1) = 8.5888248E-006$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 1.2627E+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 1.2627E+006$

$V_{r1} = V_{Col}((10.3), \text{ASCE } 41-17) = k_{nl} * V_{Col0}$

$$V_{Col0} = 1.2627E+006$$

$k_{nl} = 1$ (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v * f_y * d / s$ ' is replaced by ' $V_s + f * V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

$= 1$ (normal-weight concrete)

Mean concrete strength: $f_c' = (f_c'_{jacket} * \text{Area}_{jacket} + f_c'_{core} * \text{Area}_{core}) / \text{Area}_{section} = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$$M/Vd = 2.00$$

$$M_u = 2.3795529E-012$$

$$V_u = 2.9582284E-031$$

$$d = 0.8 * h = 600.00$$

$$N_u = 11016.808$$

$$A_g = 300000.00$$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 621900.694$

where:

$V_{s1} = 523602.964$ is calculated for jacket, with:

$$d = 600.00$$

$$A_v = 157079.633$$

$$f_y = 555.56$$

$$s = 100.00$$

V_{s1} is multiplied by $Col1 = 1.00$

$$s/d = 0.16666667$$

$V_{s2} = 98297.73$ is calculated for core, with:

$$d = 440.00$$

$$A_v = 100530.965$$

$$f_y = 555.56$$

$$s = 250.00$$

V_{s2} is multiplied by $Col2 = 1.00$

$$s/d = 0.56818182$$

V_f ((11-3)-(11.4), ACI 440) = 463792.00

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin \theta + \cos \theta$ is replaced with $(\cot \theta + \csc \theta) \sin \alpha$ which is more a generalised expression, where θ is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, \alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = 45^\circ + 90^\circ = 135^\circ$

$V_f = \text{Min}(|V_f(45, 135)|, |V_f(-45, 135)|)$, with:

total thickness per orientation, $t_{f1} = N_L * t / N_{oDir} = 1.00$

$d_{fv} = d$ (figure 11.2, ACI 440) = 707.00

f_{fe} ((11-5), ACI 440) = 328.00

$$E_f = 82000.00$$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.009$

From (11-11), ACI 440: $V_s + V_f \leq 915872.391$

$$b_w = 400.00$$

Calculation of Shear Strength at edge 2, $V_{r2} = 1.2627E+006$

$V_{r2} = V_{Col}((10.3), \text{ASCE } 41-17) = k_{nl} * V_{Col0}$

$$V_{Col0} = 1.2627E+006$$

knl = 1 (zero step-static loading)

NOTE: In expression (10-3) $V_s = A_v \cdot f_y \cdot d / s$ is replaced by $V_s + f \cdot V_f$ where V_f is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: $f_c' = (f_c'_{\text{jacket}} \cdot \text{Area}_{\text{jacket}} + f_c'_{\text{core}} \cdot \text{Area}_{\text{core}}) / \text{Area}_{\text{section}} = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$M/Vd = 2.00$

$\mu_u = 2.3795529E-012$

$\nu_u = 2.9582284E-031$

$d = 0.8 \cdot h = 600.00$

$N_u = 11016.808$

$A_g = 300000.00$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 621900.694$

where:

$V_{s1} = 523602.964$ is calculated for jacket, with:

$d = 600.00$

$A_v = 157079.633$

$f_y = 555.56$

$s = 100.00$

V_{s1} is multiplied by $\text{Col1} = 1.00$

$s/d = 0.16666667$

$V_{s2} = 98297.73$ is calculated for core, with:

$d = 440.00$

$A_v = 100530.965$

$f_y = 555.56$

$s = 250.00$

V_{s2} is multiplied by $\text{Col2} = 1.00$

$s/d = 0.56818182$

V_f ((11-3)-(11.4), ACI 440) = 463792.00

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot_a) \sin a$ which is more a generalised expression, where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, a)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = \theta_1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta_1)|, |V_f(-45, \theta_1)|)$, with:

total thickness per orientation, $t_{f1} = N_L \cdot t / N_{\text{Dir}} = 1.00$

$d_{fv} = d$ (figure 11.2, ACI 440) = 707.00

f_{fe} ((11-5), ACI 440) = 328.00

$E_f = 82000.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.009$

From (11-11), ACI 440: $V_s + V_f \leq 915872.391$

$b_w = 400.00$

End Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At local axis: 2

Start Of Calculation of Chord Rotation Capacity for element: column JC1 of floor 1

At local axis: 2

Integration Section: (a)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$

Chord Rotation is generally considered as Deformation-Controlled Action according to Table C7-1, ASCE 41-17.

Mean strengths are used for Deformation-Controlled Actions according to 7.5.1.3, ASCE 41-17

Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$
New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$
Concrete Elasticity, $E_c = 26999.444$
Steel Elasticity, $E_s = 200000.00$
Existing Column
New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$
New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$
Concrete Elasticity, $E_c = 26999.444$
Steel Elasticity, $E_s = 200000.00$
External Height, $H = 400.00$
External Width, $W = 750.00$
Internal Height, $H = 200.00$
Internal Width, $W = 550.00$
Cover Thickness, $c = 25.00$
Element Length, $L = 3000.00$
Primary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Inadequate Lap Length with $l_b/l_d = 0.30$
FRP Wrapping Data
Type: Carbon
Dry properties (design values)
Thickness, $t = 1.00$
Tensile Strength, $f_{fu} = 840.00$
Tensile Modulus, $E_f = 82000.00$
Elongation, $e_{fu} = 0.009$
Number of directions, $N_{oDir} = 1$
Fiber orientations, $b_i: 0.00^\circ$
Number of layers, $N_L = 1$
Radius of rounding corners, $R = 40.00$

Stepwise Properties

Bending Moment, $M = 4.7677845E-010$
Shear Force, $V_2 = -1995.365$
Shear Force, $V_3 = -1.4564234E-013$
Axial Force, $F = -14177.47$
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: $A_{sl} = 0.00$
-Compression: $A_{slc} = 2676.637$
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: $A_{sl,ten} = 1137.257$
-Compression: $A_{sl,com} = 1137.257$
-Middle: $A_{sl,mid} = 402.1239$
Longitudinal External Reinforcement Area Distribution (in 3 divisions)
-Tension: $A_{sl,ten,jacket} = 829.3805$
-Compression: $A_{sl,com,jacket} = 829.3805$
-Middle: $A_{sl,mid,jacket} = 402.1239$
Longitudinal Internal Reinforcement Area Distribution (in 3 divisions)
-Tension: $A_{sl,ten,core} = 307.8761$
-Compression: $A_{sl,com,core} = 307.8761$
-Middle: $A_{sl,mid,core} = 0.00$
Mean Diameter of Tension Reinforcement, $Db_L = 16.80$

New component: From table 7-7, ASCE 41_17: Final chord rotation Capacity $u_{,R} = 1.0^*$ $u = 0.00203008$
 $u = y + p = 0.00203008$

- Calculation of y -

$$y = (M_y * L_s / 3) / E_{eff} = 0.00203008 \text{ ((4.29), Biskinis Phd)}$$

$M_y = 1.3155E+008$
 $L_s = M/V$ (with $L_s > 0.1*L$ and $L_s < 2*L$) = 1500.00
 From table 10.5, ASCE 41_17: $E_{eff} = factor * E_c * I_g = 3.2399E+013$
 $factor = 0.30$
 $A_g = 300000.00$
 Mean concrete strength: $f_c' = (f_c'_{jacket} * Area_{jacket} + f_c'_{core} * Area_{core}) / Area_{section} = 33.00$
 $N = 14177.47$
 $E_c * I_g = E_{c,jacket} * I_{g,jacket} + E_{c,core} * I_{g,core} = 1.0800E+014$

Calculation of Yielding Moment M_y

Calculation of y and M_y according to Annex 7 -

$y = \text{Min}(y_{ten}, y_{com})$
 $y_{ten} = 5.6359183E-006$
 with ((10.1), ASCE 41-17) $f_y = \text{Min}(f_y, 1.25 * f_y * (l_b/d)^{2/3}) = 311.2112$
 $d = 357.00$
 $y = 0.22662123$
 $A = 0.01016692$
 $B = 0.00577058$
 with $pt = 0.00424746$
 $pc = 0.00424746$
 $pv = 0.00150186$
 $N = 14177.47$
 $b = 750.00$
 $" = 0.12044818$
 $y_{comp} = 2.7804373E-005$
 with f_c^* (12.3, (ACI 440)) = 33.35499
 $f_c = 33.00$
 $f_l = 0.77176471$
 $b = 750.00$
 $h = 400.00$
 $A_g = 300000.00$
 From (12.9), ACI 440: $k_a = 0.14671972$
 $g = pt + pc + pv = 0.00999678$
 $rc = 40.00$
 $A_e/A_c = 0.51581153$
 Effective FRP thickness, $t_f = NL * t * \text{Cos}(b_1) = 1.00$
 effective strain from (12.5) and (12.12), $e_{fe} = 0.004$
 $f_u = 0.009$
 $E_f = 82000.00$
 $E_c = 26999.444$
 $y = 0.22402532$
 $A = 0.00987772$
 $B = 0.00560044$
 with $E_s = 200000.00$

Calculation of ratio l_b/d

Inadequate Lap Length with $l_b/d = 0.30$

- Calculation of p -

From table 10-8: $p = 0.00$

with:

- Columns controlled by inadequate development or splicing along the clear height because $l_b/d < 1$
 shear control ratio $V_y E / C o I O E = 0.14265244$
 $d = d_{external} = 357.00$
 $s = s_{external} = 0.00$
 $t = s_1 + s_2 + 2 * t_f / b_w * (f_{fe} / f_s) = 0.00393687$
 jacket: $s_1 = A_v1 * h_1 / (s_1 * A_g) = 0.0020944$
 $A_v1 = 157.0796$, is the total area of all stirrups parallel to loading (shear) direction

$$h1 = 400.00$$

$$s1 = 100.00$$

$$\text{core: } s2 = Av2 \cdot h2 / (s2 \cdot Ag) = 0.00026808$$

$Av2 = 100.531$, is the total area of all stirrups parallel to loading (shear) direction

$$h2 = 200.00$$

$$s2 = 250.00$$

The term $2 \cdot tf/bw \cdot (ffe/fs)$ is implemented to account for FRP contribution where $f = 2 \cdot tf/bw$ is FRP ratio (EC8 - 3, A.4.4.3(6)) and ffe/fs normalises f to steel strength. All these variables have already been given in Shear control ratio calculation.

For the normalisation fs of jacket is used.

$$NUD = 14177.47$$

$$Ag = 300000.00$$

$$fcE = (fc_jacket \cdot Area_jacket + fc_core \cdot Area_core) / section_area = 33.00$$

$$fyIE = (fy_ext_Long_Reinf \cdot Area_ext_Long_Reinf + fy_int_Long_Reinf \cdot Area_int_Long_Reinf) / Area_Tot_Long_Rein = 555.56$$

$$fytE = (fy_ext_Trans_Reinf \cdot s1 + fy_int_Trans_Reinf \cdot s2) / (s1 + s2) = 555.56$$

$$pl = Area_Tot_Long_Rein / (b \cdot d) = 0.00999678$$

$$b = 750.00$$

$$d = 357.00$$

$$fcE = 33.00$$

End Of Calculation of Chord Rotation Capacity for element: column JC1 of floor 1

At local axis: 2

Integration Section: (a)

Calculation No. 3

column C1, Floor 1

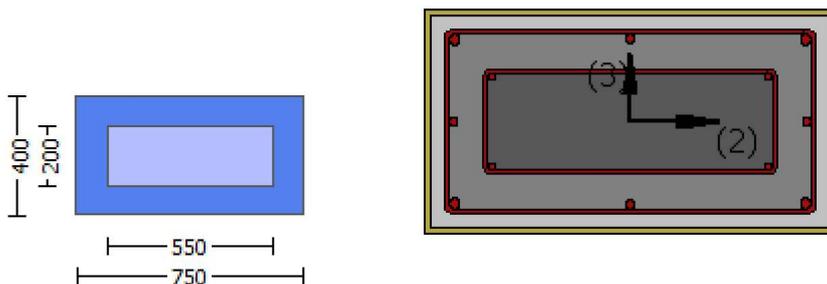
Limit State: Immediate Occupancy (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Shear capacity VRd

Edge: Start

Local Axis: (3)



```

Start Of Calculation of Shear Capacity for element: column JC1 of floor 1
At local axis: 3
Integration Section: (a)
Section Type: rcjrs

Constant Properties
-----
Knowledge Factor,   = 0.90
Member Shear Force is generally considered as Force-Controlled Action according to Table C7-1, ASCE 41-17.
Lower-bound strengths are used for Force-Controlled Actions according to 7.5.1.3, ASCE 41-17
Consequently:
Jacket
New material of Primary Member: Concrete Strength, fc = fc_lower_bound = 25.00
New material of Primary Member: Steel Strength, fs = fs_lower_bound = 500.00
Concrete Elasticity, Ec = 26999.444
Steel Elasticity, Es = 200000.00
Existing Column
New material of Primary Member: Concrete Strength, fc = fc_lower_bound = 25.00
New material of Primary Member: Steel Strength, fs = fs_lower_bound = 500.00
Concrete Elasticity, Ec = 26999.444
Steel Elasticity, Es = 200000.00
#####
Note: Especially for the calculation of  $\gamma$  for displacement ductility demand,
the expected (mean value) strengths are used (7.5.1.3, ASCE 41-17) because bending is considered as
Deformation-Controlled Action (Table C7-1, ASCE 41-17).
Jacket
New material: Concrete Strength, fc = fcm = 33.00
New material: Steel Strength, fs = fsm = 555.56
Existing Column
New material: Concrete Strength, fc = fcm = 33.00
New material: Steel Strength, fs = fsm = 555.56
#####
External Height, H = 400.00
External Width, W = 750.00
Internal Height, H = 200.00
Internal Width, W = 550.00
Cover Thickness, c = 25.00
Element Length, L = 3000.00
Primary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Inadequate Lap Length with lo/lo,min = lb/d = 0.30
FRP Wrapping Data
Type: Carbon
Dry properties (design values)
Thickness, t = 1.00
Tensile Strength, ffu = 840.00
Tensile Modulus, Ef = 82000.00
Elongation, efu = 0.009
Number of directions, NoDir = 1
Fiber orientations, bi: 0.00°
Number of layers, NL = 1
Radius of rounding corners, R = 40.00
-----

Stepwise Properties
-----
EDGE -A-
Bending Moment, Ma = 4.7677845E-010
Shear Force, Va = -1.4564234E-013
EDGE -B-
Bending Moment, Mb = -3.8976464E-011
Shear Force, Vb = 1.4564234E-013
BOTH EDGES
Axial Force, F = -14177.47

```

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_{t,ten} = 0.00$

-Compression: $As_{c,com} = 2676.637$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{t,ten} = 1137.257$

-Compression: $As_{c,com} = 1137.257$

-Middle: $As_{c,mid} = 402.1239$

Mean Diameter of Tension Reinforcement, $Db_{L,ten} = 16.80$

New component: From table 7-7, ASCE 41-17: Final Shear Capacity $V_R = 1.0 \cdot V_n = 776632.031$

V_n ((10.3), ASCE 41-17) = $k_n \cdot V_{CoI} = 776632.031$

$V_{CoI} = 776632.031$

$k_n = 1.00$

displacement_ductility_demand = 0.00

NOTE: In expression (10-3) ' $V_s = A_v \cdot f_y \cdot d / s$ ' is replaced by ' $V_s + f \cdot V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: $f_c' = (f_c'_{jacket} \cdot Area_{jacket} + f_c'_{core} \cdot Area_{core}) / Area_{section} = 25.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$M/Vd = 2.00$

$\mu_u = 4.7677845E-010$

$\nu_u = 1.4564234E-013$

$d = 0.8 \cdot h = 320.00$

$N_u = 14177.47$

$A_g = 300000.00$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 251327.412$

where:

$V_{s1} = 251327.412$ is calculated for jacket, with:

$d = 320.00$

$A_v = 157079.633$

$f_y = 500.00$

$s = 100.00$

V_{s1} is multiplied by $Col1 = 1.00$

$s/d = 0.3125$

$V_{s2} = 0.00$ is calculated for core, with:

$d = 160.00$

$A_v = 100530.965$

$f_y = 500.00$

$s = 250.00$

V_{s2} is multiplied by $Col2 = 0.00$

$s/d = 1.5625$

V_f ((11-3)-(11.4), ACI 440) = 234192.00

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin \theta + \cos \theta$ is replaced with $(\cot \theta + \csc \theta) \sin \alpha$ which is more a generalised expression,

where θ is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, \alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = 45^\circ + 90^\circ = 135^\circ$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, \theta)|)$, with:

total thickness per orientation, $t_{f1} = N_L \cdot t / N_{oDir} = 1.00$

$d_{fv} = d$ (figure 11.2, ACI 440) = 357.00

f_{fe} ((11-5), ACI 440) = 328.00

$E_f = 82000.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.009$

From (11-11), ACI 440: $V_s + V_f \leq 797164.595$

$b_w = 750.00$

displacement_ductility_demand is calculated as δ / y

- Calculation of ϕ / γ for END A -
for rotation axis 2 and integ. section (a)

From analysis, chord rotation = 6.4462528E-021

$y = (M_y * L_s / 3) / E_{eff} = 0.00203008$ ((4.29), Biskinis Phd))

$M_y = 1.3155E+008$

$L_s = M/V$ (with $L_s > 0.1 * L$ and $L_s < 2 * L$) = 1500.00

From table 10.5, ASCE 41_17: $E_{eff} = factor * E_c * I_g = 3.2399E+013$

factor = 0.30

$A_g = 300000.00$

Mean concrete strength: $f_c' = (f_c'_{jacket} * Area_{jacket} + f_c'_{core} * Area_{core}) / Area_{section} = 33.00$

$N = 14177.47$

$E_c * I_g = E_c_{jacket} * I_{g,jacket} + E_c_{core} * I_{g,core} = 1.0800E+014$

Calculation of Yielding Moment M_y

Calculation of ϕ / γ and M_y according to Annex 7 -

$y = \text{Min}(y_{ten}, y_{com})$

$y_{ten} = 5.6359183E-006$

with ((10.1), ASCE 41-17) $f_y = \text{Min}(f_y, 1.25 * f_y * (l_b / l_d)^{2/3}) = 311.2112$

$d = 357.00$

$y = 0.22662123$

$A = 0.01016692$

$B = 0.00577058$

with $p_t = 0.00424746$

$p_c = 0.00424746$

$p_v = 0.00150186$

$N = 14177.47$

$b = 750.00$

" = 0.12044818

$y_{comp} = 2.7804373E-005$

with $f_c' (12.3, (ACI 440)) = 33.35499$

$f_c = 33.00$

$f_l = 0.77176471$

$b = 750.00$

$h = 400.00$

$A_g = 300000.00$

From (12.9), ACI 440: $k_a = 0.14671972$

$g = p_t + p_c + p_v = 0.00999678$

$r_c = 40.00$

$A_e / A_c = 0.51581153$

Effective FRP thickness, $t_f = N L * t * \text{Cos}(\theta_1) = 1.00$

effective strain from (12.5) and (12.12), $\epsilon_{fe} = 0.004$

$f_u = 0.009$

$E_f = 82000.00$

$E_c = 26999.444$

$y = 0.22402532$

$A = 0.00987772$

$B = 0.00560044$

with $E_s = 200000.00$

Calculation of ratio l_b / l_d

Inadequate Lap Length with $l_b / l_d = 0.30$

End Of Calculation of Shear Capacity for element: column JC1 of floor 1

At local axis: 3

Integration Section: (a)

Calculation No. 4

column C1, Floor 1

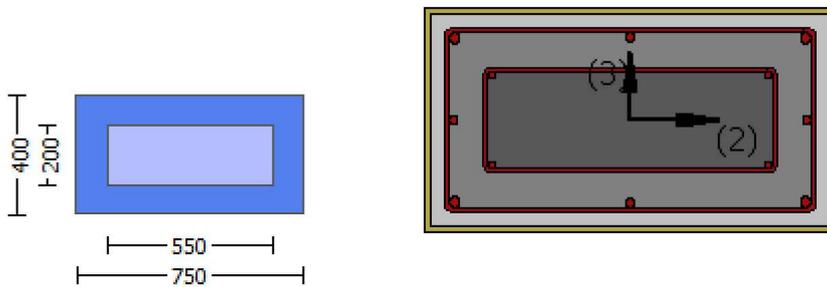
Limit State: Immediate Occupancy (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Chord rotation capacity (θ)

Edge: Start

Local Axis: (3)



Start Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At Shear local axis: 3

(Bending local axis: 2)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\phi = 0.90$

Mean strength values are used for both shear and moment calculations.

Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Existing Column

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of moment strengths,

the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

Jacket

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

Existing Column

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

#####

External Height, $H = 400.00$

External Width, $W = 750.00$

Internal Height, H = 200.00
Internal Width, W = 550.00
Cover Thickness, c = 25.00
Mean Confinement Factor overall section = 1.0435
Element Length, L = 3000.00
Primary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Inadequate Lap Length with $l_o/l_{o,min}$ = 0.30
FRP Wrapping Data
Type: Carbon
Dry properties (design values)
Thickness, t = 1.00
Tensile Strength, f_{fu} = 840.00
Tensile Modulus, E_f = 82000.00
Elongation, $ε_{fu}$ = 0.009
Number of directions, NoDir = 1
Fiber orientations, $β_i$: 0.00°
Number of layers, NL = 1
Radius of rounding corners, R = 40.00

Stepwise Properties

At local axis: 3
EDGE -A-
Shear Force, V_a = 3.4586346E-031
EDGE -B-
Shear Force, V_b = -3.4586346E-031
BOTH EDGES
Axial Force, F = -11016.808
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: A_{st} = 0.00
-Compression: A_{sc} = 2676.637
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: $A_{st,ten}$ = 1137.257
-Compression: $A_{st,com}$ = 1137.257
-Middle: $A_{st,mid}$ = 402.1239

Calculation of Shear Capacity ratio, V_e/V_r = 0.14265244

Member Controlled by Flexure ($V_e/V_r < 1$)

Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 121055.918$
with

$M_{pr1} = \text{Max}(M_{u1+}, M_{u1-}) = 1.8158E+008$

$M_{u1+} = 1.8158E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination

$M_{u1-} = 1.8158E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination

$M_{pr2} = \text{Max}(M_{u2+}, M_{u2-}) = 1.8158E+008$

$M_{u2+} = 1.8158E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the the static loading combination

$M_{u2-} = 1.8158E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the the static loading combination

Calculation of M_{u1+}

Calculation of ultimate curvature $ϕ_u$ according to 4.1, Biskinis/Fardis 2013:

$ϕ_u = 1.7255512E-005$

$M_u = 1.8158E+008$

with full section properties:

$$b = 750.00$$

$$d = 357.00$$

$$d' = 43.00$$

$$v = 0.00124684$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$\alpha (5A.5, TBDY) = 0.002$$

$$\text{Final value of } \alpha: \alpha^* = \text{shear_factor} * \text{Max}(\alpha, \alpha_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \alpha = 0.01006403$$

$$\omega ((5.4c), TBDY) = \alpha^* \cdot \text{sh}_{\min} \cdot f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$$

where $f = \alpha^* \cdot p_f \cdot f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.02504965$$

$$\alpha_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/bw = 0.00266667$$

$$bw = 750.00$$

$$\text{effective stress from (A.35), } f_{fe} = 800.0873$$

$$f_y = 0.04491794$$

$$\alpha_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/bw = 0.005$$

$$bw = 400.00$$

$$\text{effective stress from (A.35), } f_{fe} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL \cdot t \cdot \text{Cos}(b_1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{,f} = 0.015$$

$$\alpha_{se} ((5.4d), TBDY) = (\alpha_{se1} \cdot A_{ext} + \alpha_{se2} \cdot A_{int}) / A_{sec} = 0.12601038$$

$$\alpha_{se1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i2,1} = 1.1834E+006$$

$$\alpha_{se2} = \text{Max}(\alpha_{se1}, \alpha_{se2}) = 0.12601038$$

$$b_{o,2} = 542.00$$

$$h_{o,2} = 192.00$$

$$b_{i2,2} = 661256.00$$

$$p_{sh,\min} \cdot F_{ywe} = \text{Min}(p_{sh,x} \cdot F_{ywe}, p_{sh,y} \cdot F_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $p_{sh,\min} \cdot F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$p_{sh,x} \cdot F_{ywe} = p_{sh1} \cdot F_{ywe1} + p_{sh2} \cdot F_{ywe2} = 1.64062$$

$$p_{s1} \text{ (external)} = (A_{sh1} \cdot h_1 / s_1) / A_{sec} = 0.0020944$$

$$A_{sh1} = A_{stir,1} \cdot n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

$$h_1 = 400.00$$

$$p_{s2} \text{ (internal)} = (A_{sh2} \cdot h_2 / s_2) / A_{sec} = 0.00026808$$

$$A_{sh2} = A_{stir,2} \cdot n_{s,2} = 100.531$$

$$\text{No stirrups, } n_{s,2} = 2.00$$

$$h_2 = 200.00$$

$$p_{sh,y} \cdot F_{ywe} = p_{sh1} \cdot F_{ywe1} + p_{sh2} \cdot F_{ywe2} = 3.23907$$

$$p_{s1} \text{ (external)} = (A_{sh1} \cdot h_1 / s_1) / A_{sec} = 0.00392699$$

$$A_{sh1} = A_{stir,1} \cdot n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

$$h_1 = 750.00$$

$$p_{s2} \text{ (internal)} = (A_{sh2} \cdot h_2 / s_2) / A_{sec} = 0.00073723$$

$$A_{sh2} = A_{stir,2} \cdot n_{s,2} = 100.531$$

$$\text{No stirrups, } n_{s,2} = 2.00$$

h2 = 550.00

Asec = 300000.00

s1 = 100.00

s2 = 250.00

fywe1 = 694.45

fywe2 = 694.45

fce = 33.00

From ((5.A.5), TBDY), TBDY: cc = 0.00243499

c = confinement factor = 1.0435

y1 = 0.00140044

sh1 = 0.0044814

ft1 = 466.8167

fy1 = 389.0139

su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^2/3), from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044

sh2 = 0.0044814

ft2 = 466.8167

fy2 = 389.0139

su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^2/3), from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044

shv = 0.0044814

ftv = 466.8167

fyv = 389.0139

suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^2/3), from 10.3.5, ASCE 41-17.

with fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.0500703

2 = Asl,com/(b*d)*(fs2/fc) = 0.0500703

v = Asl,mid/(b*d)*(fsv/fc) = 0.01770442

and confined core properties:

b = 690.00

d = 327.00

d' = 13.00

fcc (5A.2, TBDY) = 34.43547

$$cc \text{ (5A.5, TBDY)} = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = \text{Asl,ten}/(b*d)*(fs1/fc) = 0.05941729$$

$$2 = \text{Asl,com}/(b*d)*(fs2/fc) = 0.05941729$$

$$v = \text{Asl,mid}/(b*d)*(fsv/fc) = 0.02100943$$

Case/Assumption: Unconfined full section - Steel rupture
' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$su \text{ (4.9)} = 0.16886055$$

$$\text{Mu} = \text{MRc (4.14)} = 1.8158\text{E}+008$$

$$u = su \text{ (4.1)} = 1.7255512\text{E}-005$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of Mu_1 -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$$u = 1.7255512\text{E}-005$$

$$\text{Mu} = 1.8158\text{E}+008$$

with full section properties:

$$b = 750.00$$

$$d = 357.00$$

$$d' = 43.00$$

$$v = 0.00124684$$

$$N = 11016.808$$

$$fc = 33.00$$

$$co \text{ (5A.5, TBDY)} = 0.002$$

$$\text{Final value of } cu: cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } cu = 0.01006403$$

$$we \text{ ((5.4c), TBDY)} = ase * sh_{,min} * fy_{we}/f_{ce} + \text{Min}(fx, fy) = 0.02692906$$

where $f = af * pf * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$fx = 0.02504965$$

$$af = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } pf = 2tf/bw = 0.00266667$$

$$bw = 750.00$$

$$\text{effective stress from (A.35), } ff,e = 800.0873$$

$$fy = 0.04491794$$

$$af = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } pf = 2tf/bw = 0.005$$

$$bw = 400.00$$

$$\text{effective stress from (A.35), } ff,e = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } tf = NL * t * \text{Cos}(b1) = 1.00$$

$$fu,f = 840.00$$

$$Ef = 82000.00$$

$$u,f = 0.015$$

$$\text{ase ((5.4d), TBDY)} = (ase1 * A_{ext} + ase2 * A_{int}) / A_{sec} = 0.12601038$$

$$ase1 = 0.12601038$$

$$bo_1 = 690.00$$

ho_1 = 340.00
bi2_1 = 1.1834E+006
ase2 = Max(ase1,ase2) = 0.12601038
bo_2 = 542.00
ho_2 = 192.00
bi2_2 = 661256.00

psh,min*Fywe = Min(psh,x*Fywe , psh,y*Fywe) = 1.64062

Expression ((5.4d), TBDY) for psh,min*Fywe has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh_x*Fywe = psh1*Fywe1+ps2*Fywe2 = 1.64062
ps1 (external) = (Ash1*h1/s1)/Asec = 0.0020944
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 400.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00026808
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 200.00

psh_y*Fywe = psh1*Fywe1+ps2*Fywe2 = 3.23907
ps1 (external) = (Ash1*h1/s1)/Asec = 0.00392699
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 750.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00073723
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/d = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^ 2/3), from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044
sh2 = 0.0044814
ft2 = 466.8167
fy2 = 389.0139
su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs_2 = (fs_{jacket} \cdot A_{s1,com,jacket} + fs_{core} \cdot A_{s1,com,core}) / A_{s1,com} = 389.0139$

with $Es_2 = (Es_{jacket} \cdot A_{s1,com,jacket} + Es_{core} \cdot A_{s1,com,core}) / A_{s1,com} = 200000.00$

$y_v = 0.00140044$

$sh_v = 0.0044814$

$ft_v = 466.8167$

$fy_v = 389.0139$

$suv = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

$l_o/l_{o,min} = l_b/l_d = 0.30$

$suv = 0.4 \cdot esuv_{nominal} ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esuv_{nominal} = 0.08$,

considering characteristic value $fs_{yv} = f_{sv}/1.2$, from table 5.1, TBDY

For calculation of $esuv_{nominal}$ and y_v, sh_v, ft_v, fy_v , it is considered

characteristic value $fs_{yv} = f_{sv}/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $f_{sv} = (f_{s,jacket} \cdot A_{s,mid,jacket} + f_{s,mid} \cdot A_{s,mid,core}) / A_{s,mid} = 389.0139$

with $Es_v = (E_{s,jacket} \cdot A_{s,mid,jacket} + E_{s,mid} \cdot A_{s,mid,core}) / A_{s,mid} = 200000.00$

$1 = A_{s,ten} / (b \cdot d) \cdot (f_{s1} / f_c) = 0.0500703$

$2 = A_{s,com} / (b \cdot d) \cdot (f_{s2} / f_c) = 0.0500703$

$v = A_{s,mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.01770442$

and confined core properties:

$b = 690.00$

$d = 327.00$

$d' = 13.00$

$f_{cc} (5A.2, TBDY) = 34.43547$

$cc (5A.5, TBDY) = 0.00243499$

$c = \text{confinement factor} = 1.0435$

$1 = A_{s,ten} / (b \cdot d) \cdot (f_{s1} / f_c) = 0.05941729$

$2 = A_{s,com} / (b \cdot d) \cdot (f_{s2} / f_c) = 0.05941729$

$v = A_{s,mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.02100943$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$su (4.9) = 0.16886055$

$Mu = MRc (4.14) = 1.8158E+008$

$u = su (4.1) = 1.7255512E-005$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of Mu_{2+}

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 1.7255512E-005$

$Mu = 1.8158E+008$

with full section properties:

$b = 750.00$

$d = 357.00$

$d' = 43.00$

$v = 0.00124684$

$N = 11016.808$

$f_c = 33.00$

$cc (5A.5, TBDY) = 0.002$

Final value of cu : $cu^* = \text{shear_factor} \cdot \text{Max}(cu, cc) = 0.01006403$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $cu = 0.01006403$

w_e ((5.4c), TBDY) = $ase * sh_{min} * fy_{we} / f_{ce} + Min(f_x, f_y) = 0.02692906$

where $f = af * pf * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.02504965$

$af = 0.38744444$

$b = 750.00$

$h = 400.00$

From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.00266667$

$bw = 750.00$

effective stress from (A.35), $ff_e = 800.0873$

$f_y = 0.04491794$

$af = 0.38744444$

$b = 400.00$

$h = 750.00$

From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.005$

$bw = 400.00$

effective stress from (A.35), $ff_e = 765.1636$

$R = 40.00$

Effective FRP thickness, $tf = NL * t * Cos(b_1) = 1.00$

$f_{u,f} = 840.00$

$E_f = 82000.00$

$u_{,f} = 0.015$

ase ((5.4d), TBDY) = $(ase_1 * A_{ext} + ase_2 * A_{int}) / A_{sec} = 0.12601038$

$ase_1 = 0.12601038$

$bo_1 = 690.00$

$ho_1 = 340.00$

$bi_{2,1} = 1.1834E+006$

$ase_2 = Max(ase_1, ase_2) = 0.12601038$

$bo_2 = 542.00$

$ho_2 = 192.00$

$bi_{2,2} = 661256.00$

$psh_{min} * Fy_{we} = Min(psh_x * Fy_{we}, psh_y * Fy_{we}) = 1.64062$

Expression ((5.4d), TBDY) for $psh_{min} * Fy_{we}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh_x * Fy_{we} = psh_1 * Fy_{we1} + ps_2 * Fy_{we2} = 1.64062$

ps_1 (external) = $(Ash_1 * h_1 / s_1) / A_{sec} = 0.0020944$

$Ash_1 = Astir_1 * ns_1 = 157.0796$

No stirrups, $ns_1 = 2.00$

$h_1 = 400.00$

ps_2 (internal) = $(Ash_2 * h_2 / s_2) / A_{sec} = 0.00026808$

$Ash_2 = Astir_2 * ns_2 = 100.531$

No stirrups, $ns_2 = 2.00$

$h_2 = 200.00$

$psh_y * Fy_{we} = psh_1 * Fy_{we1} + ps_2 * Fy_{we2} = 3.23907$

ps_1 (external) = $(Ash_1 * h_1 / s_1) / A_{sec} = 0.00392699$

$Ash_1 = Astir_1 * ns_1 = 157.0796$

No stirrups, $ns_1 = 2.00$

$h_1 = 750.00$

ps_2 (internal) = $(Ash_2 * h_2 / s_2) / A_{sec} = 0.00073723$

$Ash_2 = Astir_2 * ns_2 = 100.531$

No stirrups, $ns_2 = 2.00$

$h_2 = 550.00$

$A_{sec} = 300000.00$

$s_1 = 100.00$

$s_2 = 250.00$

$fy_{we1} = 694.45$

$fy_{we2} = 694.45$

$f_{ce} = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00243499$

$c =$ confinement factor = 1.0435

$y_1 = 0.00140044$

$sh1 = 0.0044814$
 $ft1 = 466.8167$
 $fy1 = 389.0139$
 $su1 = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou,min = lb/ld = 0.30$
 $su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu1_nominal = 0.08$,
 For calculation of $esu1_nominal$ and $y1, sh1,ft1,fy1$, it is considered
 characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.
 $y1, sh1,ft1,fy1$, are also multiplied by $Min(1,1.25*(lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139$
 with $Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00$
 $y2 = 0.00140044$
 $sh2 = 0.0044814$
 $ft2 = 466.8167$
 $fy2 = 389.0139$
 $su2 = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou,min = lb/lb,min = 0.30$
 $su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu2_nominal = 0.08$,
 For calculation of $esu2_nominal$ and $y2, sh2,ft2,fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.
 $y1, sh1,ft1,fy1$, are also multiplied by $Min(1,1.25*(lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139$
 with $Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00$
 $yv = 0.00140044$
 $shv = 0.0044814$
 $ftv = 466.8167$
 $fyv = 389.0139$
 $suv = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou,min = lb/ld = 0.30$
 $suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_nominal = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv, shv,ftv,fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 $y1, sh1,ft1,fy1$, are also multiplied by $Min(1,1.25*(lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139$
 with $Es_v = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00$
 $1 = Asl,ten/(b*d)*(fs1/fc) = 0.0500703$
 $2 = Asl,com/(b*d)*(fs2/fc) = 0.0500703$
 $v = Asl,mid/(b*d)*(fsv/fc) = 0.01770442$
 and confined core properties:
 $b = 690.00$
 $d = 327.00$
 $d' = 13.00$
 $fcc (5A.2, TBDY) = 34.43547$
 $cc (5A.5, TBDY) = 0.00243499$
 $c = \text{confinement factor} = 1.0435$
 $1 = Asl,ten/(b*d)*(fs1/fc) = 0.05941729$
 $2 = Asl,com/(b*d)*(fs2/fc) = 0.05941729$
 $v = Asl,mid/(b*d)*(fsv/fc) = 0.02100943$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)
 --->
 $v < vs,y2$ - LHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.16886055$

$$\begin{aligned} \mu &= MRC(4.14) = 1.8158E+008 \\ u &= s_u(4.1) = 1.7255512E-005 \end{aligned}$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of μ_2

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:

$$\begin{aligned} u &= 1.7255512E-005 \\ \mu &= 1.8158E+008 \end{aligned}$$

with full section properties:

$$\begin{aligned} b &= 750.00 \\ d &= 357.00 \\ d' &= 43.00 \\ v &= 0.00124684 \\ N &= 11016.808 \\ f_c &= 33.00 \\ c_o(5A.5, TBDY) &= 0.002 \\ \text{Final value of } \mu: \mu^* &= \text{shear_factor} * \text{Max}(\mu_c, \mu_s) = 0.01006403 \\ \text{The Shear_factor is considered equal to 1 (pure moment strength)} \\ \text{From (5.4b), TBDY: } \mu_c &= 0.01006403 \\ \mu_s(5.4c, TBDY) &= a_{se} * s_{h,\min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.02692906 \\ \text{where } f &= a_f * p_f * f_{fe}/f_{ce} \text{ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)} \end{aligned}$$

$$\begin{aligned} f_x &= 0.02504965 \\ a_f &= 0.38744444 \\ b &= 750.00 \\ h &= 400.00 \\ \text{From EC8 A.4.4.3(6), } p_f &= 2t_f/b_w = 0.00266667 \\ b_w &= 750.00 \\ \text{effective stress from (A.35), } f_{f,e} &= 800.0873 \end{aligned}$$

$$\begin{aligned} f_y &= 0.04491794 \\ a_f &= 0.38744444 \\ b &= 400.00 \\ h &= 750.00 \\ \text{From EC8 A.4.4.3(6), } p_f &= 2t_f/b_w = 0.005 \\ b_w &= 400.00 \\ \text{effective stress from (A.35), } f_{f,e} &= 765.1636 \end{aligned}$$

$$\begin{aligned} R &= 40.00 \\ \text{Effective FRP thickness, } t_f &= NL * t * \text{Cos}(b_1) = 1.00 \\ f_{u,f} &= 840.00 \\ E_f &= 82000.00 \\ u_{f} &= 0.015 \\ a_{se}((5.4d), TBDY) &= (a_{se1} * A_{ext} + a_{se2} * A_{int}) / A_{sec} = 0.12601038 \\ a_{se1} &= 0.12601038 \\ b_{o_1} &= 690.00 \\ h_{o_1} &= 340.00 \\ b_{i2_1} &= 1.1834E+006 \\ a_{se2} &= \text{Max}(a_{se1}, a_{se2}) = 0.12601038 \\ b_{o_2} &= 542.00 \\ h_{o_2} &= 192.00 \\ b_{i2_2} &= 661256.00 \end{aligned}$$

$p_{sh,\min} * F_{ywe} = \text{Min}(p_{sh,x} * F_{ywe}, p_{sh,y} * F_{ywe}) = 1.64062$
Expression ((5.4d), TBDY) for $p_{sh,\min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$p_{sh,x} * F_{ywe} = p_{sh1} * F_{ywe1} + p_{sh2} * F_{ywe2} = 1.64062$$

$$ps1 \text{ (external)} = (Ash1 \cdot h1 / s1) / Asec = 0.0020944$$

$$Ash1 = Astir_1 \cdot ns_1 = 157.0796$$

$$\text{No stirups, } ns_1 = 2.00$$

$$h1 = 400.00$$

$$ps2 \text{ (internal)} = (Ash2 \cdot h2 / s2) / Asec = 0.00026808$$

$$Ash2 = Astir_2 \cdot ns_2 = 100.531$$

$$\text{No stirups, } ns_2 = 2.00$$

$$h2 = 200.00$$

$$psh_y \cdot Fywe = psh1 \cdot Fywe1 + ps2 \cdot Fywe2 = 3.23907$$

$$ps1 \text{ (external)} = (Ash1 \cdot h1 / s1) / Asec = 0.00392699$$

$$Ash1 = Astir_1 \cdot ns_1 = 157.0796$$

$$\text{No stirups, } ns_1 = 2.00$$

$$h1 = 750.00$$

$$ps2 \text{ (internal)} = (Ash2 \cdot h2 / s2) / Asec = 0.00073723$$

$$Ash2 = Astir_2 \cdot ns_2 = 100.531$$

$$\text{No stirups, } ns_2 = 2.00$$

$$h2 = 550.00$$

$$Asec = 300000.00$$

$$s1 = 100.00$$

$$s2 = 250.00$$

$$fywe1 = 694.45$$

$$fywe2 = 694.45$$

$$fce = 33.00$$

$$\text{From } ((5.A.5), \text{ TBDY}), \text{ TBDY: } cc = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$y1 = 0.00140044$$

$$sh1 = 0.0044814$$

$$ft1 = 466.8167$$

$$fy1 = 389.0139$$

$$su1 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lou, \text{min} = lb/ld = 0.30$$

$$su1 = 0.4 \cdot esu1_{\text{nominal}} ((5.5), \text{ TBDY}) = 0.032$$

From table 5A.1, TBDY: $esu1_{\text{nominal}} = 0.08$,

For calculation of $esu1_{\text{nominal}}$ and $y1, sh1, ft1, fy1$, it is considered
characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fs1 = (fs_{\text{jacket}} \cdot Asl, \text{ten, jacket} + fs_{\text{core}} \cdot Asl, \text{ten, core}) / Asl, \text{ten} = 389.0139$$

$$\text{with } Es1 = (Es_{\text{jacket}} \cdot Asl, \text{ten, jacket} + Es_{\text{core}} \cdot Asl, \text{ten, core}) / Asl, \text{ten} = 200000.00$$

$$y2 = 0.00140044$$

$$sh2 = 0.0044814$$

$$ft2 = 466.8167$$

$$fy2 = 389.0139$$

$$su2 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lou, \text{min} = lb/lb, \text{min} = 0.30$$

$$su2 = 0.4 \cdot esu2_{\text{nominal}} ((5.5), \text{ TBDY}) = 0.032$$

From table 5A.1, TBDY: $esu2_{\text{nominal}} = 0.08$,

For calculation of $esu2_{\text{nominal}}$ and $y2, sh2, ft2, fy2$, it is considered
characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fs2 = (fs_{\text{jacket}} \cdot Asl, \text{com, jacket} + fs_{\text{core}} \cdot Asl, \text{com, core}) / Asl, \text{com} = 389.0139$$

$$\text{with } Es2 = (Es_{\text{jacket}} \cdot Asl, \text{com, jacket} + Es_{\text{core}} \cdot Asl, \text{com, core}) / Asl, \text{com} = 200000.00$$

$$yv = 0.00140044$$

$$shv = 0.0044814$$

$$ftv = 466.8167$$

$$fyv = 389.0139$$

$$suv = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$l_0/l_{ou,min} = l_b/l_d = 0.30$$

$$s_{uv} = 0.4 * e_{suv_nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $e_{suv_nominal} = 0.08$,

considering characteristic value $f_{sv} = f_{sv}/1.2$, from table 5.1, TBDY

For calculation of $e_{suv_nominal}$ and γ_v , sh_v, ft_v, fy_v , it is considered

characteristic value $f_{sv} = f_{sv}/1.2$, from table 5.1, TBDY.

$$\gamma_1, sh_1, ft_1, fy_1, \text{ are also multiplied by } \text{Min}(1, 1.25 * (l_b/l_d)^{2/3}), \text{ from 10.3.5, ASCE 41-17.}$$

$$\text{with } f_{sv} = (f_{s,jacket} * A_{s,mid,jacket} + f_{s,mid} * A_{s,mid,core}) / A_{s,mid} = 389.0139$$

$$\text{with } E_{sv} = (E_{s,jacket} * A_{s,mid,jacket} + E_{s,mid} * A_{s,mid,core}) / A_{s,mid} = 200000.00$$

$$1 = A_{s,ten} / (b * d) * (f_{s1} / f_c) = 0.0500703$$

$$2 = A_{s,com} / (b * d) * (f_{s2} / f_c) = 0.0500703$$

$$v = A_{s,mid} / (b * d) * (f_{sv} / f_c) = 0.01770442$$

and confined core properties:

$$b = 690.00$$

$$d = 327.00$$

$$d' = 13.00$$

$$f_{cc} (5A.2, TBDY) = 34.43547$$

$$c_c (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = A_{s,ten} / (b * d) * (f_{s1} / f_c) = 0.05941729$$

$$2 = A_{s,com} / (b * d) * (f_{s2} / f_c) = 0.05941729$$

$$v = A_{s,mid} / (b * d) * (f_{sv} / f_c) = 0.02100943$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$s_u (4.9) = 0.16886055$$

$$\mu_u = M_{Rc} (4.14) = 1.8158E+008$$

$$u = s_u (4.1) = 1.7255512E-005$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 848607.437$

Calculation of Shear Strength at edge 1, $V_{r1} = 848607.437$

$V_{r1} = V_{Col} ((10.3), ASCE 41-17) = k_{nl} * V_{Col0}$

$$V_{Col0} = 848607.437$$

$$k_{nl} = 1 \text{ (zero step-static loading)}$$

NOTE: In expression (10-3) ' $V_s = A_v * f_y * d / s$ ' is replaced by ' $V_s + f * V_f$ '

where V_f is the contribution of FRPs (11.3), ACI 440).

$$= 1 \text{ (normal-weight concrete)}$$

Mean concrete strength: $f_c' = (f_c'_{jacket} * Area_{jacket} + f_c'_{core} * Area_{core}) / Area_{section} = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$$M/Vd = 2.00$$

$$\mu_u = 3.5345895E-012$$

$$V_u = 3.4586346E-031$$

$$d = 0.8 * h = 320.00$$

$$N_u = 11016.808$$

$$A_g = 300000.00$$

$$\text{From (11.5.4.8), ACI 318-14: } V_s = V_{s1} + V_{s2} = 279254.914$$

where:

$V_{s1} = 279254.914$ is calculated for jacket, with:

$$d = 320.00$$

$$A_v = 157079.633$$

$$f_y = 555.56$$

$$s = 100.00$$

V_{s1} is multiplied by $Col1 = 1.00$

$$s/d = 0.3125$$

Vs2 = 0.00 is calculated for core, with:

$$d = 160.00$$

$$A_v = 100530.965$$

$$f_y = 555.56$$

$$s = 250.00$$

Vs2 is multiplied by Col2 = 0.00

$$s/d = 1.5625$$

Vf ((11-3)-(11.4), ACI 440) = 234192.00

$$f = 0.95, \text{ for fully-wrapped sections}$$

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) $\sin^2 + \cos^2$ is replaced with $(\cot^2 + \csc^2)\sin^2\alpha$ which is more a generalised expression,

where α is the angle of the crack direction (see KANEPE).

This later relation, considered as a function Vf(α), is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\alpha = 45^\circ$ and $\alpha = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\alpha = b1 + 90^\circ = 90.00$

Vf = Min(|Vf(45, α)|, |Vf(-45, α)|), with:

total thickness per orientation, tf1 = NL*t/NoDir = 1.00

dfv = d (figure 11.2, ACI 440) = 357.00

ffe ((11-5), ACI 440) = 328.00

$$E_f = 82000.00$$

$$f_e = 0.004, \text{ from (11.6a), ACI 440}$$

$$\text{with } f_u = 0.009$$

From (11-11), ACI 440: Vs + Vf <= 915872.391

$$b_w = 750.00$$

Calculation of Shear Strength at edge 2, Vr2 = 848607.437

Vr2 = VCol ((10.3), ASCE 41-17) = knl*VCol0

$$V_{Col0} = 848607.437$$

kn1 = 1 (zero step-static loading)

NOTE: In expression (10-3) 'Vs = Av*fy*d/s' is replaced by 'Vs+ f*Vf' where Vf is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: $f_c' = (f_c'_{jacket} * Area_{jacket} + f_c'_{core} * Area_{core}) / Area_{section} = 33.00$, but $f_c'^{0.5} <= 8.3$ MPa (22.5.3.1, ACI 318-14)

$$M/Vd = 2.00$$

$$\mu_u = 3.5345895E-012$$

$$V_u = 3.4586346E-031$$

$$d = 0.8 * h = 320.00$$

$$N_u = 11016.808$$

$$A_g = 300000.00$$

From (11.5.4.8), ACI 318-14: Vs = Vs1 + Vs2 = 279254.914

where:

Vs1 = 279254.914 is calculated for jacket, with:

$$d = 320.00$$

$$A_v = 157079.633$$

$$f_y = 555.56$$

$$s = 100.00$$

Vs1 is multiplied by Col1 = 1.00

$$s/d = 0.3125$$

Vs2 = 0.00 is calculated for core, with:

$$d = 160.00$$

$$A_v = 100530.965$$

$$f_y = 555.56$$

$$s = 250.00$$

Vs2 is multiplied by Col2 = 0.00

$$s/d = 1.5625$$

Vf ((11-3)-(11.4), ACI 440) = 234192.00

$$f = 0.95, \text{ for fully-wrapped sections}$$

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) $\sin^2 + \cos^2$ is replaced with $(\cot^2 + \csc^2)\sin^2\alpha$ which is more a generalised expression, where α is the angle of the crack direction (see KANEPE).

This later relation, considered as a function Vf(α), is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\alpha = 45^\circ$ and $\alpha = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\alpha = b1 + 90^\circ = 90.00$

Vf = Min(|Vf(45, 1)|,|Vf(-45,a1)|), with:
total thickness per orientation, tf1 = NL*t/NoDir = 1.00
dfv = d (figure 11.2, ACI 440) = 357.00
ffe ((11-5), ACI 440) = 328.00
Ef = 82000.00
fe = 0.004, from (11.6a), ACI 440
with fu = 0.009
From (11-11), ACI 440: Vs + Vf <= 915872.391
bw = 750.00

End Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1
At local axis: 3

Start Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1
At Shear local axis: 2
(Bending local axis: 3)
Section Type: rcjrs

Constant Properties

Knowledge Factor, = 0.90
Mean strength values are used for both shear and moment calculations.
Consequently:

Jacket

New material of Primary Member: Concrete Strength, fc = fcm = 33.00
New material of Primary Member: Steel Strength, fs = fsm = 555.56
Concrete Elasticity, Ec = 26999.444
Steel Elasticity, Es = 200000.00

Existing Column

New material of Primary Member: Concrete Strength, fc = fcm = 33.00
New material of Primary Member: Steel Strength, fs = fsm = 555.56
Concrete Elasticity, Ec = 26999.444
Steel Elasticity, Es = 200000.00

#####

Note: Especially for the calculation of moment strengths,
the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

Jacket

New material: Steel Strength, fs = 1.25*fsm = 694.45

Existing Column

New material: Steel Strength, fs = 1.25*fsm = 694.45

#####

External Height, H = 400.00

External Width, W = 750.00

Internal Height, H = 200.00

Internal Width, W = 550.00

Cover Thickness, c = 25.00

Mean Confinement Factor overall section = 1.0435

Element Length, L = 3000.00

Primary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Inadequate Lap Length with lo/lo,min = 0.30

FRP Wrapping Data

Type: Carbon

Dry properties (design values)

Thickness, t = 1.00

Tensile Strength, ffu = 840.00

Tensile Modulus, Ef = 82000.00

Elongation, efu = 0.009

Number of directions, NoDir = 1

Fiber orientations, bi: 0.00°

Number of layers, NL = 1
Radius of rounding corners, R = 40.00

Stepwise Properties

At local axis: 2
EDGE -A-
Shear Force, Va = 2.9582284E-031
EDGE -B-
Shear Force, Vb = -2.9582284E-031
BOTH EDGES
Axial Force, F = -11016.808
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: Aslt = 0.00
-Compression: Aslc = 2676.637
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: Asl,ten = 1137.257
-Compression: Asl,com = 1137.257
-Middle: Asl,mid = 402.1239

Calculation of Shear Capacity ratio , $V_e/V_r = 0.19671905$
Member Controlled by Flexure ($V_e/V_r < 1$)
Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 248405.514$
with
 $M_{pr1} = \text{Max}(\mu_{1+}, \mu_{1-}) = 3.7261E+008$
 $\mu_{1+} = 3.7261E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction
which is defined for the static loading combination
 $\mu_{1-} = 3.7261E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment
direction which is defined for the static loading combination
 $M_{pr2} = \text{Max}(\mu_{2+}, \mu_{2-}) = 3.7261E+008$
 $\mu_{2+} = 3.7261E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction
which is defined for the the static loading combination
 $\mu_{2-} = 3.7261E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment
direction which is defined for the the static loading combination

Calculation of μ_{1+}

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:
 $\mu = 8.5888248E-006$
 $M_u = 3.7261E+008$

with full section properties:

$b = 400.00$
 $d = 707.00$
 $d' = 43.00$
 $v = 0.00118049$
 $N = 11016.808$
 $f_c = 33.00$
 $\alpha (5A.5, TBDY) = 0.002$
Final value of μ_c : $\mu_c^* = \text{shear_factor} * \text{Max}(\mu_c, \mu_c) = 0.01006403$
The Shear_factor is considered equal to 1 (pure moment strength)
From (5.4b), TBDY: $\mu_c = 0.01006403$
 $\mu_e ((5.4c), TBDY) = \alpha * \text{sh}_{, \text{min}} * f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$
where $f = \alpha * \rho_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

 $f_x = 0.02504965$
 $\alpha_f = 0.38744444$
 $b = 750.00$
 $h = 400.00$
From EC8 A.4.4.3(6), $\rho_f = 2t_f/bw = 0.00266667$

bw = 750.00
effective stress from (A.35), $f_{f,e} = 800.0873$

fy = 0.04491794
af = 0.38744444
b = 400.00
h = 750.00
From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.005$
bw = 400.00
effective stress from (A.35), $f_{f,e} = 765.1636$

R = 40.00
Effective FRP thickness, $t_f = NL*t*Cos(b1) = 1.00$
fu,f = 840.00
Ef = 82000.00
u,f = 0.015
ase ((5.4d), TBDY) = $(ase1*A_{ext} + ase2*A_{int})/A_{sec} = 0.12601038$
ase1 = 0.12601038
bo_1 = 690.00
ho_1 = 340.00
bi2_1 = 1.1834E+006
ase2 = $Max(ase1, ase2) = 0.12601038$
bo_2 = 542.00
ho_2 = 192.00
bi2_2 = 661256.00

psh,min*Fywe = $Min(psh,x*Fywe, psh,y*Fywe) = 1.64062$
Expression ((5.4d), TBDY) for psh,min*Fywe has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh_x*Fywe = $psh1*Fywe1 + ps2*Fywe2 = 1.64062$
ps1 (external) = $(Ash1*h1/s1)/A_{sec} = 0.0020944$
Ash1 = $A_{stir_1}*ns_1 = 157.0796$
No stirrups, ns_1 = 2.00
h1 = 400.00
ps2 (internal) = $(Ash2*h2/s2)/A_{sec} = 0.00026808$
Ash2 = $A_{stir_2}*ns_2 = 100.531$
No stirrups, ns_2 = 2.00
h2 = 200.00

psh_y*Fywe = $psh1*Fywe1 + ps2*Fywe2 = 3.23907$
ps1 (external) = $(Ash1*h1/s1)/A_{sec} = 0.00392699$
Ash1 = $A_{stir_1}*ns_1 = 157.0796$
No stirrups, ns_1 = 2.00
h1 = 750.00
ps2 (internal) = $(Ash2*h2/s2)/A_{sec} = 0.00073723$
Ash2 = $A_{stir_2}*ns_2 = 100.531$
No stirrups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/d = 0.30
su1 = $0.4*esu1_nominal$ ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $Min(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = (fs_{jacket} * Asl_{ten,jacket} + fs_{core} * Asl_{ten,core}) / Asl_{ten} = 389.0139$

with $Es1 = (Es_{jacket} * Asl_{ten,jacket} + Es_{core} * Asl_{ten,core}) / Asl_{ten} = 200000.00$

$y2 = 0.00140044$

$sh2 = 0.0044814$

$ft2 = 466.8167$

$fy2 = 389.0139$

$su2 = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with $shear_factor$

and also multiplied by the $shear_factor$ according to 15.7.1.4, with

$Shear_factor = 1.00$

$lo/lou, min = lb/lb, min = 0.30$

$su2 = 0.4 * esu2_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $Min(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = (fs_{jacket} * Asl_{com,jacket} + fs_{core} * Asl_{com,core}) / Asl_{com} = 389.0139$

with $Es2 = (Es_{jacket} * Asl_{com,jacket} + Es_{core} * Asl_{com,core}) / Asl_{com} = 200000.00$

$yv = 0.00140044$

$shv = 0.0044814$

$ftv = 466.8167$

$fyv = 389.0139$

$suv = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with $shear_factor$

and also multiplied by the $shear_factor$ according to 15.7.1.4, with

$Shear_factor = 1.00$

$lo/lou, min = lb/ld = 0.30$

$suv = 0.4 * esuv_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esuv_nominal = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY

For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered

characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $Min(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fsv = (fs_{jacket} * Asl_{mid,jacket} + fs_{mid} * Asl_{mid,core}) / Asl_{mid} = 389.0139$

with $Esv = (Es_{jacket} * Asl_{mid,jacket} + Es_{mid} * Asl_{mid,core}) / Asl_{mid} = 200000.00$

$1 = Asl_{ten} / (b * d) * (fs1 / fc) = 0.04740567$

$2 = Asl_{com} / (b * d) * (fs2 / fc) = 0.04740567$

$v = Asl_{mid} / (b * d) * (fsv / fc) = 0.01676222$

and confined core properties:

$b = 340.00$

$d = 677.00$

$d' = 13.00$

$fcc (5A.2, TBDY) = 34.43547$

$cc (5A.5, TBDY) = 0.00243499$

$c =$ confinement factor $= 1.0435$

$1 = Asl_{ten} / (b * d) * (fs1 / fc) = 0.05824278$

$2 = Asl_{com} / (b * d) * (fs2 / fc) = 0.05824278$

$v = Asl_{mid} / (b * d) * (fsv / fc) = 0.02059413$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < vs, y2$ - LHS eq.(4.5) is satisfied

--->

$su (4.9) = 0.15682678$

$Mu = MRc (4.14) = 3.7261E+008$

$u = su (4.1) = 8.5888248E-006$

Calculation of ratio lb/ld

Inadequate Lap Length with $lb/ld = 0.30$

Calculation of Mu1-

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:

$$\mu = 8.5888248E-006$$

$$\text{Mu} = 3.7261E+008$$

with full section properties:

$$b = 400.00$$

$$d = 707.00$$

$$d' = 43.00$$

$$v = 0.00118049$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$c_o (5A.5, \text{TBDY}) = 0.002$$

$$\text{Final value of } c_u: c_u^* = \text{shear_factor} * \text{Max}(c_u, c_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } c_u = 0.01006403$$

$$w_e ((5.4c), \text{TBDY}) = a_{se} * s_{h,\min} * f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$$

where $f = a_f * p_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.02504965$$

$$a_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f / b_w = 0.00266667$$

$$b_w = 750.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 800.0873$$

$$f_y = 0.04491794$$

$$a_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f / b_w = 0.005$$

$$b_w = 400.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L * t * \text{Cos}(b_1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{,f} = 0.015$$

$$a_{se} ((5.4d), \text{TBDY}) = (a_{se1} * A_{ext} + a_{se2} * A_{int}) / A_{sec} = 0.12601038$$

$$a_{se1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i2,1} = 1.1834E+006$$

$$a_{se2} = \text{Max}(a_{se1}, a_{se2}) = 0.12601038$$

$$b_{o,2} = 542.00$$

$$h_{o,2} = 192.00$$

$$b_{i2,2} = 661256.00$$

$$p_{sh,\min} * F_{ywe} = \text{Min}(p_{sh,x} * F_{ywe}, p_{sh,y} * F_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $p_{sh,\min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$p_{sh,x} * F_{ywe} = p_{sh1} * F_{ywe1} + p_{sh2} * F_{ywe2} = 1.64062$$

$$p_{sh1} (\text{external}) = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$$

$$A_{sh1} = A_{stir,1} * n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

$$h_1 = 400.00$$

$$p_{sh2} (\text{internal}) = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00026808$$

$$A_{sh2} = A_{stir,2} * n_{s,2} = 100.531$$

$$\text{No stirrups, } n_{s,2} = 2.00$$

$$h_2 = 200.00$$

psh_y*Fywe = psh₁*Fywe₁+ps₂*Fywe₂ = 3.23907
ps₁ (external) = (Ash₁*h₁/s₁)/Asec = 0.00392699
Ash₁ = Astir₁*ns₁ = 157.0796
No stirups, ns₁ = 2.00
h₁ = 750.00
ps₂ (internal) = (Ash₂*h₂/s₂)/Asec = 0.00073723
Ash₂ = Astir₂*ns₂ = 100.531
No stirups, ns₂ = 2.00
h₂ = 550.00

Asec = 300000.00

s₁ = 100.00

s₂ = 250.00

fywe₁ = 694.45

fywe₂ = 694.45

fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499

c = confinement factor = 1.0435

y₁ = 0.00140044

sh₁ = 0.0044814

ft₁ = 466.8167

fy₁ = 389.0139

su₁ = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

su₁ = 0.4*esu_{1_nominal} ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu_{1_nominal} = 0.08,

For calculation of esu_{1_nominal} and y₁, sh₁,ft₁,fy₁, it is considered
characteristic value fsy₁ = fs₁/1.2, from table 5.1, TBDY.

y₁, sh₁,ft₁,fy₁, are also multiplied by Min(1,1.25*(lb/ld)^{2/3}), from 10.3.5, ASCE 41-17.

with fs₁ = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es₁ = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y₂ = 0.00140044

sh₂ = 0.0044814

ft₂ = 466.8167

fy₂ = 389.0139

su₂ = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su₂ = 0.4*esu_{2_nominal} ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu_{2_nominal} = 0.08,

For calculation of esu_{2_nominal} and y₂, sh₂,ft₂,fy₂, it is considered
characteristic value fsy₂ = fs₂/1.2, from table 5.1, TBDY.

y₂, sh₂,ft₂,fy₂, are also multiplied by Min(1,1.25*(lb/ld)^{2/3}), from 10.3.5, ASCE 41-17.

with fs₂ = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es₂ = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

y_v = 0.00140044

sh_v = 0.0044814

ft_v = 466.8167

fy_v = 389.0139

suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsy_v = fs_v/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and y_v, sh_v,ft_v,fy_v, it is considered
characteristic value fsy_v = fs_v/1.2, from table 5.1, TBDY.

y₁, sh₁,ft₁,fy₁, are also multiplied by Min(1,1.25*(lb/ld)^{2/3}), from 10.3.5, ASCE 41-17.

with fs_v = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Es_v = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

$$1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.04740567$$

$$2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.04740567$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.01676222$$

and confined core properties:

$$b = 340.00$$

$$d = 677.00$$

$$d' = 13.00$$

$$f_{cc} (5A.2, TBDY) = 34.43547$$

$$c_c (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.05824278$$

$$2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.05824278$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.02059413$$

Case/Assumption: Unconfined full section - Steel rupture
' satisfies Eq. (4.3)

--->
 $v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$s_u (4.9) = 0.15682678$$

$$\mu_u = M R_c (4.14) = 3.7261E+008$$

$$u = s_u (4.1) = 8.5888248E-006$$

 Calculation of ratio l_b/l_d

 Inadequate Lap Length with $l_b/l_d = 0.30$

 Calculation of μ_{u2+}

 Calculation of ultimate curvature μ_u according to 4.1, Biskinis/Fardis 2013:
 $u = 8.5888248E-006$
 $\mu_u = 3.7261E+008$

 with full section properties:

$$b = 400.00$$

$$d = 707.00$$

$$d' = 43.00$$

$$v = 0.00118049$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$c_c (5A.5, TBDY) = 0.002$$

$$\text{Final value of } c_u: c_u^* = \text{shear_factor} * \text{Max}(c_u, c_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } c_u = 0.01006403$$

$$w_e ((5.4c), TBDY) = a_{se} * s_{h,min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$$

where $f = a_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.02504965$$

$$a_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.00266667$$

$$b_w = 750.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 800.0873$$

$$f_y = 0.04491794$$

$$a_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.005$$

$$b_w = 400.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 765.1636$$

$R = 40.00$
 Effective FRP thickness, $t_f = NL * t * \cos(b_1) = 1.00$
 $f_{u,f} = 840.00$
 $E_f = 82000.00$
 $u_{,f} = 0.015$
 $ase((5.4d), TBDY) = (ase_1 * A_{ext} + ase_2 * A_{int}) / A_{sec} = 0.12601038$
 $ase_1 = 0.12601038$
 $bo_1 = 690.00$
 $ho_1 = 340.00$
 $bi_{2,1} = 1.1834E+006$
 $ase_2 = \text{Max}(ase_1, ase_2) = 0.12601038$
 $bo_2 = 542.00$
 $ho_2 = 192.00$
 $bi_{2,2} = 661256.00$

$psh_{,min} * F_{ywe} = \text{Min}(psh_{,x} * F_{ywe}, psh_{,y} * F_{ywe}) = 1.64062$
 Expression ((5.4d), TBDY) for $psh_{,min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh_{,x} * F_{ywe} = psh_1 * F_{ywe1} + ps_2 * F_{ywe2} = 1.64062$
 $ps_1 \text{ (external)} = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$
 $A_{sh1} = A_{stir_1} * ns_1 = 157.0796$
 No stirrups, $ns_1 = 2.00$
 $h_1 = 400.00$
 $ps_2 \text{ (internal)} = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00026808$
 $A_{sh2} = A_{stir_2} * ns_2 = 100.531$
 No stirrups, $ns_2 = 2.00$
 $h_2 = 200.00$

$psh_{,y} * F_{ywe} = psh_1 * F_{ywe1} + ps_2 * F_{ywe2} = 3.23907$
 $ps_1 \text{ (external)} = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.00392699$
 $A_{sh1} = A_{stir_1} * ns_1 = 157.0796$
 No stirrups, $ns_1 = 2.00$
 $h_1 = 750.00$
 $ps_2 \text{ (internal)} = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00073723$
 $A_{sh2} = A_{stir_2} * ns_2 = 100.531$
 No stirrups, $ns_2 = 2.00$
 $h_2 = 550.00$

$A_{sec} = 300000.00$
 $s_1 = 100.00$
 $s_2 = 250.00$
 $fy_{we1} = 694.45$
 $fy_{we2} = 694.45$
 $f_{ce} = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00243499$
 $c = \text{confinement factor} = 1.0435$

$y_1 = 0.00140044$
 $sh_1 = 0.0044814$
 $ft_1 = 466.8167$
 $fy_1 = 389.0139$
 $su_1 = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lo_{,min} = lb/ld = 0.30$
 $su_1 = 0.4 * esu_{1,nominal}((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu_{1,nominal} = 0.08$,

For calculation of $esu_{1,nominal}$ and y_1, sh_1, ft_1, fy_1 , it is considered
 characteristic value $fsy_1 = fs_1 / 1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs_1 = (f_{s,jacket} * A_{sl,ten,jacket} + f_{s,core} * A_{sl,ten,core}) / A_{sl,ten} = 389.0139$

with $Es_1 = (E_{s,jacket} * A_{sl,ten,jacket} + E_{s,core} * A_{sl,ten,core}) / A_{sl,ten} = 200000.00$

$y_2 = 0.00140044$
 $sh_2 = 0.0044814$
 $ft_2 = 466.8167$
 $fy_2 = 389.0139$
 $su_2 = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$l_o/l_{o,u,min} = l_b/l_{b,min} = 0.30$$

$$s_u2 = 0.4 * e_{s_u2,nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $e_{s_u2,nominal} = 0.08$,

For calculation of $e_{s_u2,nominal}$ and y_2, sh_2, ft_2, fy_2 , it is considered
characteristic value $f_{s_y2} = f_{s_2}/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } f_{s_2} = (f_{s,jacket} * A_{s_l,com,jacket} + f_{s,core} * A_{s_l,com,core}) / A_{s_l,com} = 389.0139$$

$$\text{with } E_{s_2} = (E_{s,jacket} * A_{s_l,com,jacket} + E_{s,core} * A_{s_l,com,core}) / A_{s_l,com} = 200000.00$$

$$y_v = 0.00140044$$

$$sh_v = 0.0044814$$

$$ft_v = 466.8167$$

$$fy_v = 389.0139$$

$$s_{u_v} = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$l_o/l_{o,u,min} = l_b/l_d = 0.30$$

$$s_{u_v} = 0.4 * e_{s_{u_v},nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $e_{s_{u_v},nominal} = 0.08$,

considering characteristic value $f_{s_{y_v}} = f_{s_v}/1.2$, from table 5.1, TBDY

For calculation of $e_{s_{u_v},nominal}$ and y_v, sh_v, ft_v, fy_v , it is considered
characteristic value $f_{s_{y_v}} = f_{s_v}/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } f_{s_v} = (f_{s,jacket} * A_{s_l,mid,jacket} + f_{s,mid} * A_{s_l,mid,core}) / A_{s_l,mid} = 389.0139$$

$$\text{with } E_{s_v} = (E_{s,jacket} * A_{s_l,mid,jacket} + E_{s,mid} * A_{s_l,mid,core}) / A_{s_l,mid} = 200000.00$$

$$1 = A_{s_l,ten} / (b * d) * (f_{s_1} / f_c) = 0.04740567$$

$$2 = A_{s_l,com} / (b * d) * (f_{s_2} / f_c) = 0.04740567$$

$$v = A_{s_l,mid} / (b * d) * (f_{s_v} / f_c) = 0.01676222$$

and confined core properties:

$$b = 340.00$$

$$d = 677.00$$

$$d' = 13.00$$

$$f_{cc} (5A.2, TBDY) = 34.43547$$

$$cc (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = A_{s_l,ten} / (b * d) * (f_{s_1} / f_c) = 0.05824278$$

$$2 = A_{s_l,com} / (b * d) * (f_{s_2} / f_c) = 0.05824278$$

$$v = A_{s_l,mid} / (b * d) * (f_{s_v} / f_c) = 0.02059413$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y_2}$ - LHS eq.(4.5) is satisfied

--->

$$s_u (4.9) = 0.15682678$$

$$\mu_u = M_{Rc} (4.14) = 3.7261E+008$$

$$u = s_u (4.1) = 8.5888248E-006$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of μ_u -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$$u = 8.5888248E-006$$

$$\mu_u = 3.7261E+008$$

with full section properties:

$$b = 400.00$$

$$d = 707.00$$

$$d' = 43.00$$

$$v = 0.00118049$$

$$N = 11016.808$$

$$fc = 33.00$$

$$co (5A.5, TBDY) = 0.002$$

$$\text{Final value of } cu: cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } cu = 0.01006403$$

$$we ((5.4c), TBDY) = ase^* sh_{,min} * fy_{we} / f_{ce} + \text{Min}(fx, fy) = 0.02692906$$

where $f = af * pf * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$fx = 0.02504965$$

$$af = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } pf = 2tf/bw = 0.00266667$$

$$bw = 750.00$$

$$\text{effective stress from (A.35), } ff_{,e} = 800.0873$$

$$fy = 0.04491794$$

$$af = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } pf = 2tf/bw = 0.005$$

$$bw = 400.00$$

$$\text{effective stress from (A.35), } ff_{,e} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } tf = NL * t * \text{Cos}(b1) = 1.00$$

$$fu_{,f} = 840.00$$

$$Ef = 82000.00$$

$$u_{,f} = 0.015$$

$$ase ((5.4d), TBDY) = (ase1 * A_{ext} + ase2 * A_{int}) / A_{sec} = 0.12601038$$

$$ase1 = 0.12601038$$

$$bo_{,1} = 690.00$$

$$ho_{,1} = 340.00$$

$$bi2_{,1} = 1.1834E+006$$

$$ase2 = \text{Max}(ase1, ase2) = 0.12601038$$

$$bo_{,2} = 542.00$$

$$ho_{,2} = 192.00$$

$$bi2_{,2} = 661256.00$$

$$psh_{,min} * Fy_{we} = \text{Min}(psh_{,x} * Fy_{we}, psh_{,y} * Fy_{we}) = 1.64062$$

Expression ((5.4d), TBDY) for $psh_{,min} * Fy_{we}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$psh_{,x} * Fy_{we} = psh1 * Fy_{we1} + ps2 * Fy_{we2} = 1.64062$$

$$ps1 \text{ (external)} = (Ash1 * h1 / s1) / A_{sec} = 0.0020944$$

$$Ash1 = Astir_{,1} * ns_{,1} = 157.0796$$

$$\text{No stirrups, } ns_{,1} = 2.00$$

$$h1 = 400.00$$

$$ps2 \text{ (internal)} = (Ash2 * h2 / s2) / A_{sec} = 0.00026808$$

$$Ash2 = Astir_{,2} * ns_{,2} = 100.531$$

$$\text{No stirrups, } ns_{,2} = 2.00$$

$$h2 = 200.00$$

$$psh_{,y} * Fy_{we} = psh1 * Fy_{we1} + ps2 * Fy_{we2} = 3.23907$$

$$ps1 \text{ (external)} = (Ash1 * h1 / s1) / A_{sec} = 0.00392699$$

$$Ash1 = Astir_{,1} * ns_{,1} = 157.0796$$

$$\text{No stirrups, } ns_{,1} = 2.00$$

$$h1 = 750.00$$

$$ps2 \text{ (internal)} = (Ash2 * h2 / s2) / A_{sec} = 0.00073723$$

$$Ash2 = Astir_{,2} * ns_{,2} = 100.531$$

$$\text{No stirrups, } ns_{,2} = 2.00$$

$$h2 = 550.00$$

$$A_{sec} = 300000.00$$

s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A.5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044
sh2 = 0.0044814
ft2 = 466.8167
fy2 = 389.0139
su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044
shv = 0.0044814
ftv = 466.8167
fyv = 389.0139
suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.04740567

2 = Asl,com/(b*d)*(fs2/fc) = 0.04740567

v = Asl,mid/(b*d)*(fsv/fc) = 0.01676222

and confined core properties:

b = 340.00
d = 677.00
d' = 13.00

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

1 = Asl,ten/(b*d)*(fs1/fc) = 0.05824278

$$2 = A_{sl,com}/(b*d)*(f_s2/f_c) = 0.05824278$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.02059413$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

---->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

---->

$$s_u(4.9) = 0.15682678$$

$$M_u = M_{Rc}(4.14) = 3.7261E+008$$

$$u = s_u(4.1) = 8.5888248E-006$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 1.2627E+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 1.2627E+006$

$V_{r1} = V_{Col}((10.3), ASCE 41-17) = k_{nl} * V_{Col0}$

$$V_{Col0} = 1.2627E+006$$

$k_{nl} = 1$ (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v * f_y * d / s$ ' is replaced by ' $V_{s+} = f * V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

$= 1$ (normal-weight concrete)

Mean concrete strength: $f'_c = (f'_c_{jacket} * Area_{jacket} + f'_c_{core} * Area_{core}) / Area_{section} = 33.00$, but $f_c^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$$M/Vd = 2.00$$

$$M_u = 2.3795529E-012$$

$$V_u = 2.9582284E-031$$

$$d = 0.8 * h = 600.00$$

$$N_u = 11016.808$$

$$A_g = 300000.00$$

$$\text{From (11.5.4.8), ACI 318-14: } V_s = V_{s1} + V_{s2} = 621900.694$$

where:

$V_{s1} = 523602.964$ is calculated for jacket, with:

$$d = 600.00$$

$$A_v = 157079.633$$

$$f_y = 555.56$$

$$s = 100.00$$

V_{s1} is multiplied by $Col1 = 1.00$

$$s/d = 0.16666667$$

$V_{s2} = 98297.73$ is calculated for core, with:

$$d = 440.00$$

$$A_v = 100530.965$$

$$f_y = 555.56$$

$$s = 250.00$$

V_{s2} is multiplied by $Col2 = 1.00$

$$s/d = 0.56818182$$

$V_f((11-3)-(11.4), ACI 440) = 463792.00$

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot_a) \sin a$ which is more a generalised expression, where θ is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, a_i)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta_1)|, |V_f(-45, a1)|)$, with:

total thickness per orientation, $t_{f1} = N_L * t / \text{NoDir} = 1.00$

$d_{fv} = d$ (figure 11.2, ACI 440) = 707.00

$f_{fe}((11-5), ACI 440) = 328.00$

$$E_f = 82000.00$$

$f_e = 0.004$, from (11.6a), ACI 440
with $f_u = 0.009$
From (11-11), ACI 440: $V_s + V_f \leq 915872.391$
 $bw = 400.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 1.2627E+006$
 $V_{r2} = V_{Col} ((10.3), ASCE 41-17) = knl * V_{Col0}$
 $V_{Col0} = 1.2627E+006$
 $knl = 1$ (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v * f_y * d / s$ ' is replaced by ' $V_s + f * V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

 $= 1$ (normal-weight concrete)
Mean concrete strength: $f_c' = (f_c'_{jacket} * Area_{jacket} + f_c'_{core} * Area_{core}) / Area_{section} = 33.00$, but $f_c'^{0.5} \leq 8.3$
MPa (22.5.3.1, ACI 318-14)

$M / Vd = 2.00$
 $\mu_u = 2.3795529E-012$
 $V_u = 2.9582284E-031$
 $d = 0.8 * h = 600.00$
 $N_u = 11016.808$
 $A_g = 300000.00$
From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 621900.694$

where:

$V_{s1} = 523602.964$ is calculated for jacket, with:

$d = 600.00$
 $A_v = 157079.633$
 $f_y = 555.56$
 $s = 100.00$

V_{s1} is multiplied by $Col1 = 1.00$

$s/d = 0.16666667$

$V_{s2} = 98297.73$ is calculated for core, with:

$d = 440.00$
 $A_v = 100530.965$
 $f_y = 555.56$
 $s = 250.00$

V_{s2} is multiplied by $Col2 = 1.00$

$s/d = 0.56818182$

$V_f ((11-3)-(11.4), ACI 440) = 463792.00$

$f = 0.95$, for fully-wrapped sections

$w_f / s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,
where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, a)$, is implemented for every different fiber orientation a_i ,
as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, a1)|)$, with:

total thickness per orientation, $t_{f1} = NL * t / \text{NoDir} = 1.00$

$d_{fv} = d$ (figure 11.2, ACI 440) = 707.00

$f_{fe} ((11-5), ACI 440) = 328.00$

$E_f = 82000.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.009$

From (11-11), ACI 440: $V_s + V_f \leq 915872.391$

$bw = 400.00$

End Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At local axis: 2

Start Of Calculation of Chord Rotation Capacity for element: column JC1 of floor 1

At local axis: 3

Integration Section: (a)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$

Chord Rotation is generally considered as Deformation-Controlled Action according to Table C7-1, ASCE 41-17.

Mean strengths are used for Deformation-Controlled Actions according to 7.5.1.3, ASCE 41-17

Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Existing Column

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

External Height, $H = 400.00$

External Width, $W = 750.00$

Internal Height, $H = 200.00$

Internal Width, $W = 550.00$

Cover Thickness, $c = 25.00$

Element Length, $L = 3000.00$

Primary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Inadequate Lap Length with $l_b/l_d = 0.30$

FRP Wrapping Data

Type: Carbon

Dry properties (design values)

Thickness, $t = 1.00$

Tensile Strength, $f_{fu} = 840.00$

Tensile Modulus, $E_f = 82000.00$

Elongation, $e_{fu} = 0.009$

Number of directions, $NoDir = 1$

Fiber orientations, $bi = 0.00^\circ$

Number of layers, $NL = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

Bending Moment, $M = -7.0400E+006$

Shear Force, $V_2 = -1995.365$

Shear Force, $V_3 = -1.4564234E-013$

Axial Force, $F = -14177.47$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $As_c = 2676.637$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{t,ten} = 1137.257$

-Compression: $As_{l,com} = 1137.257$

-Middle: $As_{l,mid} = 402.1239$

Longitudinal External Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{t,ten,jacket} = 829.3805$

-Compression: $As_{l,com,jacket} = 829.3805$

-Middle: $As_{l,mid,jacket} = 402.1239$

Longitudinal Internal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{t,ten,core} = 307.8761$

-Compression: $As_{l,com,core} = 307.8761$

-Middle: $As_{l,mid,core} = 0.00$

Mean Diameter of Tension Reinforcement, $Db_L = 16.80$

New component: From table 7-7, ASCE 41_17: Final chord rotation Capacity $u, R = 1.0^* u = 0.00270555$
 $u = y + p = 0.00270555$

- Calculation of y -

$y = (M_y * L_s / 3) / E_{eff} = 0.00270555$ ((4.29), Biskinis Phd))
 $M_y = 2.6204E+008$
 $L_s = M/V$ (with $L_s > 0.1 * L$ and $L_s < 2 * L$) = 3528.173
From table 10.5, ASCE 41_17: $E_{eff} = factor * E_c * I_g = 1.1390E+014$
 $factor = 0.30$
 $A_g = 300000.00$
Mean concrete strength: $f_c' = (f_c'_{jacket} * Area_{jacket} + f_c'_{core} * Area_{core}) / Area_{section} = 33.00$
 $N = 14177.47$
 $E_c * I_g = E_{c,jacket} * I_{g,jacket} + E_{c,core} * I_{g,core} = 3.7968E+014$

Calculation of Yielding Moment M_y

Calculation of y and M_y according to Annex 7 -

$y = \text{Min}(y_{ten}, y_{com})$
 $y_{ten} = 2.8029545E-006$
with ((10.1), ASCE 41-17) $f_y = \text{Min}(f_y, 1.25 * f_y * (l_b / l_d)^{2/3}) = 311.2112$
 $d = 707.00$
 $y = 0.214783$
 $A = 0.00962586$
 $B = 0.0051813$
with $p_t = 0.00402142$
 $p_c = 0.00402142$
 $p_v = 0.00142194$
 $N = 14177.47$
 $b = 400.00$
 $" = 0.06082037$
 $y_{comp} = 1.4829079E-005$
with f_c^* (12.3, (ACI 440)) = 33.35516
 $f_c = 33.00$
 $f_l = 0.77176471$
 $b = 400.00$
 $h = 750.00$
 $A_g = 300000.00$
From (12.9), ACI 440: $k_a = 0.14679369$
 $g = p_t + p_c + p_v = 0.00946477$
 $rc = 40.00$
 $A_e / A_c = 0.51607158$
Effective FRP thickness, $t_f = NL * t * \text{Cos}(b_1) = 1.00$
effective strain from (12.5) and (12.12), $e_{fe} = 0.004$
 $f_u = 0.009$
 $E_f = 82000.00$
 $E_c = 26999.444$
 $y = 0.21210318$
 $A = 0.00935205$
 $B = 0.00502021$
with $E_s = 200000.00$

Calculation of ratio l_b / l_d

Inadequate Lap Length with $l_b / l_d = 0.30$

- Calculation of p -

From table 10-8: $p = 0.00$

with:

- Columns controlled by inadequate development or splicing along the clear height because $l_b/l_d < 1$

shear control ratio $V_y E / V_{CoI} E = 0.19671905$

$d = d_{external} = 707.00$

$s = s_{external} = 0.00$

$t = s_1 + s_2 + 2 * t_f / b_w * (f_{fe} / f_s) = 0.00761619$

jacket: $s_1 = A_{v1} * h_1 / (s_1 * A_g) = 0.00392699$

$A_{v1} = 157.0796$, is the total area of all stirrups parallel to loading (shear) direction

$h_1 = 750.00$

$s_1 = 100.00$

core: $s_2 = A_{v2} * h_2 / (s_2 * A_g) = 0.00073723$

$A_{v2} = 100.531$, is the total area of all stirrups parallel to loading (shear) direction

$h_2 = 550.00$

$s_2 = 250.00$

The term $2 * t_f / b_w * (f_{fe} / f_s)$ is implemented to account for FRP contribution

where $f = 2 * t_f / b_w$ is FRP ratio (EC8 - 3, A.4.4.3(6)) and f_{fe} / f_s normalises f to steel strength

All these variables have already been given in Shear control ratio calculation.

For the normalisation f_s of jacket is used.

$N_{UD} = 14177.47$

$A_g = 300000.00$

$f_{cE} = (f_{c,jacket} * Area_{jacket} + f_{c,core} * Area_{core}) / section_area = 33.00$

$f_{yE} = (f_{y,ext_Long_Reinf} * Area_{ext_Long_Reinf} + f_{y,int_Long_Reinf} * Area_{int_Long_Reinf}) / Area_{Tot_Long_Rein} = 555.56$

$f_{yE} = (f_{y,ext_Trans_Reinf} * s_1 + f_{y,int_Trans_Reinf} * s_2) / (s_1 + s_2) = 555.56$

$p_l = Area_{Tot_Long_Rein} / (b * d) = 0.00946477$

$b = 400.00$

$d = 707.00$

$f_{cE} = 33.00$

End Of Calculation of Chord Rotation Capacity for element: column JC1 of floor 1

At local axis: 3

Integration Section: (a)

Calculation No. 5

column C1, Floor 1

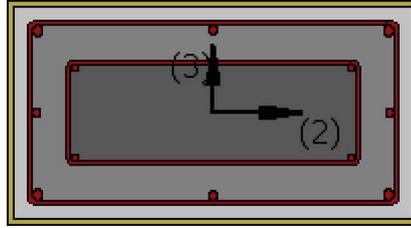
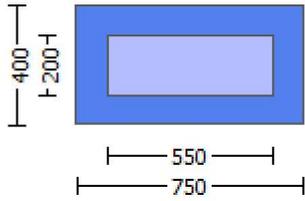
Limit State: Immediate Occupancy (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Shear capacity V_{Rd}

Edge: End

Local Axis: (2)



Start Of Calculation of Shear Capacity for element: column JC1 of floor 1

At local axis: 2

Integration Section: (b)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$

Member Shear Force is generally considered as Force-Controlled Action according to Table C7-1, ASCE 41-17.

Lower-bound strengths are used for Force-Controlled Actions according to 7.5.1.3, ASCE 41-17

Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Primary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Existing Column

New material of Primary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Primary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of γ for displacement ductility demand, the expected (mean value) strengths are used (7.5.1.3, ASCE 41-17) because bending is considered as Deformation-Controlled Action (Table C7-1, ASCE 41-17).

Jacket

New material: Concrete Strength, $f_c = f_{cm} = 33.00$

New material: Steel Strength, $f_s = f_{sm} = 555.56$

Existing Column

New material: Concrete Strength, $f_c = f_{cm} = 33.00$

New material: Steel Strength, $f_s = f_{sm} = 555.56$

#####

External Height, $H = 400.00$

External Width, $W = 750.00$

Internal Height, $H = 200.00$

Internal Width, $W = 550.00$

Cover Thickness, $c = 25.00$

Element Length, $L = 3000.00$

Primary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Inadequate Lap Length with $l_o/l_{ou,min} = l_b/l_d = 0.30$

FRP Wrapping Data

Type: Carbon

Dry properties (design values)

Thickness, $t = 1.00$

Tensile Strength, $f_{fu} = 840.00$

Tensile Modulus, $E_f = 82000.00$

Elongation, $e_{fu} = 0.009$

Number of directions, $NoDir = 1$

Fiber orientations, b_i : 0.00°
Number of layers, NL = 1
Radius of rounding corners, R = 40.00

Stepwise Properties

EDGE -A-
Bending Moment, M_a = -7.0400E+006
Shear Force, V_a = -1995.365
EDGE -B-
Bending Moment, M_b = 1.0488E+006
Shear Force, V_b = 1995.365
BOTH EDGES
Axial Force, F = -14177.47
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: As_t = 0.00
-Compression: As_c = 2676.637
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: $As_{t,ten}$ = 1137.257
-Compression: $As_{c,com}$ = 1137.257
-Middle: As_{mid} = 402.1239
Mean Diameter of Tension Reinforcement, $Db_{L,ten}$ = 16.80

New component: From table 7-7, ASCE 41_17: Final Shear Capacity $V_R = 1.0 \cdot V_n = 1.1000E+006$
 V_n ((10.3), ASCE 41-17) = $k_n \cdot V_{CoI0} = 1.1000E+006$
 $V_{CoI} = 1.1000E+006$
 $k_n = 1.00$
displacement_ductility_demand = 0.02956278

NOTE: In expression (10-3) ' $V_s = A_v \cdot f_y \cdot d / s$ ' is replaced by ' $V_s + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)
Mean concrete strength: $f_c' = (f_c'_{jacket} \cdot Area_{jacket} + f_c'_{core} \cdot Area_{core}) / Area_{section} = 25.00$, but $f_c'^{0.5} \leq 8.3$
MPa (22.5.3.1, ACI 318-14)
 $M/Vd = 2.00$
 $M_u = 1.0488E+006$
 $V_u = 1995.365$
 $d = 0.8 \cdot h = 600.00$
 $N_u = 14177.47$
 $A_g = 300000.00$
From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 559706.147$
where:
 $V_{s1} = 471238.898$ is calculated for jacket, with:
 $d = 600.00$
 $A_v = 157079.633$
 $f_y = 500.00$
 $s = 100.00$
 V_{s1} is multiplied by $Col1 = 1.00$
 $s/d = 0.16666667$
 $V_{s2} = 88467.249$ is calculated for core, with:
 $d = 440.00$
 $A_v = 100530.965$
 $f_y = 500.00$
 $s = 250.00$
 V_{s2} is multiplied by $Col2 = 1.00$
 $s/d = 0.56818182$
 V_f ((11-3)-(11.4), ACI 440) = 463792.00
 $f = 0.95$, for fully-wrapped sections
 $w_f/s_f = 1$ (FRP strips adjacent to one another).
In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,
where a is the angle of the crack direction (see KANEPE).
This later relation, considered as a function $V_f(\theta, \alpha)$, is implemented for every different fiber orientation a_i ,

as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.
orientation 1: $\theta = b1 + 90^\circ = 90.00$
 $V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, \theta)|)$, with:
total thickness per orientation, $t_{f1} = NL * t / \text{NoDir} = 1.00$
 $d_{fv} = d$ (figure 11.2, ACI 440) = 707.00
 f_{fe} ((11-5), ACI 440) = 328.00
 $E_f = 82000.00$
 $f_e = 0.004$, from (11.6a), ACI 440
with $f_u = 0.009$
From (11-11), ACI 440: $V_s + V_f \leq 797164.595$
 $b_w = 400.00$

displacement ductility demand is calculated as δ / y

- Calculation of δ / y for END B -
for rotation axis 3 and integ. section (b)

From analysis, chord rotation $\theta = 1.1916124E-005$
 $y = (M_y * L_s / 3) / E_{eff} = 0.00040308$ ((4.29), Bisquis Phd)
 $M_y = 2.6204E+008$
 $L_s = M/V$ (with $L_s > 0.1 * L$ and $L_s < 2 * L$) = 525.6338
From table 10.5, ASCE 41_17: $E_{eff} = \text{factor} * E_c * I_g = 1.1390E+014$
factor = 0.30
 $A_g = 300000.00$
Mean concrete strength: $f_c' = (f_c'_{jacket} * \text{Area}_{jacket} + f_c'_{core} * \text{Area}_{core}) / \text{Area}_{section} = 33.00$
 $N = 14177.47$
 $E_c * I_g = E_{c,jacket} * I_{g,jacket} + E_{c,core} * I_{g,core} = 3.7968E+014$

Calculation of Yielding Moment M_y

Calculation of δ / y and M_y according to Annex 7 -

$y = \text{Min}(y_{ten}, y_{com})$
 $y_{ten} = 2.8029545E-006$
with ((10.1), ASCE 41-17) $f_y = \text{Min}(f_y, 1.25 * f_y * (I_b / I_d)^{2/3}) = 311.2112$
 $d = 707.00$
 $y = 0.214783$
 $A = 0.00962586$
 $B = 0.0051813$
with $p_t = 0.00402142$
 $p_c = 0.00402142$
 $p_v = 0.00142194$
 $N = 14177.47$
 $b = 400.00$
 $\alpha = 0.06082037$
 $y_{comp} = 1.4829079E-005$
with f_c^* (12.3, (ACI 440)) = 33.35516
 $f_c = 33.00$
 $f_l = 0.77176471$
 $b = 400.00$
 $h = 750.00$
 $A_g = 300000.00$
From (12.9), ACI 440: $k_a = 0.14679369$
 $g = p_t + p_c + p_v = 0.00946477$
 $r_c = 40.00$
 $A_e / A_c = 0.51607158$
Effective FRP thickness, $t_f = NL * t * \text{Cos}(b1) = 1.00$
effective strain from (12.5) and (12.12), $e_{fe} = 0.004$
 $f_u = 0.009$
 $E_f = 82000.00$
 $E_c = 26999.444$
 $y = 0.21210318$
 $A = 0.00935205$

B = 0.00502021
with Es = 200000.00

Calculation of ratio I_b/I_d

Inadequate Lap Length with $I_b/I_d = 0.30$

End Of Calculation of Shear Capacity for element: column JC1 of floor 1

At local axis: 2

Integration Section: (b)

Calculation No. 6

column C1, Floor 1

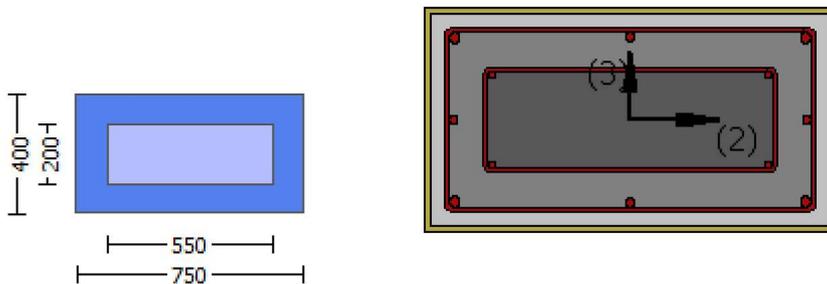
Limit State: Immediate Occupancy (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Chord rotation capacity (θ)

Edge: End

Local Axis: (2)



Start Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At Shear local axis: 3

(Bending local axis: 2)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$

Mean strength values are used for both shear and moment calculations.

Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

```

Existing Column
New material of Primary Member: Concrete Strength,  $f_c = f_{cm} = 33.00$ 
New material of Primary Member: Steel Strength,  $f_s = f_{sm} = 555.56$ 
Concrete Elasticity,  $E_c = 26999.444$ 
Steel Elasticity,  $E_s = 200000.00$ 
#####
Note: Especially for the calculation of moment strengths,
the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14
Jacket
New material: Steel Strength,  $f_s = 1.25 \cdot f_{sm} = 694.45$ 
Existing Column
New material: Steel Strength,  $f_s = 1.25 \cdot f_{sm} = 694.45$ 
#####
External Height,  $H = 400.00$ 
External Width,  $W = 750.00$ 
Internal Height,  $H = 200.00$ 
Internal Width,  $W = 550.00$ 
Cover Thickness,  $c = 25.00$ 
Mean Confinement Factor overall section = 1.0435
Element Length,  $L = 3000.00$ 
Primary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Inadequate Lap Length with  $l_o/l_{ou,min} = 0.30$ 
FRP Wrapping Data
Type: Carbon
Dry properties (design values)
Thickness,  $t = 1.00$ 
Tensile Strength,  $f_{fu} = 840.00$ 
Tensile Modulus,  $E_f = 82000.00$ 
Elongation,  $e_{fu} = 0.009$ 
Number of directions,  $NoDir = 1$ 
Fiber orientations,  $bi: 0.00^\circ$ 
Number of layers,  $NL = 1$ 
Radius of rounding corners,  $R = 40.00$ 
-----

Stepwise Properties
-----
At local axis: 3
EDGE -A-
Shear Force,  $V_a = 3.4586346E-031$ 
EDGE -B-
Shear Force,  $V_b = -3.4586346E-031$ 
BOTH EDGES
Axial Force,  $F = -11016.808$ 
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension:  $A_{sl,t} = 0.00$ 
-Compression:  $A_{sl,c} = 2676.637$ 
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension:  $A_{sl,ten} = 1137.257$ 
-Compression:  $A_{sl,com} = 1137.257$ 
-Middle:  $A_{sl,mid} = 402.1239$ 
-----
-----

Calculation of Shear Capacity ratio ,  $V_e/V_r = 0.14265244$ 
Member Controlled by Flexure ( $V_e/V_r < 1$ )
Calculation of Shear Demand from fig. R18.6.5, ACI 318-14  $V_e = (M_{pr1} + M_{pr2})/l_n = 121055.918$ 
with
 $M_{pr1} = \text{Max}(M_{u1+} , M_{u1-}) = 1.8158E+008$ 
 $M_{u1+} = 1.8158E+008$ , is the ultimate moment strength at the edge 1 of the member in the actual moment direction
which is defined for the static loading combination
 $M_{u1-} = 1.8158E+008$ , is the ultimate moment strength at the edge 1 of the member in the opposite moment

```

direction which is defined for the static loading combination

$$M_{pr2} = \text{Max}(M_{u2+}, M_{u2-}) = 1.8158E+008$$

$M_{u2+} = 1.8158E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the the static loading combination

$M_{u2-} = 1.8158E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the the static loading combination

Calculation of M_{u1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$$\phi_u = 1.7255512E-005$$

$$M_u = 1.8158E+008$$

with full section properties:

$$b = 750.00$$

$$d = 357.00$$

$$d' = 43.00$$

$$v = 0.00124684$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$\omega (5A.5, \text{TBDY}) = 0.002$$

$$\text{Final value of } \phi_{cu}: \phi_{cu}^* = \text{shear_factor} * \text{Max}(\phi_{cu}, \phi_{cc}) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \phi_{cu} = 0.01006403$$

$$\phi_{we} ((5.4c), \text{TBDY}) = a_{se} * \phi_{sh, \min} * f_{ywe} / f_{ce} + \text{Min}(\phi_{fx}, \phi_{fy}) = 0.02692906$$

where $\phi_f = a_f * \rho_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\phi_{fx} = 0.02504965$$

$$a_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } \rho_f = 2t_f / b_w = 0.00266667$$

$$b_w = 750.00$$

$$\text{effective stress from (A.35), } f_{fe} = 800.0873$$

$$\phi_{fy} = 0.04491794$$

$$a_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } \rho_f = 2t_f / b_w = 0.005$$

$$b_w = 400.00$$

$$\text{effective stress from (A.35), } f_{fe} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L * t * \text{Cos}(b_1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{,f} = 0.015$$

$$a_{se} ((5.4d), \text{TBDY}) = (a_{se1} * A_{ext} + a_{se2} * A_{int}) / A_{sec} = 0.12601038$$

$$a_{se1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i2,1} = 1.1834E+006$$

$$a_{se2} = \text{Max}(a_{se1}, a_{se2}) = 0.12601038$$

$$b_{o,2} = 542.00$$

$$h_{o,2} = 192.00$$

$$b_{i2,2} = 661256.00$$

$$\phi_{psh, \min} * f_{ywe} = \text{Min}(\phi_{psh, x} * f_{ywe}, \phi_{psh, y} * f_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $\phi_{psh, \min} * f_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\phi_{psh, x} * f_{ywe} = \phi_{psh1} * f_{ywe1} + \phi_{ps2} * f_{ywe2} = 1.64062$$

$$\phi_{ps1} (\text{external}) = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$$

$$A_{sh1} = A_{stir,1} * n_{s,1} = 157.0796$$

No stirups, ns₁ = 2.00
h₁ = 400.00
ps₂ (internal) = (Ash₂*h₂/s₂)/Asec = 0.00026808
Ash₂ = Astir₂*ns₂ = 100.531
No stirups, ns₂ = 2.00
h₂ = 200.00

psh_y*Fywe = psh₁*Fywe₁+ps₂*Fywe₂ = 3.23907
ps₁ (external) = (Ash₁*h₁/s₁)/Asec = 0.00392699
Ash₁ = Astir₁*ns₁ = 157.0796
No stirups, ns₁ = 2.00
h₁ = 750.00
ps₂ (internal) = (Ash₂*h₂/s₂)/Asec = 0.00073723
Ash₂ = Astir₂*ns₂ = 100.531
No stirups, ns₂ = 2.00
h₂ = 550.00

Asec = 300000.00
s₁ = 100.00
s₂ = 250.00
fywe₁ = 694.45
fywe₂ = 694.45
fce = 33.00

From ((5.A.5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y₁ = 0.00140044
sh₁ = 0.0044814
ft₁ = 466.8167
fy₁ = 389.0139
su₁ = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/l_d = 0.30

su₁ = 0.4*esu_{1_nominal} ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu_{1_nominal} = 0.08,

For calculation of esu_{1_nominal} and y₁, sh₁,ft₁,fy₁, it is considered
characteristic value fsy₁ = fs₁/1.2, from table 5.1, TBDY.

y₁, sh₁,ft₁,fy₁, are also multiplied by Min(1,1.25*(lb/l_d)^{2/3}), from 10.3.5, ASCE 41-17.

with fs₁ = (fs_{jacket}*Asl_{ten,jacket} + fs_{core}*Asl_{ten,core})/Asl_{ten} = 389.0139

with Es₁ = (Es_{jacket}*Asl_{ten,jacket} + Es_{core}*Asl_{ten,core})/Asl_{ten} = 200000.00

y₂ = 0.00140044
sh₂ = 0.0044814
ft₂ = 466.8167
fy₂ = 389.0139
su₂ = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/l_{b,min} = 0.30

su₂ = 0.4*esu_{2_nominal} ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu_{2_nominal} = 0.08,

For calculation of esu_{2_nominal} and y₂, sh₂,ft₂,fy₂, it is considered
characteristic value fsy₂ = fs₂/1.2, from table 5.1, TBDY.

y₂, sh₂,ft₂,fy₂, are also multiplied by Min(1,1.25*(lb/l_d)^{2/3}), from 10.3.5, ASCE 41-17.

with fs₂ = (fs_{jacket}*Asl_{com,jacket} + fs_{core}*Asl_{com,core})/Asl_{com} = 389.0139

with Es₂ = (Es_{jacket}*Asl_{com,jacket} + Es_{core}*Asl_{com,core})/Asl_{com} = 200000.00

y_v = 0.00140044
sh_v = 0.0044814
ft_v = 466.8167
fy_v = 389.0139
su_v = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/l_d = 0.30

su_v = 0.4*esuv_{nominal} ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: $e_{suv_nominal} = 0.08$,
considering characteristic value $f_{sv} = f_{sv}/1.2$, from table 5.1, TBDY
For calculation of $e_{suv_nominal}$ and y_v , sh_v , ft_v , fy_v , it is considered
characteristic value $f_{sv} = f_{sv}/1.2$, from table 5.1, TBDY.

y_1 , sh_1 , ft_1 , fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $f_{sv} = (f_{s,jacket} \cdot A_{s1,mid,jacket} + f_{s,mid} \cdot A_{s1,mid,core}) / A_{s1,mid} = 389.0139$

with $E_{sv} = (E_{s,jacket} \cdot A_{s1,mid,jacket} + E_{s,mid} \cdot A_{s1,mid,core}) / A_{s1,mid} = 200000.00$

$1 = A_{s1,ten} / (b \cdot d) \cdot (f_{s1} / f_c) = 0.0500703$

$2 = A_{s1,com} / (b \cdot d) \cdot (f_{s2} / f_c) = 0.0500703$

$v = A_{s1,mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.01770442$

and confined core properties:

$b = 690.00$

$d = 327.00$

$d' = 13.00$

f_{cc} (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

$1 = A_{s1,ten} / (b \cdot d) \cdot (f_{s1} / f_c) = 0.05941729$

$2 = A_{s1,com} / (b \cdot d) \cdot (f_{s2} / f_c) = 0.05941729$

$v = A_{s1,mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.02100943$

Case/Assumption: Unconfined full section - Steel rupture
' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

su (4.9) = 0.16886055

$Mu = MRc$ (4.14) = 1.8158E+008

$u = su$ (4.1) = 1.7255512E-005

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of Mu_1 -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 1.7255512E-005$

$Mu = 1.8158E+008$

with full section properties:

$b = 750.00$

$d = 357.00$

$d' = 43.00$

$v = 0.00124684$

$N = 11016.808$

$f_c = 33.00$

co (5A.5, TBDY) = 0.002

Final value of cu : $cu^* = \text{shear_factor} \cdot \text{Max}(cu, cc) = 0.01006403$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $cu = 0.01006403$

where ((5.4c), TBDY) = $ase \cdot sh_{\min} \cdot fy_{we} / f_{ce} + \text{Min}(fx, fy) = 0.02692906$

where $f = af \cdot pf \cdot f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

 $fx = 0.02504965$

$af = 0.38744444$

$b = 750.00$

$h = 400.00$

From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.00266667$

$bw = 750.00$

effective stress from (A.35), $f_{fe} = 800.0873$

$fy = 0.04491794$

af = 0.38744444
b = 400.00
h = 750.00
From EC8 A.4.4.3(6), pf = 2tf/bw = 0.005
bw = 400.00
effective stress from (A.35), ff,e = 765.1636

R = 40.00
Effective FRP thickness, tf = NL*t*Cos(b1) = 1.00
fu,f = 840.00
Ef = 82000.00
u,f = 0.015
ase ((5.4d), TBDY) = (ase1*Aext+ase2*Aint)/Asec = 0.12601038
ase1 = 0.12601038
bo_1 = 690.00
ho_1 = 340.00
bi2_1 = 1.1834E+006
ase2 = Max(ase1,ase2) = 0.12601038
bo_2 = 542.00
ho_2 = 192.00
bi2_2 = 661256.00
psh,min*Fywe = Min(psh,x*Fywe , psh,y*Fywe) = 1.64062

Expression ((5.4d), TBDY) for psh,min*Fywe has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh_x*Fywe = psh1*Fywe1+ps2*Fywe2 = 1.64062
ps1 (external) = (Ash1*h1/s1)/Asec = 0.0020944
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 400.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00026808
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 200.00

psh_y*Fywe = psh1*Fywe1+ps2*Fywe2 = 3.23907
ps1 (external) = (Ash1*h1/s1)/Asec = 0.00392699
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 750.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00073723
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/d = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^ 2/3), from 10.3.5, ASCE 41-17.

with $f_{s1} = (f_{s,jacket} \cdot A_{s,ten,jacket} + f_{s,core} \cdot A_{s,ten,core}) / A_{s,ten} = 389.0139$
 with $E_{s1} = (E_{s,jacket} \cdot A_{s,ten,jacket} + E_{s,core} \cdot A_{s,ten,core}) / A_{s,ten} = 200000.00$
 $y_2 = 0.00140044$
 $sh_2 = 0.0044814$
 $ft_2 = 466.8167$
 $fy_2 = 389.0139$
 $su_2 = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_o/l_{ou,min} = l_b/l_{b,min} = 0.30$
 $su_2 = 0.4 \cdot esu_{2,nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu_{2,nominal} = 0.08$,
 For calculation of $esu_{2,nominal}$ and y_2, sh_2, ft_2, fy_2 , it is considered
 characteristic value $f_{sy2} = f_{s2}/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $f_{s2} = (f_{s,jacket} \cdot A_{s,com,jacket} + f_{s,core} \cdot A_{s,com,core}) / A_{s,com} = 389.0139$
 with $E_{s2} = (E_{s,jacket} \cdot A_{s,com,jacket} + E_{s,core} \cdot A_{s,com,core}) / A_{s,com} = 200000.00$
 $y_v = 0.00140044$
 $sh_v = 0.0044814$
 $ft_v = 466.8167$
 $fy_v = 389.0139$
 $su_v = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_o/l_{ou,min} = l_b/l_d = 0.30$
 $su_v = 0.4 \cdot esu_{v,nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu_{v,nominal} = 0.08$,
 considering characteristic value $f_{syv} = f_{sv}/1.2$, from table 5.1, TBDY
 For calculation of $esu_{v,nominal}$ and y_v, sh_v, ft_v, fy_v , it is considered
 characteristic value $f_{syv} = f_{sv}/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $f_{sv} = (f_{s,jacket} \cdot A_{s,mid,jacket} + f_{s,mid} \cdot A_{s,mid,core}) / A_{s,mid} = 389.0139$
 with $E_{sv} = (E_{s,jacket} \cdot A_{s,mid,jacket} + E_{s,mid} \cdot A_{s,mid,core}) / A_{s,mid} = 200000.00$
 $1 = A_{s,ten} / (b \cdot d) \cdot (f_{s1} / f_c) = 0.0500703$
 $2 = A_{s,com} / (b \cdot d) \cdot (f_{s2} / f_c) = 0.0500703$
 $v = A_{s,mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.01770442$
 and confined core properties:
 $b = 690.00$
 $d = 327.00$
 $d' = 13.00$
 $f_{cc} (5A.2, TBDY) = 34.43547$
 $cc (5A.5, TBDY) = 0.00243499$
 $c = \text{confinement factor} = 1.0435$
 $1 = A_{s,ten} / (b \cdot d) \cdot (f_{s1} / f_c) = 0.05941729$
 $2 = A_{s,com} / (b \cdot d) \cdot (f_{s2} / f_c) = 0.05941729$
 $v = A_{s,mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.02100943$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)
 --->
 $v < v_{s,y2}$ - LHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.16886055$
 $Mu = MRc (4.14) = 1.8158E+008$
 $u = su (4.1) = 1.7255512E-005$

 Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of Mu_{2+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$$u = 1.7255512E-005$$

$$Mu = 1.8158E+008$$

with full section properties:

$$b = 750.00$$

$$d = 357.00$$

$$d' = 43.00$$

$$v = 0.00124684$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$\alpha_{co} (5A.5, TBDY) = 0.002$$

$$\text{Final value of } \phi_u: \phi_u^* = \text{shear_factor} * \text{Max}(\phi_{cu}, \phi_{cc}) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \phi_{cu} = 0.01006403$$

$$\phi_{we} ((5.4c), TBDY) = \alpha_{se} * \text{sh}_{,min} * f_{ywe} / f_{ce} + \text{Min}(\phi_{fx}, \phi_{fy}) = 0.02692906$$

where $\phi_f = \alpha_f * \rho_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\phi_{fx} = 0.02504965$$

$$\alpha_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A 4.4.3(6), } \rho_f = 2t_f/bw = 0.00266667$$

$$bw = 750.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 800.0873$$

$$\phi_{fy} = 0.04491794$$

$$\alpha_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A 4.4.3(6), } \rho_f = 2t_f/bw = 0.005$$

$$bw = 400.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL * t * \text{Cos}(b1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{,f} = 0.015$$

$$\alpha_{se} ((5.4d), TBDY) = (\alpha_{se1} * A_{ext} + \alpha_{se2} * A_{int}) / A_{sec} = 0.12601038$$

$$\alpha_{se1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i2,1} = 1.1834E+006$$

$$\alpha_{se2} = \text{Max}(\alpha_{se1}, \alpha_{se2}) = 0.12601038$$

$$b_{o,2} = 542.00$$

$$h_{o,2} = 192.00$$

$$b_{i2,2} = 661256.00$$

$$\text{psh}_{,min} * F_{ywe} = \text{Min}(\text{psh}_{,x} * F_{ywe}, \text{psh}_{,y} * F_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $\text{psh}_{,min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\text{psh}_{,x} * F_{ywe} = \text{psh}_1 * F_{ywe1} + \text{ps}_2 * F_{ywe2} = 1.64062$$

$$\text{ps}_1 (\text{external}) = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$$

$$A_{sh1} = A_{stir_1} * n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

$$h_1 = 400.00$$

$$\text{ps}_2 (\text{internal}) = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00026808$$

$$A_{sh2} = A_{stir_2} * n_{s,2} = 100.531$$

$$\text{No stirrups, } n_{s,2} = 2.00$$

$$h_2 = 200.00$$

$$\text{psh}_{,y} * F_{ywe} = \text{psh}_1 * F_{ywe1} + \text{ps}_2 * F_{ywe2} = 3.23907$$

$$\text{ps}_1 (\text{external}) = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.00392699$$

$$A_{sh1} = A_{stir_1} * n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

h1 = 750.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00073723
Ash2 = Astir_2*ns_2 = 100.531
No stirups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/d = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044
sh2 = 0.0044814
ft2 = 466.8167
fy2 = 389.0139
su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044
shv = 0.0044814
ftv = 466.8167
fyv = 389.0139
suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/d = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.0500703
2 = Asl,com/(b*d)*(fs2/fc) = 0.0500703

v = Asl,mid/(b*d)*(fsv/fc) = 0.01770442

and confined core properties:

b = 690.00
d = 327.00
d' = 13.00
fcc (5A.2, TBDY) = 34.43547
cc (5A.5, TBDY) = 0.00243499
c = confinement factor = 1.0435
1 = $Asl_{ten}/(b*d)*(fs1/fc) = 0.05941729$
2 = $Asl_{com}/(b*d)*(fs2/fc) = 0.05941729$
v = $Asl_{mid}/(b*d)*(fsv/fc) = 0.02100943$

Case/Assumption: Unconfined full section - Steel rupture
' satisfies Eq. (4.3)

--->
v < vs,y2 - LHS eq.(4.5) is satisfied
--->
su (4.9) = 0.16886055
Mu = MRc (4.14) = 1.8158E+008
u = su (4.1) = 1.7255512E-005

Calculation of ratio lb/l_d

Inadequate Lap Length with lb/l_d = 0.30

Calculation of Mu₂-

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

u = 1.7255512E-005
Mu = 1.8158E+008

with full section properties:

b = 750.00
d = 357.00
d' = 43.00
v = 0.00124684
N = 11016.808
fc = 33.00
co (5A.5, TBDY) = 0.002
Final value of cu: $cu^* = shear_factor * Max(cu, cc) = 0.01006403$
The Shear_factor is considered equal to 1 (pure moment strength)
From (5.4b), TBDY: $cu = 0.01006403$
 $w_e ((5.4c), TBDY) = ase * sh_{min} * fy_{we} / f_{ce} + Min(f_x, f_y) = 0.02692906$
where $f = af * pf * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

fx = 0.02504965
af = 0.38744444
b = 750.00
h = 400.00
From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.00266667$
bw = 750.00
effective stress from (A.35), $ff_e = 800.0873$

fy = 0.04491794
af = 0.38744444
b = 400.00
h = 750.00
From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.005$
bw = 400.00
effective stress from (A.35), $ff_e = 765.1636$

R = 40.00
Effective FRP thickness, $tf = NL * t * Cos(b1) = 1.00$
fu,f = 840.00
Ef = 82000.00

$$u,f = 0.015$$

$$ase \text{ ((5.4d), TBDY)} = (ase1 \cdot A_{ext} + ase2 \cdot A_{int}) / A_{sec} = 0.12601038$$

$$ase1 = 0.12601038$$

$$bo_1 = 690.00$$

$$ho_1 = 340.00$$

$$bi2_1 = 1.1834E+006$$

$$ase2 = \text{Max}(ase1, ase2) = 0.12601038$$

$$bo_2 = 542.00$$

$$ho_2 = 192.00$$

$$bi2_2 = 661256.00$$

$$psh_{min} \cdot F_{ywe} = \text{Min}(psh_x \cdot F_{ywe}, psh_y \cdot F_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $psh_{min} \cdot F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$psh_x \cdot F_{ywe} = psh1 \cdot F_{ywe1} + ps2 \cdot F_{ywe2} = 1.64062$$

$$ps1 \text{ (external)} = (Ash1 \cdot h1 / s1) / A_{sec} = 0.0020944$$

$$Ash1 = Astir_1 \cdot ns_1 = 157.0796$$

$$\text{No stirrups, } ns_1 = 2.00$$

$$h1 = 400.00$$

$$ps2 \text{ (internal)} = (Ash2 \cdot h2 / s2) / A_{sec} = 0.00026808$$

$$Ash2 = Astir_2 \cdot ns_2 = 100.531$$

$$\text{No stirrups, } ns_2 = 2.00$$

$$h2 = 200.00$$

$$psh_y \cdot F_{ywe} = psh1 \cdot F_{ywe1} + ps2 \cdot F_{ywe2} = 3.23907$$

$$ps1 \text{ (external)} = (Ash1 \cdot h1 / s1) / A_{sec} = 0.00392699$$

$$Ash1 = Astir_1 \cdot ns_1 = 157.0796$$

$$\text{No stirrups, } ns_1 = 2.00$$

$$h1 = 750.00$$

$$ps2 \text{ (internal)} = (Ash2 \cdot h2 / s2) / A_{sec} = 0.00073723$$

$$Ash2 = Astir_2 \cdot ns_2 = 100.531$$

$$\text{No stirrups, } ns_2 = 2.00$$

$$h2 = 550.00$$

$$A_{sec} = 300000.00$$

$$s1 = 100.00$$

$$s2 = 250.00$$

$$fywe1 = 694.45$$

$$fywe2 = 694.45$$

$$fce = 33.00$$

$$\text{From ((5.A5), TBDY), TBDY: } cc = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$y1 = 0.00140044$$

$$sh1 = 0.0044814$$

$$ft1 = 466.8167$$

$$fy1 = 389.0139$$

$$su1 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$$lo/lou_{min} = lb/ld = 0.30$$

$$su1 = 0.4 \cdot esu1_{nominal} \text{ ((5.5), TBDY)} = 0.032$$

$$\text{From table 5A.1, TBDY: } esu1_{nominal} = 0.08,$$

For calculation of $esu1_{nominal}$ and $y1, sh1, ft1, fy1$, it is considered characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$$y1, sh1, ft1, fy1, \text{ are also multiplied by } \text{Min}(1, 1.25 \cdot (lb/ld)^{2/3}), \text{ from 10.3.5, ASCE 41-17.}$$

$$\text{with } fs1 = (fs_{jacket} \cdot Asl_{ten,jacket} + fs_{core} \cdot Asl_{ten,core}) / Asl_{ten} = 389.0139$$

$$\text{with } Es1 = (Es_{jacket} \cdot Asl_{ten,jacket} + Es_{core} \cdot Asl_{ten,core}) / Asl_{ten} = 200000.00$$

$$y2 = 0.00140044$$

$$sh2 = 0.0044814$$

$$ft2 = 466.8167$$

$$fy2 = 389.0139$$

$$su2 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$$lo/lou_{min} = lb/lb_{min} = 0.30$$

$$su_2 = 0.4 * esu_{2_nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $esu_{2_nominal} = 0.08$,

For calculation of $esu_{2_nominal}$ and y_2, sh_2, ft_2, fy_2 , it is considered characteristic value $fsy_2 = fs_2/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fs_2 = (fs_{jacket} * Asl_{,com,jacket} + fs_{core} * Asl_{,com,core}) / Asl_{,com} = 389.0139$$

$$\text{with } Es_2 = (Es_{jacket} * Asl_{,com,jacket} + Es_{core} * Asl_{,com,core}) / Asl_{,com} = 200000.00$$

$$yv = 0.00140044$$

$$shv = 0.0044814$$

$$ftv = 466.8167$$

$$fyv = 389.0139$$

$$suv = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$$lo/lo_{u,min} = lb/ld = 0.30$$

$$suv = 0.4 * esuv_{nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $esuv_{nominal} = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
For calculation of $esuv_{nominal}$ and yv, shv, ftv, fyv , it is considered

characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fsv = (fs_{jacket} * Asl_{,mid,jacket} + fs_{mid} * Asl_{,mid,core}) / Asl_{,mid} = 389.0139$$

$$\text{with } Esv = (Es_{jacket} * Asl_{,mid,jacket} + Es_{mid} * Asl_{,mid,core}) / Asl_{,mid} = 200000.00$$

$$1 = Asl_{,ten} / (b * d) * (fs_1 / fc) = 0.0500703$$

$$2 = Asl_{,com} / (b * d) * (fs_2 / fc) = 0.0500703$$

$$v = Asl_{,mid} / (b * d) * (fsv / fc) = 0.01770442$$

and confined core properties:

$$b = 690.00$$

$$d = 327.00$$

$$d' = 13.00$$

$$fcc (5A.2, TBDY) = 34.43547$$

$$cc (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = Asl_{,ten} / (b * d) * (fs_1 / fc) = 0.05941729$$

$$2 = Asl_{,com} / (b * d) * (fs_2 / fc) = 0.05941729$$

$$v = Asl_{,mid} / (b * d) * (fsv / fc) = 0.02100943$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$su (4.9) = 0.16886055$$

$$\mu_u = MR_c (4.14) = 1.8158E+008$$

$$u = su (4.1) = 1.7255512E-005$$

Calculation of ratio lb/ld

Inadequate Lap Length with $lb/ld = 0.30$

Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 848607.437$

Calculation of Shear Strength at edge 1, $V_{r1} = 848607.437$

$$V_{r1} = V_{CoI} ((10.3), ASCE 41-17) = knl * V_{CoI0}$$

$$V_{CoI0} = 848607.437$$

$$knl = 1 \text{ (zero step-static loading)}$$

NOTE: In expression (10-3) ' $V_s = A_v * f_y * d / s$ ' is replaced by ' $V_s + f * V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

$$= 1 \text{ (normal-weight concrete)}$$

Mean concrete strength: $fc' = (fc'_{jacket} * Area_{jacket} + fc'_{core} * Area_{core}) / Area_{section} = 33.00$, but $fc'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$M/Vd = 2.00$
 $\mu_u = 3.5345895E-012$
 $\mu_v = 3.4586346E-031$
 $d = 0.8 \cdot h = 320.00$
 $N_u = 11016.808$
 $A_g = 300000.00$
 From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 279254.914$
 where:
 $V_{s1} = 279254.914$ is calculated for jacket, with:
 $d = 320.00$
 $A_v = 157079.633$
 $f_y = 555.56$
 $s = 100.00$
 V_{s1} is multiplied by $Col1 = 1.00$
 $s/d = 0.3125$
 $V_{s2} = 0.00$ is calculated for core, with:
 $d = 160.00$
 $A_v = 100530.965$
 $f_y = 555.56$
 $s = 250.00$
 V_{s2} is multiplied by $Col2 = 0.00$
 $s/d = 1.5625$
 $V_f ((11-3)-(11.4), ACI 440) = 234192.00$
 $f = 0.95$, for fully-wrapped sections
 $w_f/s_f = 1$ (FRP strips adjacent to one another).
 In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,
 where a is the angle of the crack direction (see KANEPE).
 This later relation, considered as a function $V_f(\theta, a)$, is implemented for every different fiber orientation a_i ,
 as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.
 orientation 1: $\theta = b1 + 90^\circ = 90.00$
 $V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, a1)|)$, with:
 total thickness per orientation, $t_{f1} = NL \cdot t / N_{oDir} = 1.00$
 $d_{fv} = d$ (figure 11.2, ACI 440) = 357.00
 $f_{fe} ((11-5), ACI 440) = 328.00$
 $E_f = 82000.00$
 $f_e = 0.004$, from (11.6a), ACI 440
 with $f_u = 0.009$
 From (11-11), ACI 440: $V_s + V_f \leq 915872.391$
 $b_w = 750.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 848607.437$
 $V_{r2} = V_{Col} ((10.3), ASCE 41-17) = knl \cdot V_{Col0}$
 $V_{Col0} = 848607.437$
 $knl = 1$ (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v \cdot f_y \cdot d / s$ ' is replaced by ' $V_s + f \cdot V_f$ '
 where V_f is the contribution of FRPs (11.3), ACI 440).

$\lambda = 1$ (normal-weight concrete)
 Mean concrete strength: $f_c' = (f_c'_{jacket} \cdot Area_{jacket} + f_c'_{core} \cdot Area_{core}) / Area_{section} = 33.00$, but $f_c'^{0.5} \leq 8.3$
 MPa (22.5.3.1, ACI 318-14)

$M/Vd = 2.00$
 $\mu_u = 3.5345895E-012$
 $\mu_v = 3.4586346E-031$
 $d = 0.8 \cdot h = 320.00$
 $N_u = 11016.808$
 $A_g = 300000.00$
 From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 279254.914$
 where:
 $V_{s1} = 279254.914$ is calculated for jacket, with:
 $d = 320.00$
 $A_v = 157079.633$
 $f_y = 555.56$
 $s = 100.00$
 V_{s1} is multiplied by $Col1 = 1.00$
 $s/d = 0.3125$

Vs2 = 0.00 is calculated for core, with:

d = 160.00

Av = 100530.965

fy = 555.56

s = 250.00

Vs2 is multiplied by Col2 = 0.00

s/d = 1.5625

Vf ((11-3)-(11.4), ACI 440) = 234192.00

f = 0.95, for fully-wrapped sections

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression,

where θ is the angle of the crack direction (see KANEPE).

This later relation, considered as a function Vf(θ), is implemented for every different fiber orientation ai, as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta_1 = b1 + 90^\circ = 90.00$

Vf = Min(|Vf(45, θ_1)|, |Vf(-45, θ_1)|), with:

total thickness per orientation, tf1 = NL*t/NoDir = 1.00

dfv = d (figure 11.2, ACI 440) = 357.00

ffe ((11-5), ACI 440) = 328.00

Ef = 82000.00

fe = 0.004, from (11.6a), ACI 440

with fu = 0.009

From (11-11), ACI 440: Vs + Vf <= 915872.391

bw = 750.00

End Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At local axis: 3

Start Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At Shear local axis: 2

(Bending local axis: 3)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$

Mean strength values are used for both shear and moment calculations.

Consequently:

Jacket

New material of Primary Member: Concrete Strength, fc = fcm = 33.00

New material of Primary Member: Steel Strength, fs = fsm = 555.56

Concrete Elasticity, Ec = 26999.444

Steel Elasticity, Es = 200000.00

Existing Column

New material of Primary Member: Concrete Strength, fc = fcm = 33.00

New material of Primary Member: Steel Strength, fs = fsm = 555.56

Concrete Elasticity, Ec = 26999.444

Steel Elasticity, Es = 200000.00

#####

Note: Especially for the calculation of moment strengths,

the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

Jacket

New material: Steel Strength, fs = 1.25*fsm = 694.45

Existing Column

New material: Steel Strength, fs = 1.25*fsm = 694.45

#####

External Height, H = 400.00

External Width, W = 750.00

Internal Height, H = 200.00

Internal Width, W = 550.00

Cover Thickness, c = 25.00

Mean Confinement Factor overall section = 1.0435

Element Length, L = 3000.00

Primary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Inadequate Lap Length with $l_o/l_{o,min} = 0.30$
FRP Wrapping Data
Type: Carbon
Dry properties (design values)
Thickness, $t = 1.00$
Tensile Strength, $f_{fu} = 840.00$
Tensile Modulus, $E_f = 82000.00$
Elongation, $e_{fu} = 0.009$
Number of directions, $NoDir = 1$
Fiber orientations, $bi = 0.00^\circ$
Number of layers, $NL = 1$
Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 2
EDGE -A-
Shear Force, $V_a = 2.9582284E-031$
EDGE -B-
Shear Force, $V_b = -2.9582284E-031$
BOTH EDGES
Axial Force, $F = -11016.808$
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: $As_t = 0.00$
-Compression: $As_c = 2676.637$
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: $As_{t,ten} = 1137.257$
-Compression: $As_{c,com} = 1137.257$
-Middle: $As_{c,mid} = 402.1239$

Calculation of Shear Capacity ratio , $V_e/V_r = 0.19671905$
Member Controlled by Flexure ($V_e/V_r < 1$)
Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 248405.514$
with

$M_{pr1} = \text{Max}(Mu_{1+} , Mu_{1-}) = 3.7261E+008$

$Mu_{1+} = 3.7261E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination

$Mu_{1-} = 3.7261E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination

$M_{pr2} = \text{Max}(Mu_{2+} , Mu_{2-}) = 3.7261E+008$

$Mu_{2+} = 3.7261E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the the static loading combination

$Mu_{2-} = 3.7261E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the the static loading combination

Calculation of Mu_{1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$\phi_u = 8.5888248E-006$

$M_u = 3.7261E+008$

with full section properties:

$b = 400.00$

$d = 707.00$

$d' = 43.00$

$$v = 0.00118049$$

$$N = 11016.808$$

$$fc = 33.00$$

$$co \text{ (5A.5, TBDY)} = 0.002$$

$$\text{Final value of } cu: cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } cu = 0.01006403$$

$$\text{we ((5.4c), TBDY)} = ase^* sh_{,min} * fy_{we} / f_{ce} + \text{Min}(fx, fy) = 0.02692906$$

where $f = af * pf * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$fx = 0.02504965$$

$$af = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } pf = 2tf/bw = 0.00266667$$

$$bw = 750.00$$

$$\text{effective stress from (A.35), } ff,e = 800.0873$$

$$fy = 0.04491794$$

$$af = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } pf = 2tf/bw = 0.005$$

$$bw = 400.00$$

$$\text{effective stress from (A.35), } ff,e = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } tf = NL * t * \text{Cos}(b1) = 1.00$$

$$fu,f = 840.00$$

$$Ef = 82000.00$$

$$u,f = 0.015$$

$$ase \text{ ((5.4d), TBDY)} = (ase1 * A_{ext} + ase2 * A_{int}) / A_{sec} = 0.12601038$$

$$ase1 = 0.12601038$$

$$bo_1 = 690.00$$

$$ho_1 = 340.00$$

$$bi2_1 = 1.1834E+006$$

$$ase2 = \text{Max}(ase1, ase2) = 0.12601038$$

$$bo_2 = 542.00$$

$$ho_2 = 192.00$$

$$bi2_2 = 661256.00$$

$$psh_{,min} * Fy_{we} = \text{Min}(psh_{,x} * Fy_{we}, psh_{,y} * Fy_{we}) = 1.64062$$

Expression ((5.4d), TBDY) for $psh_{,min} * Fy_{we}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$psh_{,x} * Fy_{we} = psh1 * Fy_{we1} + ps2 * Fy_{we2} = 1.64062$$

$$ps1 \text{ (external)} = (Ash1 * h1 / s1) / A_{sec} = 0.0020944$$

$$Ash1 = Astir_1 * ns_1 = 157.0796$$

$$\text{No stirrups, } ns_1 = 2.00$$

$$h1 = 400.00$$

$$ps2 \text{ (internal)} = (Ash2 * h2 / s2) / A_{sec} = 0.00026808$$

$$Ash2 = Astir_2 * ns_2 = 100.531$$

$$\text{No stirrups, } ns_2 = 2.00$$

$$h2 = 200.00$$

$$psh_{,y} * Fy_{we} = psh1 * Fy_{we1} + ps2 * Fy_{we2} = 3.23907$$

$$ps1 \text{ (external)} = (Ash1 * h1 / s1) / A_{sec} = 0.00392699$$

$$Ash1 = Astir_1 * ns_1 = 157.0796$$

$$\text{No stirrups, } ns_1 = 2.00$$

$$h1 = 750.00$$

$$ps2 \text{ (internal)} = (Ash2 * h2 / s2) / A_{sec} = 0.00073723$$

$$Ash2 = Astir_2 * ns_2 = 100.531$$

$$\text{No stirrups, } ns_2 = 2.00$$

$$h2 = 550.00$$

$$A_{sec} = 300000.00$$

$$s1 = 100.00$$

$$s2 = 250.00$$

fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A.5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044
sh2 = 0.0044814
ft2 = 466.8167
fy2 = 389.0139
su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044
shv = 0.0044814
ftv = 466.8167
fyv = 389.0139
suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.04740567

2 = Asl,com/(b*d)*(fs2/fc) = 0.04740567

v = Asl,mid/(b*d)*(fsv/fc) = 0.01676222

and confined core properties:

b = 340.00
d = 677.00
d' = 13.00

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

1 = Asl,ten/(b*d)*(fs1/fc) = 0.05824278

2 = Asl,com/(b*d)*(fs2/fc) = 0.05824278

v = Asl,mid/(b*d)*(fsv/fc) = 0.02059413

Case/Assumption: Unconfined full section - Steel rupture
' satisfies Eq. (4.3)

--->
v < vs,y2 - LHS eq.(4.5) is satisfied

--->
su (4.9) = 0.15682678
Mu = MRc (4.14) = 3.7261E+008
u = su (4.1) = 8.5888248E-006

Calculation of ratio lb/l_d

Inadequate Lap Length with lb/l_d = 0.30

Calculation of Mu1-

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:
u = 8.5888248E-006
Mu = 3.7261E+008

with full section properties:

b = 400.00
d = 707.00
d' = 43.00
v = 0.00118049
N = 11016.808
f_c = 33.00
co (5A.5, TBDY) = 0.002

Final value of cu: cu* = shear_factor * Max(cu, cc) = 0.01006403
The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: cu = 0.01006403

we ((5.4c), TBDY) = ase* sh,min*fywe/fce+ Min(fx, fy) = 0.02692906

where f = af*pf*ffe/fce is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

fx = 0.02504965
af = 0.38744444
b = 750.00
h = 400.00

From EC8 A4.4.3(6), pf = 2tf/bw = 0.00266667
bw = 750.00
effective stress from (A.35), ff,e = 800.0873

fy = 0.04491794
af = 0.38744444
b = 400.00
h = 750.00

From EC8 A4.4.3(6), pf = 2tf/bw = 0.005
bw = 400.00
effective stress from (A.35), ff,e = 765.1636

R = 40.00

Effective FRP thickness, tf = NL*t*Cos(b1) = 1.00

fu,f = 840.00

Ef = 82000.00

u,f = 0.015

ase ((5.4d), TBDY) = (ase1*Aext+ase2*Aint)/Asec = 0.12601038

ase1 = 0.12601038

bo_1 = 690.00

ho_1 = 340.00

bi2_1 = 1.1834E+006

ase2 = Max(ase1,ase2) = 0.12601038

bo_2 = 542.00

ho_2 = 192.00

$$bi2_2 = 661256.00$$

$$psh_min*Fywe = \text{Min}(psh,x*Fywe, psh,y*Fywe) = 1.64062$$

Expression ((5.4d), TBDY) for $psh_min*Fywe$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$psh_x*Fywe = psh1*Fywe1 + ps2*Fywe2 = 1.64062$$

$$ps1 \text{ (external)} = (Ash1*h1/s1)/Asec = 0.0020944$$

$$Ash1 = Astir_1*ns_1 = 157.0796$$

$$\text{No stirrups, } ns_1 = 2.00$$

$$h1 = 400.00$$

$$ps2 \text{ (internal)} = (Ash2*h2/s2)/Asec = 0.00026808$$

$$Ash2 = Astir_2*ns_2 = 100.531$$

$$\text{No stirrups, } ns_2 = 2.00$$

$$h2 = 200.00$$

$$psh_y*Fywe = psh1*Fywe1 + ps2*Fywe2 = 3.23907$$

$$ps1 \text{ (external)} = (Ash1*h1/s1)/Asec = 0.00392699$$

$$Ash1 = Astir_1*ns_1 = 157.0796$$

$$\text{No stirrups, } ns_1 = 2.00$$

$$h1 = 750.00$$

$$ps2 \text{ (internal)} = (Ash2*h2/s2)/Asec = 0.00073723$$

$$Ash2 = Astir_2*ns_2 = 100.531$$

$$\text{No stirrups, } ns_2 = 2.00$$

$$h2 = 550.00$$

$$Asec = 300000.00$$

$$s1 = 100.00$$

$$s2 = 250.00$$

$$fywe1 = 694.45$$

$$fywe2 = 694.45$$

$$fce = 33.00$$

$$\text{From ((5.A5), TBDY), TBDY: } cc = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$y1 = 0.00140044$$

$$sh1 = 0.0044814$$

$$ft1 = 466.8167$$

$$fy1 = 389.0139$$

$$su1 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$$lo/lou,min = lb/ld = 0.30$$

$$su1 = 0.4*esu1_nominal \text{ ((5.5), TBDY)} = 0.032$$

$$\text{From table 5A.1, TBDY: } esu1_nominal = 0.08,$$

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$$y1, sh1, ft1, fy1, \text{ are also multiplied by } \text{Min}(1, 1.25*(lb/ld)^{2/3}), \text{ from 10.3.5, ASCE 41-17.}$$

$$\text{with } fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139$$

$$\text{with } Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00$$

$$y2 = 0.00140044$$

$$sh2 = 0.0044814$$

$$ft2 = 466.8167$$

$$fy2 = 389.0139$$

$$su2 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$$lo/lou,min = lb/lb,min = 0.30$$

$$su2 = 0.4*esu2_nominal \text{ ((5.5), TBDY)} = 0.032$$

$$\text{From table 5A.1, TBDY: } esu2_nominal = 0.08,$$

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$$y1, sh1, ft1, fy1, \text{ are also multiplied by } \text{Min}(1, 1.25*(lb/ld)^{2/3}), \text{ from 10.3.5, ASCE 41-17.}$$

$$\text{with } fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139$$

$$\text{with } Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00$$

$$yv = 0.00140044$$

$$shv = 0.0044814$$

$$ftv = 466.8167$$

$$fyv = 389.0139$$

$$suv = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lo_{u,min} = lb/ld = 0.30$$

$$suv = 0.4 * esuv_{nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $esuv_{nominal} = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY

For calculation of $esuv_{nominal}$ and yv , shv , ftv , fyv , it is considered
characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.

$y1$, $sh1$, $ft1$, $fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fsv = (fs_{jacket} * Asl_{mid,jacket} + fs_{mid} * Asl_{mid,core}) / Asl_{mid} = 389.0139$$

$$\text{with } Es_v = (Es_{jacket} * Asl_{mid,jacket} + Es_{mid} * Asl_{mid,core}) / Asl_{mid} = 200000.00$$

$$1 = Asl_{ten} / (b * d) * (fs1 / fc) = 0.04740567$$

$$2 = Asl_{com} / (b * d) * (fs2 / fc) = 0.04740567$$

$$v = Asl_{mid} / (b * d) * (fsv / fc) = 0.01676222$$

and confined core properties:

$$b = 340.00$$

$$d = 677.00$$

$$d' = 13.00$$

$$fcc (5A.2, TBDY) = 34.43547$$

$$cc (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = Asl_{ten} / (b * d) * (fs1 / fc) = 0.05824278$$

$$2 = Asl_{com} / (b * d) * (fs2 / fc) = 0.05824278$$

$$v = Asl_{mid} / (b * d) * (fsv / fc) = 0.02059413$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < vs_{y2}$ - LHS eq.(4.5) is satisfied

--->

$$su (4.9) = 0.15682678$$

$$Mu = MRc (4.14) = 3.7261E+008$$

$$u = su (4.1) = 8.5888248E-006$$

Calculation of ratio lb/ld

Inadequate Lap Length with $lb/ld = 0.30$

Calculation of Mu_{2+}

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$$u = 8.5888248E-006$$

$$Mu = 3.7261E+008$$

with full section properties:

$$b = 400.00$$

$$d = 707.00$$

$$d' = 43.00$$

$$v = 0.00118049$$

$$N = 11016.808$$

$$fc = 33.00$$

$$co (5A.5, TBDY) = 0.002$$

$$\text{Final value of } cu: cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } cu = 0.01006403$$

$$we ((5.4c), TBDY) = ase * sh_{min} * fy_{we} / f_{ce} + \text{Min}(fx, fy) = 0.02692906$$

where $f = af * pf * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$fx = 0.02504965$$

af = 0.38744444
b = 750.00
h = 400.00
From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.00266667$
bw = 750.00
effective stress from (A.35), $ff,e = 800.0873$

fy = 0.04491794
af = 0.38744444
b = 400.00
h = 750.00
From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.005$
bw = 400.00
effective stress from (A.35), $ff,e = 765.1636$

R = 40.00
Effective FRP thickness, $tf = NL*t*Cos(b1) = 1.00$
fu,f = 840.00
Ef = 82000.00
u,f = 0.015
ase ((5.4d), TBDY) = $(ase1*Aext+ase2*Aint)/Asec = 0.12601038$
ase1 = 0.12601038
bo_1 = 690.00
ho_1 = 340.00
bi2_1 = 1.1834E+006
ase2 = Max(ase1,ase2) = 0.12601038
bo_2 = 542.00
ho_2 = 192.00
bi2_2 = 661256.00

psh,min*Fywe = Min(psh,x*Fywe , psh,y*Fywe) = 1.64062
Expression ((5.4d), TBDY) for psh,min*Fywe has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh_x*Fywe = psh1*Fywe1+ps2*Fywe2 = 1.64062
ps1 (external) = $(Ash1*h1/s1)/Asec = 0.0020944$
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 400.00
ps2 (internal) = $(Ash2*h2/s2)/Asec = 0.00026808$
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 200.00

psh_y*Fywe = psh1*Fywe1+ps2*Fywe2 = 3.23907
ps1 (external) = $(Ash1*h1/s1)/Asec = 0.00392699$
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 750.00
ps2 (internal) = $(Ash2*h2/s2)/Asec = 0.00073723$
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

lo/lou,min = lb/d = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{d})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044

sh2 = 0.0044814

ft2 = 466.8167

fy2 = 389.0139

su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{d})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044

shv = 0.0044814

ftv = 466.8167

fyv = 389.0139

suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

lo/lou,min = lb/d = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{d})^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.04740567

2 = Asl,com/(b*d)*(fs2/fc) = 0.04740567

v = Asl,mid/(b*d)*(fsv/fc) = 0.01676222

and confined core properties:

b = 340.00

d = 677.00

d' = 13.00

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

1 = Asl,ten/(b*d)*(fs1/fc) = 0.05824278

2 = Asl,com/(b*d)*(fs2/fc) = 0.05824278

v = Asl,mid/(b*d)*(fsv/fc) = 0.02059413

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

v < vs,y2 - LHS eq.(4.5) is satisfied

--->

su (4.9) = 0.15682678

Mu = MRc (4.14) = 3.7261E+008

u = su (4.1) = 8.5888248E-006

Calculation of ratio lb/d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of μ_2 -

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:

$$\mu = 8.5888248E-006$$

$$\mu = 3.7261E+008$$

with full section properties:

$$b = 400.00$$

$$d = 707.00$$

$$d' = 43.00$$

$$v = 0.00118049$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$c_o \text{ (5A.5, TBDY)} = 0.002$$

$$\text{Final value of } \mu: \mu^* = \text{shear_factor} * \text{Max}(\mu, c_o) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \mu = 0.01006403$$

$$w_e \text{ ((5.4c), TBDY)} = a_{se} * \text{sh}_{,min} * f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$$

where $f = a_f * p_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.02504965$$

$$a_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.00266667$$

$$b_w = 750.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 800.0873$$

$$f_y = 0.04491794$$

$$a_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.005$$

$$b_w = 400.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L * t * \text{Cos}(b_1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{,f} = 0.015$$

$$a_{se} \text{ ((5.4d), TBDY)} = (a_{se1} * A_{ext} + a_{se2} * A_{int}) / A_{sec} = 0.12601038$$

$$a_{se1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i,2,1} = 1.1834E+006$$

$$a_{se2} = \text{Max}(a_{se1}, a_{se2}) = 0.12601038$$

$$b_{o,2} = 542.00$$

$$h_{o,2} = 192.00$$

$$b_{i,2,2} = 661256.00$$

$$p_{sh,min} * F_{ywe} = \text{Min}(p_{sh,x} * F_{ywe}, p_{sh,y} * F_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $p_{sh,min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$p_{sh,x} * F_{ywe} = p_{sh1} * F_{ywe1} + p_{sh2} * F_{ywe2} = 1.64062$$

$$p_{s1} \text{ (external)} = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$$

$$A_{sh1} = A_{stir,1} * n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

$$h_1 = 400.00$$

$$p_{s2} \text{ (internal)} = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00026808$$

Ash2 = Astir_2*ns_2 = 100.531
No stirups, ns_2 = 2.00
h2 = 200.00

psh_y*Fywe = psh1*Fywe1+ps2*Fywe2 = 3.23907
ps1 (external) = (Ash1*h1/s1)/Asec = 0.00392699
Ash1 = Astir_1*ns_1 = 157.0796
No stirups, ns_1 = 2.00
h1 = 750.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00073723
Ash2 = Astir_2*ns_2 = 100.531
No stirups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/l_d = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044
sh2 = 0.0044814
ft2 = 466.8167
fy2 = 389.0139
su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/l_b,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044
shv = 0.0044814
ftv = 466.8167
fyv = 389.0139
suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/l_d = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered

characteristic value $f_{sv} = f_{sv}/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $f_{sv} = (f_{s,jacket} \cdot A_{s,mid,jacket} + f_{s,mid} \cdot A_{s,mid,core}) / A_{s,mid} = 389.0139$

with $E_{sv} = (E_{s,jacket} \cdot A_{s,mid,jacket} + E_{s,mid} \cdot A_{s,mid,core}) / A_{s,mid} = 200000.00$

1 = $A_{s,ten} / (b \cdot d) \cdot (f_{s1} / f_c) = 0.04740567$

2 = $A_{s,com} / (b \cdot d) \cdot (f_{s2} / f_c) = 0.04740567$

$v = A_{s,mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.01676222$

and confined core properties:

$b = 340.00$

$d = 677.00$

$d' = 13.00$

$f_{cc} (5A.2, TBDY) = 34.43547$

$cc (5A.5, TBDY) = 0.00243499$

$c = \text{confinement factor} = 1.0435$

1 = $A_{s,ten} / (b \cdot d) \cdot (f_{s1} / f_c) = 0.05824278$

2 = $A_{s,com} / (b \cdot d) \cdot (f_{s2} / f_c) = 0.05824278$

$v = A_{s,mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.02059413$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

---->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

---->

$\mu (4.9) = 0.15682678$

$\mu_u = M_{Rc} (4.14) = 3.7261E+008$

$u = \mu_u (4.1) = 8.5888248E-006$

Calculation of ratio lb/ld

Inadequate Lap Length with $lb/ld = 0.30$

Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 1.2627E+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 1.2627E+006$

$V_{r1} = V_{Col} ((10.3), ASCE 41-17) = k_{nl} \cdot V_{Col0}$

$V_{Col0} = 1.2627E+006$

$k_{nl} = 1$ (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v \cdot f_y \cdot d / s$ ' is replaced by ' $V_s + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: $f'_c = (f'_{c,jacket} \cdot Area_{jacket} + f'_{c,core} \cdot Area_{core}) / Area_{section} = 33.00$, but $f_c^{0.5} \leq 8.3$
MPa (22.5.3.1, ACI 318-14)

$M/Vd = 2.00$

$\mu_u = 2.3795529E-012$

$V_u = 2.9582284E-031$

$d = 0.8 \cdot h = 600.00$

$N_u = 11016.808$

$A_g = 300000.00$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 621900.694$

where:

$V_{s1} = 523602.964$ is calculated for jacket, with:

$d = 600.00$

$A_v = 157079.633$

$f_y = 555.56$

$s = 100.00$

V_{s1} is multiplied by $Col1 = 1.00$

$s/d = 0.16666667$

$V_{s2} = 98297.73$ is calculated for core, with:

$d = 440.00$

$A_v = 100530.965$

$f_y = 555.56$

$s = 250.00$

Vs2 is multiplied by Col2 = 1.00

$$s/d = 0.56818182$$

$$V_f((11-3)-(11.4), \text{ACI 440}) = 463792.00$$

f = 0.95, for fully-wrapped sections

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression, where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, a)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, a1)|), \text{ with:}$$

total thickness per orientation, $tf1 = NL*t/\text{NoDir} = 1.00$

$$dfv = d \text{ (figure 11.2, ACI 440)} = 707.00$$

$$ffe((11-5), \text{ACI 440}) = 328.00$$

$$E_f = 82000.00$$

$$f_e = 0.004, \text{ from (11.6a), ACI 440}$$

$$\text{with } f_u = 0.009$$

$$\text{From (11-11), ACI 440: } V_s + V_f \leq 915872.391$$

$$bw = 400.00$$

Calculation of Shear Strength at edge 2, $V_{r2} = 1.2627E+006$

$$V_{r2} = V_{Col}((10.3), \text{ASCE 41-17}) = knl * V_{Col0}$$

$$V_{Col0} = 1.2627E+006$$

knl = 1 (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v * f_y * d / s$ ' is replaced by ' $V_s + f * V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: $f'_c = (f'_c_{\text{jacket}} * \text{Area}_{\text{jacket}} + f'_c_{\text{core}} * \text{Area}_{\text{core}}) / \text{Area}_{\text{section}} = 33.00$, but $f'_c^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$$M/Vd = 2.00$$

$$M_u = 2.3795529E-012$$

$$V_u = 2.9582284E-031$$

$$d = 0.8 * h = 600.00$$

$$N_u = 11016.808$$

$$A_g = 300000.00$$

$$\text{From (11.5.4.8), ACI 318-14: } V_s = V_{s1} + V_{s2} = 621900.694$$

where:

$V_{s1} = 523602.964$ is calculated for jacket, with:

$$d = 600.00$$

$$A_v = 157079.633$$

$$f_y = 555.56$$

$$s = 100.00$$

V_{s1} is multiplied by Col1 = 1.00

$$s/d = 0.16666667$$

$V_{s2} = 98297.73$ is calculated for core, with:

$$d = 440.00$$

$$A_v = 100530.965$$

$$f_y = 555.56$$

$$s = 250.00$$

V_{s2} is multiplied by Col2 = 1.00

$$s/d = 0.56818182$$

$$V_f((11-3)-(11.4), \text{ACI 440}) = 463792.00$$

f = 0.95, for fully-wrapped sections

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression, where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, a)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, a1)|), \text{ with:}$$

total thickness per orientation, $tf1 = NL*t/\text{NoDir} = 1.00$

$$dfv = d \text{ (figure 11.2, ACI 440)} = 707.00$$

$$ffe((11-5), \text{ACI 440}) = 328.00$$

$$E_f = 82000.00$$

fe = 0.004, from (11.6a), ACI 440
with fu = 0.009
From (11-11), ACI 440: Vs + Vf <= 915872.391
bw = 400.00

End Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1
At local axis: 2

Start Of Calculation of Chord Rotation Capacity for element: column JC1 of floor 1
At local axis: 2
Integration Section: (b)
Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$
Chord Rotation is generally considered as Deformation-Controlled Action according to Table C7-1, ASCE 41-17.
Mean strengths are used for Deformation-Controlled Actions according to 7.5.1.3, ASCE 41-17
Consequently:
Jacket
New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$
New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$
Concrete Elasticity, $E_c = 26999.444$
Steel Elasticity, $E_s = 200000.00$
Existing Column
New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$
New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$
Concrete Elasticity, $E_c = 26999.444$
Steel Elasticity, $E_s = 200000.00$
External Height, $H = 400.00$
External Width, $W = 750.00$
Internal Height, $H = 200.00$
Internal Width, $W = 550.00$
Cover Thickness, $c = 25.00$
Element Length, $L = 3000.00$
Primary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Inadequate Lap Length with $l_b/l_d = 0.30$
FRP Wrapping Data
Type: Carbon
Dry properties (design values)
Thickness, $t = 1.00$
Tensile Strength, $f_{fu} = 840.00$
Tensile Modulus, $E_f = 82000.00$
Elongation, $e_{fu} = 0.009$
Number of directions, $NoDir = 1$
Fiber orientations, $bi = 0.00^\circ$
Number of layers, $NL = 1$
Radius of rounding corners, $R = 40.00$

Stepwise Properties

Bending Moment, $M = -3.8976464E-011$
Shear Force, $V_2 = 1995.365$
Shear Force, $V_3 = 1.4564234E-013$
Axial Force, $F = -14177.47$
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: $As_t = 0.00$
-Compression: $As_c = 2676.637$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $Asl,ten = 1137.257$

-Compression: $Asl,com = 1137.257$

-Middle: $Asl,mid = 402.1239$

Longitudinal External Reinforcement Area Distribution (in 3 divisions)

-Tension: $Asl,ten,jacket = 829.3805$

-Compression: $Asl,com,jacket = 829.3805$

-Middle: $Asl,mid,jacket = 402.1239$

Longitudinal Internal Reinforcement Area Distribution (in 3 divisions)

-Tension: $Asl,ten,core = 307.8761$

-Compression: $Asl,com,core = 307.8761$

-Middle: $Asl,mid,core = 0.00$

Mean Diameter of Tension Reinforcement, $DbL = 16.80$

New component: From table 7-7, ASCE 41_17: Final chord rotation Capacity $u,R = 1.0^*$ $u = 0.00203008$

$u = y + p = 0.00203008$

- Calculation of y -

$y = (My*Ls/3)/Eleff = 0.00203008$ ((4.29),Biskinis Phd))

$My = 1.3155E+008$

$Ls = M/V$ (with $Ls > 0.1*L$ and $Ls < 2*L$) = 1500.00

From table 10.5, ASCE 41_17: $Eleff = factor*Ec*Ig = 3.2399E+013$

$factor = 0.30$

$Ag = 300000.00$

Mean concrete strength: $fc' = (fc'_jacket*Area_jacket + fc'_core*Area_core)/Area_section = 33.00$

$N = 14177.47$

$Ec*Ig = Ec_jacket*Ig_jacket + Ec_core*Ig_core = 1.0800E+014$

Calculation of Yielding Moment My

Calculation of y and My according to Annex 7 -

$y = \text{Min}(y_{ten}, y_{com})$

$y_{ten} = 5.6359183E-006$

with ((10.1), ASCE 41-17) $f_y = \text{Min}(f_y, 1.25*f_y*(lb/d)^{2/3}) = 311.2112$

$d = 357.00$

$y = 0.22662123$

$A = 0.01016692$

$B = 0.00577058$

with $pt = 0.00424746$

$pc = 0.00424746$

$p_v = 0.00150186$

$N = 14177.47$

$b = 750.00$

$" = 0.12044818$

$y_{comp} = 2.7804373E-005$

with fc^* (12.3, (ACI 440)) = 33.35499

$fc = 33.00$

$fl = 0.77176471$

$b = 750.00$

$h = 400.00$

$Ag = 300000.00$

From (12.9), ACI 440: $ka = 0.14671972$

$g = pt + pc + p_v = 0.00999678$

$rc = 40.00$

$Ae/Ac = 0.51581153$

Effective FRP thickness, $tf = NL*t*\text{Cos}(b1) = 1.00$

effective strain from (12.5) and (12.12), $efe = 0.004$

$fu = 0.009$

$Ef = 82000.00$

$E_c = 26999.444$
 $y = 0.22402532$
 $A = 0.00987772$
 $B = 0.00560044$
with $E_s = 200000.00$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

- Calculation of p -

From table 10-8: $p = 0.00$

with:

- Columns controlled by inadequate development or splicing along the clear height because $l_b/l_d < 1$

shear control ratio $V_y E / V_{Col} O E = 0.14265244$

$d = d_{external} = 357.00$

$s = s_{external} = 0.00$

$t = s_1 + s_2 + 2 * t_f / b_w * (f_{fe} / f_s) = 0.00393687$

jacket: $s_1 = A_{v1} * h_1 / (s_1 * A_g) = 0.0020944$

$A_{v1} = 157.0796$, is the total area of all stirrups parallel to loading (shear) direction

$h_1 = 400.00$

$s_1 = 100.00$

core: $s_2 = A_{v2} * h_2 / (s_2 * A_g) = 0.00026808$

$A_{v2} = 100.531$, is the total area of all stirrups parallel to loading (shear) direction

$h_2 = 200.00$

$s_2 = 250.00$

The term $2 * t_f / b_w * (f_{fe} / f_s)$ is implemented to account for FRP contribution

where $f = 2 * t_f / b_w$ is FRP ratio (EC8 - 3, A.4.4.3(6)) and f_{fe} / f_s normalises f to steel strength

All these variables have already been given in Shear control ratio calculation.

For the normalisation f_s of jacket is used.

$NUD = 14177.47$

$A_g = 300000.00$

$f_{cE} = (f_{c_jacket} * Area_jacket + f_{c_core} * Area_core) / section_area = 33.00$

$f_{yE} = (f_{y_ext_Long_Reinf} * Area_ext_Long_Reinf + f_{y_int_Long_Reinf} * Area_int_Long_Reinf) / Area_Tot_Long_Rein =$

555.56

$f_{ytE} = (f_{y_ext_Trans_Reinf} * s_1 + f_{y_int_Trans_Reinf} * s_2) / (s_1 + s_2) = 555.56$

$p_l = Area_Tot_Long_Rein / (b * d) = 0.00999678$

$b = 750.00$

$d = 357.00$

$f_{cE} = 33.00$

End Of Calculation of Chord Rotation Capacity for element: column JC1 of floor 1

At local axis: 2

Integration Section: (b)

Calculation No. 7

column C1, Floor 1

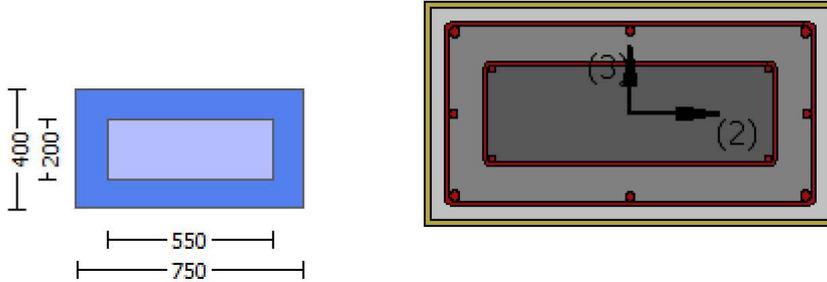
Limit State: Immediate Occupancy (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Shear capacity VRd

Edge: End

Local Axis: (3)



Start Of Calculation of Shear Capacity for element: column JC1 of floor 1

At local axis: 3

Integration Section: (b)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$

Member Shear Force is generally considered as Force-Controlled Action according to Table C7-1, ASCE 41-17.

Lower-bound strengths are used for Force-Controlled Actions according to 7.5.1.3, ASCE 41-17

Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Primary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Existing Column

New material of Primary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Primary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of γ for displacement ductility demand, the expected (mean value) strengths are used (7.5.1.3, ASCE 41-17) because bending is considered as Deformation-Controlled Action (Table C7-1, ASCE 41-17).

Jacket

New material: Concrete Strength, $f_c = f_{cm} = 33.00$

New material: Steel Strength, $f_s = f_{sm} = 555.56$

Existing Column

New material: Concrete Strength, $f_c = f_{cm} = 33.00$

New material: Steel Strength, $f_s = f_{sm} = 555.56$

#####

External Height, $H = 400.00$

External Width, $W = 750.00$

Internal Height, $H = 200.00$

Internal Width, $W = 550.00$

Cover Thickness, $c = 25.00$

Element Length, $L = 3000.00$

Primary Member

Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Inadequate Lap Length with $l_o/l_{ou,min} = l_b/l_d = 0.30$
FRP Wrapping Data
Type: Carbon
Dry properties (design values)
Thickness, $t = 1.00$
Tensile Strength, $f_{fu} = 840.00$
Tensile Modulus, $E_f = 82000.00$
Elongation, $e_{fu} = 0.009$
Number of directions, $NoDir = 1$
Fiber orientations, $bi: 0.00^\circ$
Number of layers, $NL = 1$
Radius of rounding corners, $R = 40.00$

Stepwise Properties

EDGE -A-
Bending Moment, $M_a = 4.7677845E-010$
Shear Force, $V_a = -1.4564234E-013$
EDGE -B-
Bending Moment, $M_b = -3.8976464E-011$
Shear Force, $V_b = 1.4564234E-013$
BOTH EDGES
Axial Force, $F = -14177.47$
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: $A_{st} = 0.00$
-Compression: $A_{sc} = 2676.637$
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: $A_{st,ten} = 1137.257$
-Compression: $A_{st,com} = 1137.257$
-Middle: $A_{st,mid} = 402.1239$
Mean Diameter of Tension Reinforcement, $Db_{L,ten} = 16.80$

New component: From table 7-7, ASCE 41_17: Final Shear Capacity $V_R = 1.0 * V_n = 776632.031$
 V_n ((10.3), ASCE 41-17) = $k_n * V_{CoI0} = 776632.031$
 $V_{CoI} = 776632.031$
 $k_n = 1.00$
 $displacement_ductility_demand = 0.00$

NOTE: In expression (10-3) ' $V_s = A_v * f_y * d / s$ ' is replaced by ' $V_s + f * V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)
Mean concrete strength: $f_c' = (f_c'_{jacket} * Area_{jacket} + f_c'_{core} * Area_{core}) / Area_{section} = 25.00$, but $f_c'^{0.5} \leq 8.3$
MPa (22.5.3.1, ACI 318-14)
 $M / Vd = 2.00$
 $M_u = 3.8976464E-011$
 $V_u = 1.4564234E-013$
 $d = 0.8 * h = 320.00$
 $N_u = 14177.47$
 $A_g = 300000.00$
From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 251327.412$
where:
 $V_{s1} = 251327.412$ is calculated for jacket, with:
 $d = 320.00$
 $A_v = 157079.633$
 $f_y = 500.00$
 $s = 100.00$
 V_{s1} is multiplied by $Col1 = 1.00$
 $s/d = 0.3125$

Vs2 = 0.00 is calculated for core, with:

$$d = 160.00$$

$$A_v = 100530.965$$

$$f_y = 500.00$$

$$s = 250.00$$

Vs2 is multiplied by Col2 = 0.00

$$s/d = 1.5625$$

Vf ((11-3)-(11.4), ACI 440) = 234192.00

$$f = 0.95, \text{ for fully-wrapped sections}$$

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression,

where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function Vf(θ), is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

Vf = Min(|Vf(45, θ)|, |Vf(-45, θ)|), with:

total thickness per orientation, $t_{f1} = NL * t / \text{NoDir} = 1.00$

dfv = d (figure 11.2, ACI 440) = 357.00

ffe ((11-5), ACI 440) = 328.00

$$E_f = 82000.00$$

$$f_e = 0.004, \text{ from (11.6a), ACI 440}$$

$$\text{with } f_u = 0.009$$

From (11-11), ACI 440: Vs + Vf <= 797164.595

$$b_w = 750.00$$

displacement ductility demand is calculated as δ / y

- Calculation of δ / y for END B -

for rotation axis 2 and integ. section (b)

From analysis, chord rotation $\theta = 2.3933076E-021$

$$y = (M_y * L_s / 3) / E_{\text{eff}} = 0.00203008 \text{ ((4.29), Biskinis Phd)}$$

$$M_y = 1.3155E+008$$

$$L_s = M/V \text{ (with } L_s > 0.1 * L \text{ and } L_s < 2 * L) = 1500.00$$

From table 10.5, ASCE 41_17: $E_{\text{eff}} = \text{factor} * E_c * I_g = 3.2399E+013$

$$\text{factor} = 0.30$$

$$A_g = 300000.00$$

$$\text{Mean concrete strength: } f_c' = (f_c'_{\text{jacket}} * A_{\text{jacket}} + f_c'_{\text{core}} * A_{\text{core}}) / A_{\text{section}} = 33.00$$

$$N = 14177.47$$

$$E_c * I_g = E_c_{\text{jacket}} * I_{g_{\text{jacket}}} + E_c_{\text{core}} * I_{g_{\text{core}}} = 1.0800E+014$$

Calculation of Yielding Moment My

Calculation of δ / y and My according to Annex 7 -

$$y = \text{Min}(y_{\text{ten}}, y_{\text{com}})$$

$$y_{\text{ten}} = 5.6359183E-006$$

$$\text{with ((10.1), ASCE 41-17) } f_y = \text{Min}(f_y, 1.25 * f_y * (I_b / d)^{2/3}) = 311.2112$$

$$d = 357.00$$

$$y = 0.22662123$$

$$A = 0.01016692$$

$$B = 0.00577058$$

$$\text{with } p_t = 0.00424746$$

$$p_c = 0.00424746$$

$$p_v = 0.00150186$$

$$N = 14177.47$$

$$b = 750.00$$

$$\theta = 0.12044818$$

$$y_{\text{comp}} = 2.7804373E-005$$

with $f_c' * (12.3, \text{ (ACI 440)}) = 33.35499$

$$f_c = 33.00$$

$$f_l = 0.77176471$$

$$b = 750.00$$

$h = 400.00$
 $A_g = 300000.00$
 From (12.9), ACI 440: $k_a = 0.14671972$
 $g = p_t + p_c + p_v = 0.00999678$
 $rc = 40.00$
 $A_e/A_c = 0.51581153$
 Effective FRP thickness, $t_f = NL * t * \cos(b_1) = 1.00$
 effective strain from (12.5) and (12.12), $\epsilon_{fe} = 0.004$
 $f_u = 0.009$
 $E_f = 82000.00$
 $E_c = 26999.444$
 $y = 0.22402532$
 $A = 0.00987772$
 $B = 0.00560044$
 with $E_s = 200000.00$

Calculation of ratio I_b/I_d

Inadequate Lap Length with $I_b/I_d = 0.30$

End Of Calculation of Shear Capacity for element: column JC1 of floor 1

At local axis: 3

Integration Section: (b)

Calculation No. 8

column C1, Floor 1

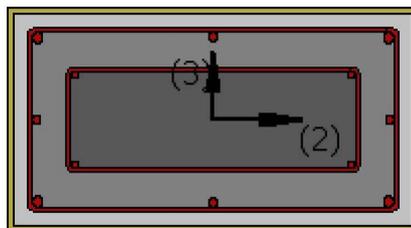
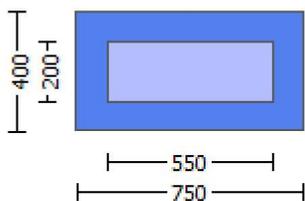
Limit State: Immediate Occupancy (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Chord rotation capacity (θ_u)

Edge: End

Local Axis: (3)



Start Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At Shear local axis: 3

(Bending local axis: 2)
Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$
Mean strength values are used for both shear and moment calculations.
Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$
New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$
Concrete Elasticity, $E_c = 26999.444$
Steel Elasticity, $E_s = 200000.00$

Existing Column

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$
New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$
Concrete Elasticity, $E_c = 26999.444$
Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of moment strengths,
the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

Jacket

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

Existing Column

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

#####

External Height, $H = 400.00$

External Width, $W = 750.00$

Internal Height, $H = 200.00$

Internal Width, $W = 550.00$

Cover Thickness, $c = 25.00$

Mean Confinement Factor overall section = 1.0435

Element Length, $L = 3000.00$

Primary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Inadequate Lap Length with $l_o/l_{ou,min} = 0.30$

FRP Wrapping Data

Type: Carbon

Dry properties (design values)

Thickness, $t = 1.00$

Tensile Strength, $f_{fu} = 840.00$

Tensile Modulus, $E_f = 82000.00$

Elongation, $e_{fu} = 0.009$

Number of directions, $NoDir = 1$

Fiber orientations, $bi = 0.00^\circ$

Number of layers, $NL = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 3

EDGE -A-

Shear Force, $V_a = 3.4586346E-031$

EDGE -B-

Shear Force, $V_b = -3.4586346E-031$

BOTH EDGES

Axial Force, $F = -11016.808$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $As_c = 2676.637$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{l,ten} = 1137.257$

-Compression: $As_{l,com} = 1137.257$

-Middle: Asl,mid = 402.1239

Calculation of Shear Capacity ratio , $V_e/V_r = 0.14265244$

Member Controlled by Flexure ($V_e/V_r < 1$)

Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 121055.918$

with

$M_{pr1} = \text{Max}(\mu_{1+}, \mu_{1-}) = 1.8158E+008$

$\mu_{1+} = 1.8158E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination

$\mu_{1-} = 1.8158E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination

$M_{pr2} = \text{Max}(\mu_{2+}, \mu_{2-}) = 1.8158E+008$

$\mu_{2+} = 1.8158E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the the static loading combination

$\mu_{2-} = 1.8158E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the the static loading combination

Calculation of μ_{1+}

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:

$\mu = 1.7255512E-005$

$\mu_u = 1.8158E+008$

with full section properties:

$b = 750.00$

$d = 357.00$

$d' = 43.00$

$v = 0.00124684$

$N = 11016.808$

$f_c = 33.00$

α (5A.5, TBDY) = 0.002

Final value of μ_u : $\mu_u^* = \text{shear_factor} * \text{Max}(\mu_u, \mu_c) = 0.01006403$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $\mu_u = 0.01006403$

μ_u ((5.4c), TBDY) = $\alpha * \text{sh, min} * f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$

where $f = \alpha * p_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.02504965$

$\alpha_f = 0.38744444$

$b = 750.00$

$h = 400.00$

From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.00266667$

$bw = 750.00$

effective stress from (A.35), $f_{f,e} = 800.0873$

$f_y = 0.04491794$

$\alpha_f = 0.38744444$

$b = 400.00$

$h = 750.00$

From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.005$

$bw = 400.00$

effective stress from (A.35), $f_{f,e} = 765.1636$

$R = 40.00$

Effective FRP thickness, $t_f = NL * t * \text{Cos}(b_1) = 1.00$

$f_{u,f} = 840.00$

$E_f = 82000.00$

$u_{,f} = 0.015$

α_{se} ((5.4d), TBDY) = $(\alpha_{se1} * A_{ext} + \alpha_{se2} * A_{int}) / A_{sec} = 0.12601038$

$\alpha_{se1} = 0.12601038$

$b_{o,1} = 690.00$

ho_1 = 340.00
bi2_1 = 1.1834E+006
ase2 = Max(ase1,ase2) = 0.12601038
bo_2 = 542.00
ho_2 = 192.00
bi2_2 = 661256.00

psh,min*Fywe = Min(psh,x*Fywe , psh,y*Fywe) = 1.64062

Expression ((5.4d), TBDY) for psh,min*Fywe has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh_x*Fywe = psh1*Fywe1+ps2*Fywe2 = 1.64062
ps1 (external) = (Ash1*h1/s1)/Asec = 0.0020944
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 400.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00026808
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 200.00

psh_y*Fywe = psh1*Fywe1+ps2*Fywe2 = 3.23907
ps1 (external) = (Ash1*h1/s1)/Asec = 0.00392699
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 750.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00073723
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/d = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^ 2/3), from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044
sh2 = 0.0044814
ft2 = 466.8167
fy2 = 389.0139
su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs_2 = (fs_{jacket} \cdot A_{s1,com,jacket} + fs_{core} \cdot A_{s1,com,core}) / A_{s1,com} = 389.0139$

with $Es_2 = (Es_{jacket} \cdot A_{s1,com,jacket} + Es_{core} \cdot A_{s1,com,core}) / A_{s1,com} = 200000.00$

$y_v = 0.00140044$

$sh_v = 0.0044814$

$ft_v = 466.8167$

$fy_v = 389.0139$

$suv = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

$l_o/l_{ou,min} = l_b/l_d = 0.30$

$suv = 0.4 \cdot esuv_{nominal} ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esuv_{nominal} = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY

For calculation of $esuv_{nominal}$ and y_v, sh_v, ft_v, fy_v , it is considered

characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fsv = (fs_{jacket} \cdot A_{s,mid,jacket} + fs_{mid} \cdot A_{s,mid,core}) / A_{s,mid} = 389.0139$

with $Es_v = (Es_{jacket} \cdot A_{s,mid,jacket} + Es_{mid} \cdot A_{s,mid,core}) / A_{s,mid} = 200000.00$

$1 = A_{s,ten} / (b \cdot d) \cdot (fs_1 / fc) = 0.0500703$

$2 = A_{s,com} / (b \cdot d) \cdot (fs_2 / fc) = 0.0500703$

$v = A_{s,mid} / (b \cdot d) \cdot (fsv / fc) = 0.01770442$

and confined core properties:

$b = 690.00$

$d = 327.00$

$d' = 13.00$

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

$1 = A_{s,ten} / (b \cdot d) \cdot (fs_1 / fc) = 0.05941729$

$2 = A_{s,com} / (b \cdot d) \cdot (fs_2 / fc) = 0.05941729$

$v = A_{s,mid} / (b \cdot d) \cdot (fsv / fc) = 0.02100943$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

su (4.9) = 0.16886055

$Mu = MRc$ (4.14) = 1.8158E+008

$u = su$ (4.1) = 1.7255512E-005

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of Mu_1 -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 1.7255512E-005$

$Mu = 1.8158E+008$

with full section properties:

$b = 750.00$

$d = 357.00$

$d' = 43.00$

$v = 0.00124684$

$N = 11016.808$

$fc = 33.00$

co (5A.5, TBDY) = 0.002

Final value of cu : $cu^* = \text{shear_factor} \cdot \text{Max}(cu, cc) = 0.01006403$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $cu = 0.01006403$

w_e ((5.4c), TBDY) = $ase * sh_{min} * fy_{we} / f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$

where $f = af * pf * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.02504965$

$af = 0.38744444$

$b = 750.00$

$h = 400.00$

From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.00266667$

$bw = 750.00$

effective stress from (A.35), $ff_e = 800.0873$

$f_y = 0.04491794$

$af = 0.38744444$

$b = 400.00$

$h = 750.00$

From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.005$

$bw = 400.00$

effective stress from (A.35), $ff_e = 765.1636$

$R = 40.00$

Effective FRP thickness, $tf = NL * t * \text{Cos}(b_1) = 1.00$

$f_{u,f} = 840.00$

$E_f = 82000.00$

$u_{,f} = 0.015$

ase ((5.4d), TBDY) = $(ase_1 * A_{ext} + ase_2 * A_{int}) / A_{sec} = 0.12601038$

$ase_1 = 0.12601038$

$bo_1 = 690.00$

$ho_1 = 340.00$

$bi_{2,1} = 1.1834E+006$

$ase_2 = \text{Max}(ase_1, ase_2) = 0.12601038$

$bo_2 = 542.00$

$ho_2 = 192.00$

$bi_{2,2} = 661256.00$

$psh_{min} * Fy_{we} = \text{Min}(psh_x * Fy_{we}, psh_y * Fy_{we}) = 1.64062$

Expression ((5.4d), TBDY) for $psh_{min} * Fy_{we}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh_x * Fy_{we} = psh_1 * Fy_{we1} + ps_2 * Fy_{we2} = 1.64062$

ps_1 (external) = $(Ash_1 * h_1 / s_1) / A_{sec} = 0.0020944$

$Ash_1 = Astir_1 * ns_1 = 157.0796$

No stirrups, $ns_1 = 2.00$

$h_1 = 400.00$

ps_2 (internal) = $(Ash_2 * h_2 / s_2) / A_{sec} = 0.00026808$

$Ash_2 = Astir_2 * ns_2 = 100.531$

No stirrups, $ns_2 = 2.00$

$h_2 = 200.00$

$psh_y * Fy_{we} = psh_1 * Fy_{we1} + ps_2 * Fy_{we2} = 3.23907$

ps_1 (external) = $(Ash_1 * h_1 / s_1) / A_{sec} = 0.00392699$

$Ash_1 = Astir_1 * ns_1 = 157.0796$

No stirrups, $ns_1 = 2.00$

$h_1 = 750.00$

ps_2 (internal) = $(Ash_2 * h_2 / s_2) / A_{sec} = 0.00073723$

$Ash_2 = Astir_2 * ns_2 = 100.531$

No stirrups, $ns_2 = 2.00$

$h_2 = 550.00$

$A_{sec} = 300000.00$

$s_1 = 100.00$

$s_2 = 250.00$

$fy_{we1} = 694.45$

$fy_{we2} = 694.45$

$f_{ce} = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00243499$

$c =$ confinement factor = 1.0435

$y_1 = 0.00140044$

$sh1 = 0.0044814$
 $ft1 = 466.8167$
 $fy1 = 389.0139$
 $su1 = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou,min = lb/ld = 0.30$
 $su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu1_nominal = 0.08$,
 For calculation of $esu1_nominal$ and $y1, sh1,ft1,fy1$, it is considered
 characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.
 $y1, sh1,ft1,fy1$, are also multiplied by $Min(1,1.25*(lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139$
 with $Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00$
 $y2 = 0.00140044$
 $sh2 = 0.0044814$
 $ft2 = 466.8167$
 $fy2 = 389.0139$
 $su2 = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou,min = lb/lb,min = 0.30$
 $su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu2_nominal = 0.08$,
 For calculation of $esu2_nominal$ and $y2, sh2,ft2,fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.
 $y1, sh1,ft1,fy1$, are also multiplied by $Min(1,1.25*(lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139$
 with $Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00$
 $yv = 0.00140044$
 $shv = 0.0044814$
 $ftv = 466.8167$
 $fyv = 389.0139$
 $suv = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou,min = lb/ld = 0.30$
 $suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_nominal = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv, shv,ftv,fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 $y1, sh1,ft1,fy1$, are also multiplied by $Min(1,1.25*(lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139$
 with $Es_v = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00$
 $1 = Asl,ten/(b*d)*(fs1/fc) = 0.0500703$
 $2 = Asl,com/(b*d)*(fs2/fc) = 0.0500703$
 $v = Asl,mid/(b*d)*(fsv/fc) = 0.01770442$
 and confined core properties:
 $b = 690.00$
 $d = 327.00$
 $d' = 13.00$
 $fcc (5A.2, TBDY) = 34.43547$
 $cc (5A.5, TBDY) = 0.00243499$
 $c = \text{confinement factor} = 1.0435$
 $1 = Asl,ten/(b*d)*(fs1/fc) = 0.05941729$
 $2 = Asl,com/(b*d)*(fs2/fc) = 0.05941729$
 $v = Asl,mid/(b*d)*(fsv/fc) = 0.02100943$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)
 --->
 $v < vs,y2$ - LHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.16886055$

$$\begin{aligned} \mu &= MRC(4.14) = 1.8158E+008 \\ u &= su(4.1) = 1.7255512E-005 \end{aligned}$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of μ_{2+}

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:

$$\begin{aligned} u &= 1.7255512E-005 \\ \mu &= 1.8158E+008 \end{aligned}$$

with full section properties:

$$\begin{aligned} b &= 750.00 \\ d &= 357.00 \\ d' &= 43.00 \\ v &= 0.00124684 \\ N &= 11016.808 \\ f_c &= 33.00 \\ c_o(5A.5, TBDY) &= 0.002 \\ \text{Final value of } \mu: \mu^* &= \text{shear_factor} * \text{Max}(\mu_c, \mu_{cc}) = 0.01006403 \\ \text{The Shear_factor is considered equal to 1 (pure moment strength)} \\ \text{From (5.4b), TBDY: } \mu_c &= 0.01006403 \\ \mu_{we}((5.4c), TBDY) &= a_{se} * s_{h,min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.02692906 \\ \text{where } f &= a_f * p_f * f_{fe}/f_{ce} \text{ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)} \end{aligned}$$

$$\begin{aligned} f_x &= 0.02504965 \\ a_f &= 0.38744444 \\ b &= 750.00 \\ h &= 400.00 \\ \text{From EC8 A.4.4.3(6), } p_f &= 2t_f/b_w = 0.00266667 \\ b_w &= 750.00 \\ \text{effective stress from (A.35), } f_{f,e} &= 800.0873 \end{aligned}$$

$$\begin{aligned} f_y &= 0.04491794 \\ a_f &= 0.38744444 \\ b &= 400.00 \\ h &= 750.00 \\ \text{From EC8 A.4.4.3(6), } p_f &= 2t_f/b_w = 0.005 \\ b_w &= 400.00 \\ \text{effective stress from (A.35), } f_{f,e} &= 765.1636 \end{aligned}$$

$$\begin{aligned} R &= 40.00 \\ \text{Effective FRP thickness, } t_f &= NL * t * \text{Cos}(b_1) = 1.00 \\ f_{u,f} &= 840.00 \\ E_f &= 82000.00 \\ u_{f} &= 0.015 \\ a_{se}((5.4d), TBDY) &= (a_{se1} * A_{ext} + a_{se2} * A_{int}) / A_{sec} = 0.12601038 \\ a_{se1} &= 0.12601038 \\ b_{o,1} &= 690.00 \\ h_{o,1} &= 340.00 \\ b_{i,1} &= 1.1834E+006 \\ a_{se2} &= \text{Max}(a_{se1}, a_{se2}) = 0.12601038 \\ b_{o,2} &= 542.00 \\ h_{o,2} &= 192.00 \\ b_{i,2} &= 661256.00 \end{aligned}$$

$p_{sh,min} * F_{ywe} = \text{Min}(p_{sh,x} * F_{ywe}, p_{sh,y} * F_{ywe}) = 1.64062$
Expression ((5.4d), TBDY) for $p_{sh,min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$p_{sh,x} * F_{ywe} = p_{sh1} * F_{ywe1} + p_{sh2} * F_{ywe2} = 1.64062$$

$$ps1 \text{ (external)} = (Ash1 \cdot h1 / s1) / Asec = 0.0020944$$

$$Ash1 = Astir_1 \cdot ns_1 = 157.0796$$

$$\text{No stirups, } ns_1 = 2.00$$

$$h1 = 400.00$$

$$ps2 \text{ (internal)} = (Ash2 \cdot h2 / s2) / Asec = 0.00026808$$

$$Ash2 = Astir_2 \cdot ns_2 = 100.531$$

$$\text{No stirups, } ns_2 = 2.00$$

$$h2 = 200.00$$

$$psh_y \cdot Fywe = psh1 \cdot Fywe1 + ps2 \cdot Fywe2 = 3.23907$$

$$ps1 \text{ (external)} = (Ash1 \cdot h1 / s1) / Asec = 0.00392699$$

$$Ash1 = Astir_1 \cdot ns_1 = 157.0796$$

$$\text{No stirups, } ns_1 = 2.00$$

$$h1 = 750.00$$

$$ps2 \text{ (internal)} = (Ash2 \cdot h2 / s2) / Asec = 0.00073723$$

$$Ash2 = Astir_2 \cdot ns_2 = 100.531$$

$$\text{No stirups, } ns_2 = 2.00$$

$$h2 = 550.00$$

$$Asec = 300000.00$$

$$s1 = 100.00$$

$$s2 = 250.00$$

$$fywe1 = 694.45$$

$$fywe2 = 694.45$$

$$fce = 33.00$$

$$\text{From } ((5.A5), \text{ TBDY}), \text{ TBDY: } cc = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$y1 = 0.00140044$$

$$sh1 = 0.0044814$$

$$ft1 = 466.8167$$

$$fy1 = 389.0139$$

$$su1 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lou, \text{min} = lb/ld = 0.30$$

$$su1 = 0.4 \cdot esu1_{\text{nominal}} ((5.5), \text{ TBDY}) = 0.032$$

From table 5A.1, TBDY: $esu1_{\text{nominal}} = 0.08$,

For calculation of $esu1_{\text{nominal}}$ and $y1, sh1, ft1, fy1$, it is considered
characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fs1 = (fs_{\text{jacket}} \cdot Asl, \text{ten, jacket} + fs_{\text{core}} \cdot Asl, \text{ten, core}) / Asl, \text{ten} = 389.0139$$

$$\text{with } Es1 = (Es_{\text{jacket}} \cdot Asl, \text{ten, jacket} + Es_{\text{core}} \cdot Asl, \text{ten, core}) / Asl, \text{ten} = 200000.00$$

$$y2 = 0.00140044$$

$$sh2 = 0.0044814$$

$$ft2 = 466.8167$$

$$fy2 = 389.0139$$

$$su2 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lou, \text{min} = lb/lb, \text{min} = 0.30$$

$$su2 = 0.4 \cdot esu2_{\text{nominal}} ((5.5), \text{ TBDY}) = 0.032$$

From table 5A.1, TBDY: $esu2_{\text{nominal}} = 0.08$,

For calculation of $esu2_{\text{nominal}}$ and $y2, sh2, ft2, fy2$, it is considered
characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fs2 = (fs_{\text{jacket}} \cdot Asl, \text{com, jacket} + fs_{\text{core}} \cdot Asl, \text{com, core}) / Asl, \text{com} = 389.0139$$

$$\text{with } Es2 = (Es_{\text{jacket}} \cdot Asl, \text{com, jacket} + Es_{\text{core}} \cdot Asl, \text{com, core}) / Asl, \text{com} = 200000.00$$

$$yv = 0.00140044$$

$$shv = 0.0044814$$

$$ftv = 466.8167$$

$$fyv = 389.0139$$

$$suv = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$l_0/l_{ou,min} = l_b/l_d = 0.30$$

$$s_{uv} = 0.4 * e_{suv,nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $e_{suv,nominal} = 0.08$,

considering characteristic value $f_{sv} = f_{sv}/1.2$, from table 5.1, TBDY

For calculation of $e_{suv,nominal}$ and γ_v , sh_v, ft_v, fy_v , it is considered

characteristic value $f_{sv} = f_{sv}/1.2$, from table 5.1, TBDY.

$$\gamma_1, sh_1, ft_1, fy_1, \text{ are also multiplied by } \text{Min}(1, 1.25 * (l_b/l_d)^{2/3}), \text{ from 10.3.5, ASCE 41-17.}$$

$$\text{with } f_{sv} = (f_{s,jacket} * A_{s,mid,jacket} + f_{s,mid} * A_{s,mid,core}) / A_{s,mid} = 389.0139$$

$$\text{with } E_{sv} = (E_{s,jacket} * A_{s,mid,jacket} + E_{s,mid} * A_{s,mid,core}) / A_{s,mid} = 200000.00$$

$$1 = A_{s,ten} / (b * d) * (f_{s1} / f_c) = 0.0500703$$

$$2 = A_{s,com} / (b * d) * (f_{s2} / f_c) = 0.0500703$$

$$v = A_{s,mid} / (b * d) * (f_{sv} / f_c) = 0.01770442$$

and confined core properties:

$$b = 690.00$$

$$d = 327.00$$

$$d' = 13.00$$

$$f_{cc} (5A.2, TBDY) = 34.43547$$

$$c_c (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = A_{s,ten} / (b * d) * (f_{s1} / f_c) = 0.05941729$$

$$2 = A_{s,com} / (b * d) * (f_{s2} / f_c) = 0.05941729$$

$$v = A_{s,mid} / (b * d) * (f_{sv} / f_c) = 0.02100943$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$s_u (4.9) = 0.16886055$$

$$M_u = M_{Rc} (4.14) = 1.8158E+008$$

$$u = s_u (4.1) = 1.7255512E-005$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of M_u2 -

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$$u = 1.7255512E-005$$

$$M_u = 1.8158E+008$$

with full section properties:

$$b = 750.00$$

$$d = 357.00$$

$$d' = 43.00$$

$$v = 0.00124684$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$c_o (5A.5, TBDY) = 0.002$$

$$\text{Final value of } \phi_u: \phi_u^* = \text{shear_factor} * \text{Max}(\phi_{cu}, \phi_{cc}) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \phi_{cu} = 0.01006403$$

$$\phi_{cc} \text{ ((5.4c), TBDY) = } a_s e * sh_{,min} * f_{ywe} / f_{ce} + \text{Min}(\phi_{fx}, \phi_{fy}) = 0.02692906$$

where $\phi_f = a_f * p_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\phi_{fx} = 0.02504965$$

$$a_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f / b_w = 0.00266667$$

$$b_w = 750.00$$

$$\text{effective stress from (A.35), } f_{fe} = 800.0873$$

fy = 0.04491794
af = 0.38744444
b = 400.00
h = 750.00
From EC8 A.4.4.3(6), pf = 2tf/bw = 0.005
bw = 400.00
effective stress from (A.35), ff,e = 765.1636

R = 40.00
Effective FRP thickness, tf = NL*t*Cos(b1) = 1.00
fu,f = 840.00
Ef = 82000.00
u,f = 0.015
ase ((5.4d), TBDY) = (ase1*Aext+ase2*Aint)/Asec = 0.12601038
ase1 = 0.12601038
bo_1 = 690.00
ho_1 = 340.00
bi2_1 = 1.1834E+006
ase2 = Max(ase1,ase2) = 0.12601038
bo_2 = 542.00
ho_2 = 192.00
bi2_2 = 661256.00
psh,min*Fywe = Min(psh,x*Fywe , psh,y*Fywe) = 1.64062

Expression ((5.4d), TBDY) for psh,min*Fywe has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh_x*Fywe = psh1*Fywe1+ps2*Fywe2 = 1.64062
ps1 (external) = (Ash1*h1/s1)/Asec = 0.0020944
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 400.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00026808
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 200.00

psh_y*Fywe = psh1*Fywe1+ps2*Fywe2 = 3.23907
ps1 (external) = (Ash1*h1/s1)/Asec = 0.00392699
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 750.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00073723
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/d = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered

characteristic value $f_{s1} = f_{s1}/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $f_{s1} = (f_{s,jacket} \cdot A_{s,ten,jacket} + f_{s,core} \cdot A_{s,ten,core})/A_{s,ten} = 389.0139$

with $E_{s1} = (E_{s,jacket} \cdot A_{s,ten,jacket} + E_{s,core} \cdot A_{s,ten,core})/A_{s,ten} = 200000.00$

$y_2 = 0.00140044$

$sh_2 = 0.0044814$

$ft_2 = 466.8167$

$fy_2 = 389.0139$

$su_2 = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

$l_o/l_{o,min} = l_b/l_{d,min} = 0.30$

$su_2 = 0.4 \cdot esu_{2,nominal} ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu_{2,nominal} = 0.08$,

For calculation of $esu_{2,nominal}$ and y_2, sh_2, ft_2, fy_2 , it is considered

characteristic value $f_{s2} = f_{s2}/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $f_{s2} = (f_{s,jacket} \cdot A_{s,com,jacket} + f_{s,core} \cdot A_{s,com,core})/A_{s,com} = 389.0139$

with $E_{s2} = (E_{s,jacket} \cdot A_{s,com,jacket} + E_{s,core} \cdot A_{s,com,core})/A_{s,com} = 200000.00$

$y_v = 0.00140044$

$sh_v = 0.0044814$

$ft_v = 466.8167$

$fy_v = 389.0139$

$su_v = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

$l_o/l_{o,min} = l_b/l_d = 0.30$

$su_v = 0.4 \cdot esu_{v,nominal} ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu_{v,nominal} = 0.08$,

considering characteristic value $f_{sv} = f_{sv}/1.2$, from table 5.1, TBDY

For calculation of $esu_{v,nominal}$ and y_v, sh_v, ft_v, fy_v , it is considered

characteristic value $f_{sv} = f_{sv}/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $f_{sv} = (f_{s,jacket} \cdot A_{s,mid,jacket} + f_{s,mid} \cdot A_{s,mid,core})/A_{s,mid} = 389.0139$

with $E_{sv} = (E_{s,jacket} \cdot A_{s,mid,jacket} + E_{s,mid} \cdot A_{s,mid,core})/A_{s,mid} = 200000.00$

$1 = A_{s,ten}/(b \cdot d) \cdot (f_{s1}/f_c) = 0.0500703$

$2 = A_{s,com}/(b \cdot d) \cdot (f_{s2}/f_c) = 0.0500703$

$v = A_{s,mid}/(b \cdot d) \cdot (f_{sv}/f_c) = 0.01770442$

and confined core properties:

$b = 690.00$

$d = 327.00$

$d' = 13.00$

$f_{cc} (5A.2, TBDY) = 34.43547$

$cc (5A.5, TBDY) = 0.00243499$

$c = \text{confinement factor} = 1.0435$

$1 = A_{s,ten}/(b \cdot d) \cdot (f_{s1}/f_c) = 0.05941729$

$2 = A_{s,com}/(b \cdot d) \cdot (f_{s2}/f_c) = 0.05941729$

$v = A_{s,mid}/(b \cdot d) \cdot (f_{sv}/f_c) = 0.02100943$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$su (4.9) = 0.16886055$

$Mu = MRc (4.14) = 1.8158E+008$

$u = su (4.1) = 1.7255512E-005$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 848607.437$

Calculation of Shear Strength at edge 1, $V_{r1} = 848607.437$

$V_{r1} = V_{Col} \text{ ((10.3), ASCE 41-17)} = knl * V_{ColO}$

$V_{ColO} = 848607.437$

$knl = 1$ (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v * f_y * d / s$ ' is replaced by ' $V_s + f * V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

$= 1$ (normal-weight concrete)

Mean concrete strength: $f_c' = (f_c'_{jacket} * Area_{jacket} + f_c'_{core} * Area_{core}) / Area_{section} = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$M / Vd = 2.00$

$\mu_u = 3.5345895E-012$

$\nu_u = 3.4586346E-031$

$d = 0.8 * h = 320.00$

$N_u = 11016.808$

$A_g = 300000.00$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 279254.914$

where:

$V_{s1} = 279254.914$ is calculated for jacket, with:

$d = 320.00$

$A_v = 157079.633$

$f_y = 555.56$

$s = 100.00$

V_{s1} is multiplied by $Col1 = 1.00$

$s/d = 0.3125$

$V_{s2} = 0.00$ is calculated for core, with:

$d = 160.00$

$A_v = 100530.965$

$f_y = 555.56$

$s = 250.00$

V_{s2} is multiplied by $Col2 = 0.00$

$s/d = 1.5625$

$V_f \text{ ((11-3)-(11.4), ACI 440)} = 234192.00$

$f = 0.95$, for fully-wrapped sections

$w_f / s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,

where α is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\alpha = 45^\circ$ and $\alpha = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\alpha = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \alpha)|, |V_f(-45, \alpha)|)$, with:

total thickness per orientation, $t_{f1} = NL * t / \text{NoDir} = 1.00$

$d_{fv} = d$ (figure 11.2, ACI 440) = 357.00

$f_{fe} \text{ ((11-5), ACI 440)} = 328.00$

$E_f = 82000.00$

$f_{e} = 0.004$, from (11.6a), ACI 440

with $f_u = 0.009$

From (11-11), ACI 440: $V_s + V_f \leq 915872.391$

$b_w = 750.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 848607.437$

$V_{r2} = V_{Col} \text{ ((10.3), ASCE 41-17)} = knl * V_{ColO}$

$V_{ColO} = 848607.437$

$knl = 1$ (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v * f_y * d / s$ ' is replaced by ' $V_s + f * V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

$= 1$ (normal-weight concrete)

Mean concrete strength: $f_c' = (f_c'_{jacket} * Area_{jacket} + f_c'_{core} * Area_{core}) / Area_{section} = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$M / Vd = 2.00$

$\mu_u = 3.5345895E-012$

Vu = 3.4586346E-031
d = 0.8*h = 320.00
Nu = 11016.808
Ag = 300000.00
From (11.5.4.8), ACI 318-14: Vs = Vs1 + Vs2 = 279254.914

where:

Vs1 = 279254.914 is calculated for jacket, with:

d = 320.00
Av = 157079.633
fy = 555.56
s = 100.00

Vs1 is multiplied by Col1 = 1.00

s/d = 0.3125

Vs2 = 0.00 is calculated for core, with:

d = 160.00
Av = 100530.965
fy = 555.56
s = 250.00

Vs2 is multiplied by Col2 = 0.00

s/d = 1.5625

Vf ((11-3)-(11.4), ACI 440) = 234192.00

f = 0.95, for fully-wrapped sections

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) $\sin \theta + \cos \theta$ is replaced with $(\cot \theta + \cot \alpha) \sin \alpha$ which is more a generalised expression, where θ is the angle of the crack direction (see KANEPE).

This later relation, considered as a function Vf(θ , α), is implemented for every different fiber orientation ai, as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

Vf = Min(|Vf(45, θ)|, |Vf(-45, θ)|), with:

total thickness per orientation, tf1 = NL*t/NoDir = 1.00

dfv = d (figure 11.2, ACI 440) = 357.00

ffe ((11-5), ACI 440) = 328.00

Ef = 82000.00
fe = 0.004, from (11.6a), ACI 440
with fu = 0.009

From (11-11), ACI 440: Vs + Vf <= 915872.391

bw = 750.00

End Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1
At local axis: 3

Start Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1
At Shear local axis: 2
(Bending local axis: 3)
Section Type: rcjrs

Constant Properties

Knowledge Factor, $\phi = 0.90$
Mean strength values are used for both shear and moment calculations.
Consequently:

Jacket

New material of Primary Member: Concrete Strength, fc = fcm = 33.00

New material of Primary Member: Steel Strength, fs = fsm = 555.56

Concrete Elasticity, Ec = 26999.444

Steel Elasticity, Es = 200000.00

Existing Column

New material of Primary Member: Concrete Strength, fc = fcm = 33.00

New material of Primary Member: Steel Strength, fs = fsm = 555.56

Concrete Elasticity, Ec = 26999.444

Steel Elasticity, Es = 200000.00

#####

Note: Especially for the calculation of moment strengths,

the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

Jacket

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

Existing Column

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

#####

External Height, $H = 400.00$

External Width, $W = 750.00$

Internal Height, $H = 200.00$

Internal Width, $W = 550.00$

Cover Thickness, $c = 25.00$

Mean Confinement Factor overall section = 1.0435

Element Length, $L = 3000.00$

Primary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Inadequate Lap Length with $l_o/l_{o,u,min} = 0.30$

FRP Wrapping Data

Type: Carbon

Dry properties (design values)

Thickness, $t = 1.00$

Tensile Strength, $f_{fu} = 840.00$

Tensile Modulus, $E_f = 82000.00$

Elongation, $e_{fu} = 0.009$

Number of directions, $NoDir = 1$

Fiber orientations, $b_i = 0.00^\circ$

Number of layers, $NL = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 2

EDGE -A-

Shear Force, $V_a = 2.9582284E-031$

EDGE -B-

Shear Force, $V_b = -2.9582284E-031$

BOTH EDGES

Axial Force, $F = -11016.808$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $As_c = 2676.637$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{l,ten} = 1137.257$

-Compression: $As_{l,com} = 1137.257$

-Middle: $As_{l,mid} = 402.1239$

Calculation of Shear Capacity ratio , $V_e/V_r = 0.19671905$

Member Controlled by Flexure ($V_e/V_r < 1$)

Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 248405.514$

with

$M_{pr1} = \text{Max}(Mu_{1+} , Mu_{1-}) = 3.7261E+008$

$Mu_{1+} = 3.7261E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination

$Mu_{1-} = 3.7261E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination

$M_{pr2} = \text{Max}(Mu_{2+} , Mu_{2-}) = 3.7261E+008$

$Mu_{2+} = 3.7261E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the the static loading combination

$Mu_{2-} = 3.7261E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the the static loading combination

Calculation of Mu1+

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:

$$\mu = 8.5888248E-006$$

$$\text{Mu} = 3.7261E+008$$

with full section properties:

$$b = 400.00$$

$$d = 707.00$$

$$d' = 43.00$$

$$v = 0.00118049$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$c_o (5A.5, \text{TBDY}) = 0.002$$

$$\text{Final value of } c_u: c_u^* = \text{shear_factor} * \text{Max}(c_u, c_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } c_u = 0.01006403$$

$$w_e ((5.4c), \text{TBDY}) = a_{se} * s_{h,\min} * f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$$

where $f = a_f * p_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.02504965$$

$$a_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f / b_w = 0.00266667$$

$$b_w = 750.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 800.0873$$

$$f_y = 0.04491794$$

$$a_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f / b_w = 0.005$$

$$b_w = 400.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L * t * \text{Cos}(b_1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{,f} = 0.015$$

$$a_{se} ((5.4d), \text{TBDY}) = (a_{se1} * A_{ext} + a_{se2} * A_{int}) / A_{sec} = 0.12601038$$

$$a_{se1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i2,1} = 1.1834E+006$$

$$a_{se2} = \text{Max}(a_{se1}, a_{se2}) = 0.12601038$$

$$b_{o,2} = 542.00$$

$$h_{o,2} = 192.00$$

$$b_{i2,2} = 661256.00$$

$$p_{sh,\min} * F_{ywe} = \text{Min}(p_{sh,x} * F_{ywe}, p_{sh,y} * F_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $p_{sh,\min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$p_{sh,x} * F_{ywe} = p_{sh1} * F_{ywe1} + p_{sh2} * F_{ywe2} = 1.64062$$

$$p_{sh1} (\text{external}) = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$$

$$A_{sh1} = A_{stir,1} * n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

$$h_1 = 400.00$$

$$p_{sh2} (\text{internal}) = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00026808$$

$$A_{sh2} = A_{stir,2} * n_{s,2} = 100.531$$

$$\text{No stirrups, } n_{s,2} = 2.00$$

$$h_2 = 200.00$$

psh_y*Fywe = psh₁*Fywe₁+ps₂*Fywe₂ = 3.23907
ps₁ (external) = (Ash₁*h₁/s₁)/Asec = 0.00392699
Ash₁ = Astir₁*ns₁ = 157.0796
No stirups, ns₁ = 2.00
h₁ = 750.00
ps₂ (internal) = (Ash₂*h₂/s₂)/Asec = 0.00073723
Ash₂ = Astir₂*ns₂ = 100.531
No stirups, ns₂ = 2.00
h₂ = 550.00

Asec = 300000.00

s₁ = 100.00

s₂ = 250.00

fywe₁ = 694.45

fywe₂ = 694.45

fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499

c = confinement factor = 1.0435

y₁ = 0.00140044

sh₁ = 0.0044814

ft₁ = 466.8167

fy₁ = 389.0139

su₁ = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/l_d = 0.30

su₁ = 0.4*esu_{1_nominal} ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu_{1_nominal} = 0.08,

For calculation of esu_{1_nominal} and y₁, sh₁,ft₁,fy₁, it is considered
characteristic value fsy₁ = fs₁/1.2, from table 5.1, TBDY.

y₁, sh₁,ft₁,fy₁, are also multiplied by Min(1,1.25*(lb/l_d)^{2/3}), from 10.3.5, ASCE 41-17.

with fs₁ = (fs_{jacket}*Asl_{ten,jacket} + fs_{core}*Asl_{ten,core})/Asl_{ten} = 389.0139

with Es₁ = (Es_{jacket}*Asl_{ten,jacket} + Es_{core}*Asl_{ten,core})/Asl_{ten} = 200000.00

y₂ = 0.00140044

sh₂ = 0.0044814

ft₂ = 466.8167

fy₂ = 389.0139

su₂ = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/l_{b,min} = 0.30

su₂ = 0.4*esu_{2_nominal} ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu_{2_nominal} = 0.08,

For calculation of esu_{2_nominal} and y₂, sh₂,ft₂,fy₂, it is considered
characteristic value fsy₂ = fs₂/1.2, from table 5.1, TBDY.

y₂, sh₂,ft₂,fy₂, are also multiplied by Min(1,1.25*(lb/l_d)^{2/3}), from 10.3.5, ASCE 41-17.

with fs₂ = (fs_{jacket}*Asl_{com,jacket} + fs_{core}*Asl_{com,core})/Asl_{com} = 389.0139

with Es₂ = (Es_{jacket}*Asl_{com,jacket} + Es_{core}*Asl_{com,core})/Asl_{com} = 200000.00

y_v = 0.00140044

sh_v = 0.0044814

ft_v = 466.8167

fy_v = 389.0139

suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/l_d = 0.30

suv = 0.4*esuv_{nominal} ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_{nominal} = 0.08,

considering characteristic value fsy_v = fs_v/1.2, from table 5.1, TBDY

For calculation of esuv_{nominal} and y_v, sh_v,ft_v,fy_v, it is considered
characteristic value fsy_v = fs_v/1.2, from table 5.1, TBDY.

y₁, sh₁,ft₁,fy₁, are also multiplied by Min(1,1.25*(lb/l_d)^{2/3}), from 10.3.5, ASCE 41-17.

with fs_v = (fs_{jacket}*Asl_{mid,jacket} + fs_{mid}*Asl_{mid,core})/Asl_{mid} = 389.0139

with Es_v = (Es_{jacket}*Asl_{mid,jacket} + Es_{mid}*Asl_{mid,core})/Asl_{mid} = 200000.00

$$1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.04740567$$

$$2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.04740567$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.01676222$$

and confined core properties:

$$b = 340.00$$

$$d = 677.00$$

$$d' = 13.00$$

$$f_{cc} (5A.2, TBDY) = 34.43547$$

$$c_c (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.05824278$$

$$2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.05824278$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.02059413$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$s_u (4.9) = 0.15682678$$

$$\mu_u = M R_c (4.14) = 3.7261E+008$$

$$u = s_u (4.1) = 8.5888248E-006$$

 Calculation of ratio l_b/l_d

 Inadequate Lap Length with $l_b/l_d = 0.30$

 Calculation of μ_{u1} -

 Calculation of ultimate curvature μ_u according to 4.1, Biskinis/Fardis 2013:

$$u = 8.5888248E-006$$

$$\mu_u = 3.7261E+008$$

 with full section properties:

$$b = 400.00$$

$$d = 707.00$$

$$d' = 43.00$$

$$v = 0.00118049$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$c_o (5A.5, TBDY) = 0.002$$

$$\text{Final value of } c_u: c_u^* = \text{shear_factor} * \text{Max}(c_u, c_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } c_u = 0.01006403$$

$$w_e ((5.4c), TBDY) = a_{se} * s_{h,min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$$

where $f = a_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.02504965$$

$$a_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.00266667$$

$$b_w = 750.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 800.0873$$

$$f_y = 0.04491794$$

$$a_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.005$$

$$b_w = 400.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 765.1636$$

$R = 40.00$
 Effective FRP thickness, $t_f = NL * t * \cos(b_1) = 1.00$
 $f_{u,f} = 840.00$
 $E_f = 82000.00$
 $u_{,f} = 0.015$
 $ase((5.4d), TBDY) = (ase_1 * A_{ext} + ase_2 * A_{int}) / A_{sec} = 0.12601038$
 $ase_1 = 0.12601038$
 $bo_1 = 690.00$
 $ho_1 = 340.00$
 $bi_{2,1} = 1.1834E+006$
 $ase_2 = \max(ase_1, ase_2) = 0.12601038$
 $bo_2 = 542.00$
 $ho_2 = 192.00$
 $bi_{2,2} = 661256.00$

$psh_{,min} * F_{ywe} = \min(psh_{,x} * F_{ywe}, psh_{,y} * F_{ywe}) = 1.64062$
 Expression ((5.4d), TBDY) for $psh_{,min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh_{,x} * F_{ywe} = psh_1 * F_{ywe1} + ps_2 * F_{ywe2} = 1.64062$
 $ps_1 \text{ (external)} = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$
 $A_{sh1} = A_{stir,1} * ns_1 = 157.0796$
 No stirrups, $ns_1 = 2.00$
 $h_1 = 400.00$
 $ps_2 \text{ (internal)} = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00026808$
 $A_{sh2} = A_{stir,2} * ns_2 = 100.531$
 No stirrups, $ns_2 = 2.00$
 $h_2 = 200.00$

$psh_{,y} * F_{ywe} = psh_1 * F_{ywe1} + ps_2 * F_{ywe2} = 3.23907$
 $ps_1 \text{ (external)} = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.00392699$
 $A_{sh1} = A_{stir,1} * ns_1 = 157.0796$
 No stirrups, $ns_1 = 2.00$
 $h_1 = 750.00$
 $ps_2 \text{ (internal)} = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00073723$
 $A_{sh2} = A_{stir,2} * ns_2 = 100.531$
 No stirrups, $ns_2 = 2.00$
 $h_2 = 550.00$

$A_{sec} = 300000.00$
 $s_1 = 100.00$
 $s_2 = 250.00$
 $f_{ywe1} = 694.45$
 $f_{ywe2} = 694.45$
 $f_{ce} = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00243499$
 $c = \text{confinement factor} = 1.0435$

$y_1 = 0.00140044$
 $sh_1 = 0.0044814$
 $ft_1 = 466.8167$
 $fy_1 = 389.0139$
 $su_1 = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $Shear_factor = 1.00$

$lo/lo_{,min} = lb/ld = 0.30$

$su_1 = 0.4 * esu_{1,nominal}((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu_{1,nominal} = 0.08$,

For calculation of $esu_{1,nominal}$ and y_1, sh_1, ft_1, fy_1 , it is considered
 characteristic value $fsy_1 = fs_1 / 1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\min(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs_1 = (f_{s,jacket} * A_{s1,ten,jacket} + f_{s,core} * A_{s1,ten,core}) / A_{s1,ten} = 389.0139$

with $Es_1 = (E_{s,jacket} * A_{s1,ten,jacket} + E_{s,core} * A_{s1,ten,core}) / A_{s1,ten} = 200000.00$

$y_2 = 0.00140044$
 $sh_2 = 0.0044814$
 $ft_2 = 466.8167$
 $fy_2 = 389.0139$
 $su_2 = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$l_o/l_{o,u,min} = l_b/l_{b,min} = 0.30$$

$$s_u2 = 0.4 * e_{s_u2,nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $e_{s_u2,nominal} = 0.08$,

For calculation of $e_{s_u2,nominal}$ and y_2, sh_2, ft_2, fy_2 , it is considered
characteristic value $f_{s_y2} = f_{s_2}/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } f_{s_2} = (f_{s,jacket} * A_{s_l,com,jacket} + f_{s,core} * A_{s_l,com,core}) / A_{s_l,com} = 389.0139$$

$$\text{with } E_{s_2} = (E_{s,jacket} * A_{s_l,com,jacket} + E_{s,core} * A_{s_l,com,core}) / A_{s_l,com} = 200000.00$$

$$y_v = 0.00140044$$

$$sh_v = 0.0044814$$

$$ft_v = 466.8167$$

$$fy_v = 389.0139$$

$$s_{u_v} = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$l_o/l_{o,u,min} = l_b/l_d = 0.30$$

$$s_{u_v} = 0.4 * e_{s_{u_v},nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $e_{s_{u_v},nominal} = 0.08$,

considering characteristic value $f_{s_{y_v}} = f_{s_v}/1.2$, from table 5.1, TBDY

For calculation of $e_{s_{u_v},nominal}$ and y_v, sh_v, ft_v, fy_v , it is considered
characteristic value $f_{s_{y_v}} = f_{s_v}/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } f_{s_v} = (f_{s,jacket} * A_{s_l,mid,jacket} + f_{s,mid} * A_{s_l,mid,core}) / A_{s_l,mid} = 389.0139$$

$$\text{with } E_{s_v} = (E_{s,jacket} * A_{s_l,mid,jacket} + E_{s,mid} * A_{s_l,mid,core}) / A_{s_l,mid} = 200000.00$$

$$1 = A_{s_l,ten} / (b * d) * (f_{s_1} / f_c) = 0.04740567$$

$$2 = A_{s_l,com} / (b * d) * (f_{s_2} / f_c) = 0.04740567$$

$$v = A_{s_l,mid} / (b * d) * (f_{s_v} / f_c) = 0.01676222$$

and confined core properties:

$$b = 340.00$$

$$d = 677.00$$

$$d' = 13.00$$

$$f_{cc} (5A.2, TBDY) = 34.43547$$

$$cc (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = A_{s_l,ten} / (b * d) * (f_{s_1} / f_c) = 0.05824278$$

$$2 = A_{s_l,com} / (b * d) * (f_{s_2} / f_c) = 0.05824278$$

$$v = A_{s_l,mid} / (b * d) * (f_{s_v} / f_c) = 0.02059413$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y_2}$ - LHS eq.(4.5) is satisfied

--->

$$s_u (4.9) = 0.15682678$$

$$\mu_u = M_{Rc} (4.14) = 3.7261E+008$$

$$u = s_u (4.1) = 8.5888248E-006$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of μ_{u2+}

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$$u = 8.5888248E-006$$

$$\mu_u = 3.7261E+008$$

with full section properties:

$$b = 400.00$$

$$d = 707.00$$

$$d' = 43.00$$

$$v = 0.00118049$$

$$N = 11016.808$$

$$fc = 33.00$$

$$co (5A.5, TBDY) = 0.002$$

$$\text{Final value of } cu: cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } cu = 0.01006403$$

$$we ((5.4c), TBDY) = ase^* sh_{,min} * fy_{we} / f_{ce} + \text{Min}(fx, fy) = 0.02692906$$

where $f = af * pf * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$fx = 0.02504965$$

$$af = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } pf = 2tf/bw = 0.00266667$$

$$bw = 750.00$$

$$\text{effective stress from (A.35), } ff_{,e} = 800.0873$$

$$fy = 0.04491794$$

$$af = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } pf = 2tf/bw = 0.005$$

$$bw = 400.00$$

$$\text{effective stress from (A.35), } ff_{,e} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } tf = NL * t * \text{Cos}(b1) = 1.00$$

$$fu_{,f} = 840.00$$

$$Ef = 82000.00$$

$$u_{,f} = 0.015$$

$$ase ((5.4d), TBDY) = (ase1 * A_{ext} + ase2 * A_{int}) / A_{sec} = 0.12601038$$

$$ase1 = 0.12601038$$

$$bo_{,1} = 690.00$$

$$ho_{,1} = 340.00$$

$$bi2_{,1} = 1.1834E+006$$

$$ase2 = \text{Max}(ase1, ase2) = 0.12601038$$

$$bo_{,2} = 542.00$$

$$ho_{,2} = 192.00$$

$$bi2_{,2} = 661256.00$$

$$psh_{,min} * fy_{we} = \text{Min}(psh_{,x} * fy_{we}, psh_{,y} * fy_{we}) = 1.64062$$

Expression ((5.4d), TBDY) for $psh_{,min} * fy_{we}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$psh_{,x} * fy_{we} = psh1 * fy_{we1} + ps2 * fy_{we2} = 1.64062$$

$$ps1 \text{ (external)} = (Ash1 * h1 / s1) / A_{sec} = 0.0020944$$

$$Ash1 = Astir_{,1} * ns_{,1} = 157.0796$$

$$\text{No stirrups, } ns_{,1} = 2.00$$

$$h1 = 400.00$$

$$ps2 \text{ (internal)} = (Ash2 * h2 / s2) / A_{sec} = 0.00026808$$

$$Ash2 = Astir_{,2} * ns_{,2} = 100.531$$

$$\text{No stirrups, } ns_{,2} = 2.00$$

$$h2 = 200.00$$

$$psh_{,y} * fy_{we} = psh1 * fy_{we1} + ps2 * fy_{we2} = 3.23907$$

$$ps1 \text{ (external)} = (Ash1 * h1 / s1) / A_{sec} = 0.00392699$$

$$Ash1 = Astir_{,1} * ns_{,1} = 157.0796$$

$$\text{No stirrups, } ns_{,1} = 2.00$$

$$h1 = 750.00$$

$$ps2 \text{ (internal)} = (Ash2 * h2 / s2) / A_{sec} = 0.00073723$$

$$Ash2 = Astir_{,2} * ns_{,2} = 100.531$$

$$\text{No stirrups, } ns_{,2} = 2.00$$

$$h2 = 550.00$$

$$A_{sec} = 300000.00$$

s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A.5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044
sh2 = 0.0044814
ft2 = 466.8167
fy2 = 389.0139
su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044
shv = 0.0044814
ftv = 466.8167
fyv = 389.0139
suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.04740567

2 = Asl,com/(b*d)*(fs2/fc) = 0.04740567

v = Asl,mid/(b*d)*(fsv/fc) = 0.01676222

and confined core properties:

b = 340.00
d = 677.00
d' = 13.00

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

1 = Asl,ten/(b*d)*(fs1/fc) = 0.05824278

$$2 = A_{sl,com}/(b*d)^*(f_s2/f_c) = 0.05824278$$

$$v = A_{sl,mid}/(b*d)^*(f_{sv}/f_c) = 0.02059413$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

---->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

---->

$$s_u(4.9) = 0.15682678$$

$$M_u = M_{Rc}(4.14) = 3.7261E+008$$

$$u = s_u(4.1) = 8.5888248E-006$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of M_{u2} -

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$$u = 8.5888248E-006$$

$$M_u = 3.7261E+008$$

with full section properties:

$$b = 400.00$$

$$d = 707.00$$

$$d' = 43.00$$

$$v = 0.00118049$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$\alpha(5A.5, TBDY) = 0.002$$

$$\text{Final value of } \phi_u: \phi_u^* = \text{shear_factor} * \text{Max}(\phi_{cu}, \phi_{cc}) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \phi_{cu} = 0.01006403$$

$$\phi_{cc} \text{ ((5.4c), TBDY) } = \alpha * s_h * \text{min}(f_{ywe}/f_{ce} + \text{Min}(\phi_x, \phi_y)) = 0.02692906$$

where $\phi = \alpha * \rho_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\phi_x = 0.02504965$$

$$\alpha_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } \rho_f = 2t_f/bw = 0.00266667$$

$$bw = 750.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 800.0873$$

$$f_y = 0.04491794$$

$$\alpha_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } \rho_f = 2t_f/bw = 0.005$$

$$bw = 400.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL * t * \text{Cos}(b1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{f,e} = 0.015$$

$$\text{ase ((5.4d), TBDY) } = (\text{ase1} * A_{ext} + \text{ase2} * A_{int}) / A_{sec} = 0.12601038$$

$$\text{ase1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i,2,1} = 1.1834E+006$$

$$\text{ase2} = \text{Max}(\text{ase1}, \text{ase2}) = 0.12601038$$

$$bo_2 = 542.00$$

$$ho_2 = 192.00$$

$$bi2_2 = 661256.00$$

$$psh,min*Fywe = \text{Min}(psh,x*Fywe, psh,y*Fywe) = 1.64062$$

Expression ((5.4d), TBDY) for psh,min*Fywe has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$psh_x*Fywe = psh1*Fywe1 + ps2*Fywe2 = 1.64062$$

$$ps1 \text{ (external)} = (Ash1*h1/s1)/Asec = 0.0020944$$

$$Ash1 = Astir_1*ns_1 = 157.0796$$

$$\text{No stirrups, } ns_1 = 2.00$$

$$h1 = 400.00$$

$$ps2 \text{ (internal)} = (Ash2*h2/s2)/Asec = 0.00026808$$

$$Ash2 = Astir_2*ns_2 = 100.531$$

$$\text{No stirrups, } ns_2 = 2.00$$

$$h2 = 200.00$$

$$psh_y*Fywe = psh1*Fywe1 + ps2*Fywe2 = 3.23907$$

$$ps1 \text{ (external)} = (Ash1*h1/s1)/Asec = 0.00392699$$

$$Ash1 = Astir_1*ns_1 = 157.0796$$

$$\text{No stirrups, } ns_1 = 2.00$$

$$h1 = 750.00$$

$$ps2 \text{ (internal)} = (Ash2*h2/s2)/Asec = 0.00073723$$

$$Ash2 = Astir_2*ns_2 = 100.531$$

$$\text{No stirrups, } ns_2 = 2.00$$

$$h2 = 550.00$$

$$Asec = 300000.00$$

$$s1 = 100.00$$

$$s2 = 250.00$$

$$fywe1 = 694.45$$

$$fywe2 = 694.45$$

$$fce = 33.00$$

From ((5.A5), TBDY), TBDY: $cc = 0.00243499$

$$c = \text{confinement factor} = 1.0435$$

$$y1 = 0.00140044$$

$$sh1 = 0.0044814$$

$$ft1 = 466.8167$$

$$fy1 = 389.0139$$

$$su1 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$$lo/lou,min = lb/d = 0.30$$

$$su1 = 0.4*esu1_nominal \text{ ((5.5), TBDY)} = 0.032$$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25*(lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139$$

$$\text{with } Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00$$

$$y2 = 0.00140044$$

$$sh2 = 0.0044814$$

$$ft2 = 466.8167$$

$$fy2 = 389.0139$$

$$su2 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$$lo/lou,min = lb/lb,min = 0.30$$

$$su2 = 0.4*esu2_nominal \text{ ((5.5), TBDY)} = 0.032$$

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25*(lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139$$

$$\text{with } Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00$$

yv = 0.00140044
shv = 0.0044814
ftv = 466.8167
fyv = 389.0139
suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.04740567

2 = Asl,com/(b*d)*(fs2/fc) = 0.04740567

v = Asl,mid/(b*d)*(fsv/fc) = 0.01676222

and confined core properties:

b = 340.00

d = 677.00

d' = 13.00

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

1 = Asl,ten/(b*d)*(fs1/fc) = 0.05824278

2 = Asl,com/(b*d)*(fs2/fc) = 0.05824278

v = Asl,mid/(b*d)*(fsv/fc) = 0.02059413

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

v < vs,y2 - LHS eq.(4.5) is satisfied

--->

su (4.9) = 0.15682678

Mu = MRc (4.14) = 3.7261E+008

u = su (4.1) = 8.5888248E-006

Calculation of ratio lb/ld

Inadequate Lap Length with lb/ld = 0.30

Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 1.2627E+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 1.2627E+006$

$V_{r1} = V_{Co1} ((10.3), \text{ASCE } 41-17) = kn1 \cdot V_{Co10}$

$V_{Co10} = 1.2627E+006$

kn1 = 1 (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v \cdot f_y \cdot d / s$ ' is replaced by ' $V_s + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: $fc' = (fc'_{\text{jacket}} \cdot \text{Area}_{\text{jacket}} + fc'_{\text{core}} \cdot \text{Area}_{\text{core}}) / \text{Area}_{\text{section}} = 33.00$, but $fc'^{0.5} \leq 8.3$
MPa (22.5.3.1, ACI 318-14)

M/Vd = 2.00

Mu = 2.3795529E-012

Vu = 2.9582284E-031

d = 0.8*h = 600.00

Nu = 11016.808

Ag = 300000.00

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 621900.694$

where:

Vs1 = 523602.964 is calculated for jacket, with:

$$d = 600.00$$

$$A_v = 157079.633$$

$$f_y = 555.56$$

$$s = 100.00$$

Vs1 is multiplied by Col1 = 1.00

$$s/d = 0.16666667$$

Vs2 = 98297.73 is calculated for core, with:

$$d = 440.00$$

$$A_v = 100530.965$$

$$f_y = 555.56$$

$$s = 250.00$$

Vs2 is multiplied by Col2 = 1.00

$$s/d = 0.56818182$$

Vf ((11-3)-(11.4), ACI 440) = 463792.00

f = 0.95, for fully-wrapped sections

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) $\sin^2 + \cos^2$ is replaced with $(\cot^2 + \csc^2)\sin^2 a_i$ which is more a generalised expression, where a_i is the angle of the crack direction (see KANEPE).

This later relation, considered as a function Vf(a_i), is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $a_i = 45^\circ$ and $a_i = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $a_i = b_1 + 90^\circ = 90.00$

Vf = Min(|Vf(45, a_i)|, |Vf(-45, a_i)|), with:

total thickness per orientation, $t_{f1} = NL * t / NoDir = 1.00$

dfv = d (figure 11.2, ACI 440) = 707.00

ffe ((11-5), ACI 440) = 328.00

$$E_f = 82000.00$$

$$f_e = 0.004, \text{ from (11.6a), ACI 440}$$

$$\text{with } f_u = 0.009$$

From (11-11), ACI 440: $V_s + V_f \leq 915872.391$

$$b_w = 400.00$$

Calculation of Shear Strength at edge 2, $V_{r2} = 1.2627E+006$

$V_{r2} = V_{Col} ((10.3), ASCE 41-17) = k_{nl} * V_{Col0}$

$$V_{Col0} = 1.2627E+006$$

$k_{nl} = 1$ (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v * f_y * d / s$ ' is replaced by ' $V_s + f * V_f$ ' where Vf is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: $f_c' = (f_c'_{jacket} * Area_{jacket} + f_c'_{core} * Area_{core}) / Area_{section} = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$$M/Vd = 2.00$$

$$\mu_u = 2.3795529E-012$$

$$\nu_u = 2.9582284E-031$$

$$d = 0.8 * h = 600.00$$

$$N_u = 11016.808$$

$$A_g = 300000.00$$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 621900.694$

where:

Vs1 = 523602.964 is calculated for jacket, with:

$$d = 600.00$$

$$A_v = 157079.633$$

$$f_y = 555.56$$

$$s = 100.00$$

Vs1 is multiplied by Col1 = 1.00

$$s/d = 0.16666667$$

Vs2 = 98297.73 is calculated for core, with:

$$d = 440.00$$

$$A_v = 100530.965$$

$$f_y = 555.56$$

$$s = 250.00$$

Vs2 is multiplied by Col2 = 1.00

$$s/d = 0.56818182$$

Vf ((11-3)-(11.4), ACI 440) = 463792.00

f = 0.95, for fully-wrapped sections

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) $\sin \theta + \cos \theta$ is replaced with $(\cot \theta + \csc \theta) \sin \alpha$ which is more a generalised expression, where θ is the angle of the crack direction (see KANEPE).

This later relation, considered as a function Vf(θ , α), is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = 45^\circ + 90^\circ = 135^\circ$

Vf = Min(|Vf(45, θ)|, |Vf(-45, θ)|), with:

total thickness per orientation, tf1 = NL*t/NoDir = 1.00

dfv = d (figure 11.2, ACI 440) = 707.00

ffe ((11-5), ACI 440) = 328.00

Ef = 82000.00

fe = 0.004, from (11.6a), ACI 440

with fu = 0.009

From (11-11), ACI 440: Vs + Vf <= 915872.391

bw = 400.00

End Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At local axis: 2

Start Of Calculation of Chord Rotation Capacity for element: column JC1 of floor 1

At local axis: 3

Integration Section: (b)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\phi = 0.90$

Chord Rotation is generally considered as Deformation-Controlled Action according to Table C7-1, ASCE 41-17.

Mean strengths are used for Deformation-Controlled Actions according to 7.5.1.3, ASCE 41-17

Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Existing Column

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

External Height, H = 400.00

External Width, W = 750.00

Internal Height, H = 200.00

Internal Width, W = 550.00

Cover Thickness, c = 25.00

Element Length, L = 3000.00

Primary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Inadequate Lap Length with $l_b/l_d = 0.30$

FRP Wrapping Data

Type: Carbon

Dry properties (design values)

Thickness, t = 1.00

Tensile Strength, $f_{fu} = 840.00$

Tensile Modulus, $E_f = 82000.00$

Elongation, $e_{fu} = 0.009$

Number of directions, NoDir = 1

Fiber orientations, $\theta_i = 0.00^\circ$

Number of layers, NL = 1
Radius of rounding corners, R = 40.00

Stepwise Properties

Bending Moment, M = 1.0488E+006
Shear Force, V2 = 1995.365
Shear Force, V3 = 1.4564234E-013
Axial Force, F = -14177.47
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: Aslt = 0.00
-Compression: Aslc = 2676.637
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: Asl,ten = 1137.257
-Compression: Asl,com = 1137.257
-Middle: Asl,mid = 402.1239
Longitudinal External Reinforcement Area Distribution (in 3 divisions)
-Tension: Asl,ten,jacket = 829.3805
-Compression: Asl,com,jacket = 829.3805
-Middle: Asl,mid,jacket = 402.1239
Longitudinal Internal Reinforcement Area Distribution (in 3 divisions)
-Tension: Asl,ten,core = 307.8761
-Compression: Asl,com,core = 307.8761
-Middle: Asl,mid,core = 0.00
Mean Diameter of Tension Reinforcement, DbL = 16.80

New component: From table 7-7, ASCE 41_17: Final chord rotation Capacity $u_R = 1.0^*$ $u = 0.00040308$
 $u = y + p = 0.00040308$

- Calculation of y -

$y = (M_y * L_s / 3) / E_{eff} = 0.00040308$ ((4.29), Biskinis Phd)
 $M_y = 2.6204E+008$
 $L_s = M/V$ (with $L_s > 0.1 * L$ and $L_s < 2 * L$) = 525.6338
From table 10.5, ASCE 41_17: $E_{eff} = factor * E_c * I_g = 1.1390E+014$
factor = 0.30
 $A_g = 300000.00$
Mean concrete strength: $f_c' = (f_c'_{jacket} * Area_{jacket} + f_c'_{core} * Area_{core}) / Area_{section} = 33.00$
 $N = 14177.47$
 $E_c * I_g = E_c * I_{g,jacket} + E_c * I_{g,core} = 3.7968E+014$

Calculation of Yielding Moment M_y

Calculation of y and M_y according to Annex 7 -

$y = \text{Min}(y_{ten}, y_{com})$
 $y_{ten} = 2.8029545E-006$
with ((10.1), ASCE 41-17) $f_y = \text{Min}(f_y, 1.25 * f_y * (l_b / d)^{2/3}) = 311.2112$
 $d = 707.00$
 $y = 0.214783$
 $A = 0.00962586$
 $B = 0.0051813$
with $pt = 0.00402142$
 $pc = 0.00402142$
 $p_v = 0.00142194$
 $N = 14177.47$
 $b = 400.00$
 $" = 0.06082037$
 $y_{comp} = 1.4829079E-005$
with f_c^* (12.3, (ACI 440)) = 33.35516

$f_c = 33.00$
 $f_l = 0.77176471$
 $b = 400.00$
 $h = 750.00$
 $A_g = 300000.00$
 From (12.9), ACI 440: $k_a = 0.14679369$
 $g = p_t + p_c + p_v = 0.00946477$
 $rc = 40.00$
 $A_e/A_c = 0.51607158$
 Effective FRP thickness, $t_f = NL*t*\cos(b1) = 1.00$
 effective strain from (12.5) and (12.12), $\epsilon_{fe} = 0.004$
 $f_u = 0.009$
 $E_f = 82000.00$
 $E_c = 26999.444$
 $y = 0.21210318$
 $A = 0.00935205$
 $B = 0.00502021$
 with $E_s = 200000.00$

 Calculation of ratio l_b/l_d

 Inadequate Lap Length with $l_b/l_d = 0.30$

 - Calculation of p -

 From table 10-8: $p = 0.00$

with:

- Columns controlled by inadequate development or splicing along the clear height because $l_b/l_d < 1$

shear control ratio $V_y E / V_{col} O E = 0.19671905$

$d = d_{external} = 707.00$

$s = s_{external} = 0.00$

$t = s_1 + s_2 + 2*t_f/bw*(f_{fe}/f_s) = 0.00761619$

jacket: $s_1 = A_{v1}*h_1/(s_1*A_g) = 0.00392699$

$A_{v1} = 157.0796$, is the total area of all stirrups parallel to loading (shear) direction

$h_1 = 750.00$

$s_1 = 100.00$

core: $s_2 = A_{v2}*h_2/(s_2*A_g) = 0.00073723$

$A_{v2} = 100.531$, is the total area of all stirrups parallel to loading (shear) direction

$h_2 = 550.00$

$s_2 = 250.00$

The term $2*t_f/bw*(f_{fe}/f_s)$ is implemented to account for FRP contribution

where $f = 2*t_f/bw$ is FRP ratio (EC8 - 3, A.4.4.3(6)) and f_{fe}/f_s normalises f to steel strength

All these variables have already been given in Shear control ratio calculation.

For the normalisation f_s of jacket is used.

$N_{UD} = 14177.47$

$A_g = 300000.00$

$f_{cE} = (f_{c_jacket}*Area_jacket + f_{c_core}*Area_core)/section_area = 33.00$

$f_{yIE} = (f_{y_ext_Long_Reinf}*Area_ext_Long_Reinf + f_{y_int_Long_Reinf}*Area_int_Long_Reinf)/Area_Tot_Long_Rein =$

555.56

$f_{ytE} = (f_{y_ext_Trans_Reinf}*s_1 + f_{y_int_Trans_Reinf}*s_2)/(s_1 + s_2) = 555.56$

$\rho_l = Area_Tot_Long_Rein/(b*d) = 0.00946477$

$b = 400.00$

$d = 707.00$

$f_{cE} = 33.00$

 End Of Calculation of Chord Rotation Capacity for element: column JC1 of floor 1

At local axis: 3

Integration Section: (b)

Calculation No. 9

column C1, Floor 1

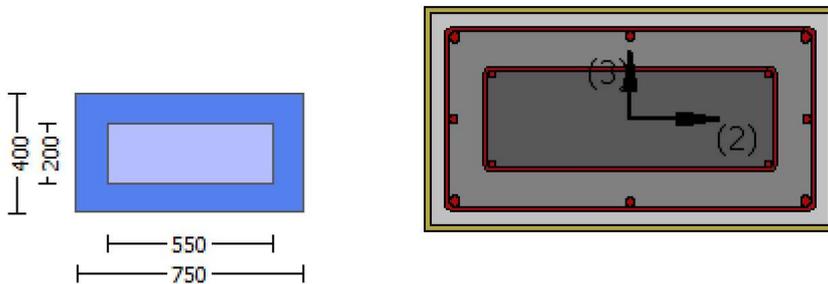
Limit State: Life Safety (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Shear capacity VRd

Edge: Start

Local Axis: (2)



Start Of Calculation of Shear Capacity for element: column JC1 of floor 1

At local axis: 2

Integration Section: (a)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$

Member Shear Force is generally considered as Force-Controlled Action according to Table C7-1, ASCE 41-17.

Lower-bound strengths are used for Force-Controlled Actions according to 7.5.1.3, ASCE 41-17

Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Primary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Existing Column

New material of Primary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Primary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of γ for displacement ductility demand, the expected (mean value) strengths are used (7.5.1.3, ASCE 41-17) because bending is considered as Deformation-Controlled Action (Table C7-1, ASCE 41-17).

Jacket

New material: Concrete Strength, $f_c = f_{cm} = 33.00$

New material: Steel Strength, $f_s = f_{sm} = 555.56$

Existing Column

New material: Concrete Strength, $f_c = f_{cm} = 33.00$

New material: Steel Strength, $f_s = f_{sm} = 555.56$

External Height, $H = 400.00$
External Width, $W = 750.00$
Internal Height, $H = 200.00$
Internal Width, $W = 550.00$
Cover Thickness, $c = 25.00$
Element Length, $L = 3000.00$
Primary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Inadequate Lap Length with $l_o/l_{ou,min} = l_b/l_d = 0.30$
FRP Wrapping Data
Type: Carbon
Dry properties (design values)
Thickness, $t = 1.00$
Tensile Strength, $f_{fu} = 840.00$
Tensile Modulus, $E_f = 82000.00$
Elongation, $ε_{fu} = 0.009$
Number of directions, $N_{oDir} = 1$
Fiber orientations, $b_i = 0.00^\circ$
Number of layers, $N_L = 1$
Radius of rounding corners, $R = 40.00$

Stepwise Properties

EDGE -A-
Bending Moment, $M_a = -1.0982E+007$
Shear Force, $V_a = -3112.69$
EDGE -B-
Bending Moment, $M_b = 1.6361E+006$
Shear Force, $V_b = 3112.69$
BOTH EDGES
Axial Force, $F = -15947.314$
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: $A_{sl,t} = 0.00$
-Compression: $A_{sl,c} = 2676.637$
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: $A_{sl,ten} = 1137.257$
-Compression: $A_{sl,com} = 1137.257$
-Middle: $A_{sl,mid} = 402.1239$
Mean Diameter of Tension Reinforcement, $D_{bL,ten} = 16.80$

New component: From table 7-7, ASCE 41_17: Final Shear Capacity $V_R = 1.0 \cdot V_n = 948750.938$
 V_n ((10.3), ASCE 41-17) = $k_n l \cdot V_{CoI0} = 948750.938$
 $V_{CoI} = 948750.938$
 $k_n l = 1.00$
displacement_ductility_demand = 0.02136767

NOTE: In expression (10-3) ' $V_s = A_v \cdot f_y \cdot d / s$ ' is replaced by ' $V_s + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)
Mean concrete strength: $f'_c = (f'_c \cdot \text{jacket} \cdot \text{Area}_{\text{jacket}} + f'_c \cdot \text{core} \cdot \text{Area}_{\text{core}}) / \text{Area}_{\text{section}} = 25.00$, but $f_c^{0.5} \leq 8.3$
MPa (22.5.3.1, ACI 318-14)
 $M/Vd = 4.00$
 $M_u = 1.0982E+007$
 $V_u = 3112.69$
 $d = 0.8 \cdot h = 600.00$
 $N_u = 15947.314$
 $A_g = 300000.00$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 559706.147$

where:

$V_{s1} = 471238.898$ is calculated for jacket, with:

$d = 600.00$

$A_v = 157079.633$

$f_y = 500.00$

$s = 100.00$

V_{s1} is multiplied by $Col1 = 1.00$

$s/d = 0.16666667$

$V_{s2} = 88467.249$ is calculated for core, with:

$d = 440.00$

$A_v = 100530.965$

$f_y = 500.00$

$s = 250.00$

V_{s2} is multiplied by $Col2 = 1.00$

$s/d = 0.56818182$

V_f ((11-3)-(11.4), ACI 440) = 463792.00

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression,

where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = 45^\circ$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, \theta)|)$, with:

total thickness per orientation, $t_{f1} = N_L * t / N_{oDir} = 1.00$

$d_{fv} = d$ (figure 11.2, ACI 440) = 707.00

f_{fe} ((11-5), ACI 440) = 328.00

$E_f = 82000.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.009$

From (11-11), ACI 440: $V_s + V_f \leq 797164.595$

$b_w = 400.00$

displacement ductility demand is calculated as δ / y

- Calculation of δ / y for END A -

for rotation axis 3 and integ. section (a)

From analysis, chord rotation $\theta = 5.7929149E-005$

$y = (M_y * L_s / 3) / E_{eff} = 0.00271107$ ((4.29), Biskinis Phd))

$M_y = 2.6257E+008$

$L_s = M/V$ (with $L_s > 0.1 * L$ and $L_s < 2 * L$) = 3528.173

From table 10.5, ASCE 41_17: $E_{eff} = \text{factor} * E_c * I_g = 1.1390E+014$

factor = 0.30

$A_g = 300000.00$

Mean concrete strength: $f'_c = (f'_{c_jacket} * Area_{jacket} + f'_{c_core} * Area_{core}) / Area_{section} = 33.00$

$N = 15947.314$

$E_c * I_g = E_{c_jacket} * I_{g_jacket} + E_{c_core} * I_{g_core} = 3.7968E+014$

Calculation of Yielding Moment M_y

Calculation of δ / y and M_y according to Annex 7 -

$y = \text{Min}(y_{ten}, y_{com})$

$y_{ten} = 2.8044129E-006$

with ((10.1), ASCE 41-17) $f_y = \text{Min}(f_y, 1.25 * f_y * ((l_b/d)^{2/3})) = 311.2112$

$d = 707.00$

$y = 0.21519134$

$A = 0.00964597$

$B = 0.00520141$

with $pt = 0.00402142$

$pc = 0.00402142$

pv = 0.00142194
N = 15947.314
b = 400.00
" = 0.06082037
y_comp = 1.4823586E-005
with fc* (12.3, (ACI 440)) = 33.35516
fc = 33.00
fl = 0.77176471
b = 400.00
h = 750.00
Ag = 300000.00
From (12.9), ACI 440: ka = 0.14679369
g = pt + pc + pv = 0.00946477
rc = 40.00
Ae/Ac = 0.51607158
Effective FRP thickness, tf = NL*t*cos(b1) = 1.00
effective strain from (12.5) and (12.12), efe = 0.004
fu = 0.009
Ef = 82000.00
Ec = 26999.444
y = 0.21218177
A = 0.00933798
B = 0.00502021
with Es = 200000.00

Calculation of ratio lb/ld

Inadequate Lap Length with lb/ld = 0.30

End Of Calculation of Shear Capacity for element: column JC1 of floor 1

At local axis: 2

Integration Section: (a)

Calculation No. 10

column C1, Floor 1

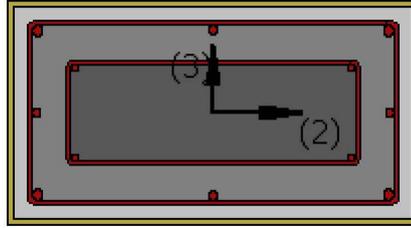
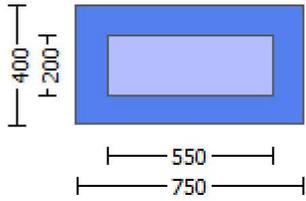
Limit State: Life Safety (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Chord rotation capacity (u)

Edge: Start

Local Axis: (2)



Start Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At Shear local axis: 3
 (Bending local axis: 2)
 Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$
 Mean strength values are used for both shear and moment calculations.
 Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$
 New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$
 Concrete Elasticity, $E_c = 26999.444$
 Steel Elasticity, $E_s = 200000.00$

Existing Column

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$
 New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$
 Concrete Elasticity, $E_c = 26999.444$
 Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of moment strengths,
 the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

Jacket

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

Existing Column

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

#####

External Height, $H = 400.00$

External Width, $W = 750.00$

Internal Height, $H = 200.00$

Internal Width, $W = 550.00$

Cover Thickness, $c = 25.00$

Mean Confinement Factor overall section = 1.0435

Element Length, $L = 3000.00$

Primary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Inadequate Lap Length with $l_o/l_{ou, \min} = 0.30$

FRP Wrapping Data

Type: Carbon

Dry properties (design values)

Thickness, $t = 1.00$

Tensile Strength, $f_{fu} = 840.00$

Tensile Modulus, $E_f = 82000.00$

Elongation, $e_{fu} = 0.009$

Number of directions, $N_{oDir} = 1$

Fiber orientations, $b_i: 0.00^\circ$

Number of layers, $N_L = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 3

EDGE -A-

Shear Force, $V_a = 3.4586346E-031$

EDGE -B-

Shear Force, $V_b = -3.4586346E-031$

BOTH EDGES

Axial Force, $F = -11016.808$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $As_c = 2676.637$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{t,ten} = 1137.257$

-Compression: $As_{l,com} = 1137.257$

-Middle: $As_{l,mid} = 402.1239$

Calculation of Shear Capacity ratio, $V_e/V_r = 0.14265244$

Member Controlled by Flexure ($V_e/V_r < 1$)

Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 121055.918$

with

$M_{pr1} = \text{Max}(M_{u1+}, M_{u1-}) = 1.8158E+008$

$M_{u1+} = 1.8158E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination

$M_{u1-} = 1.8158E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination

$M_{pr2} = \text{Max}(M_{u2+}, M_{u2-}) = 1.8158E+008$

$M_{u2+} = 1.8158E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the static loading combination

$M_{u2-} = 1.8158E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the static loading combination

Calculation of M_{u1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$\phi_u = 1.7255512E-005$

$M_u = 1.8158E+008$

with full section properties:

$b = 750.00$

$d = 357.00$

$d' = 43.00$

$v = 0.00124684$

$N = 11016.808$

$f_c = 33.00$

α (5A.5, TBDY) = 0.002

Final value of ϕ_u : $\phi_u^* = \text{shear_factor} * \text{Max}(\phi_u, \phi_c) = 0.01006403$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $\phi_u = 0.01006403$

where ϕ_u ((5.4c), TBDY) = $\alpha s_e * \text{sh}, \text{min}(f_{ywe}/f_{ce} + \text{Min}(f_x, f_y)) = 0.02692906$

where $f = \alpha f_p f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.02504965$

$\alpha f = 0.38744444$

$b = 750.00$

$h = 400.00$

From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.00266667$

$bw = 750.00$

effective stress from (A.35), $f_{f,e} = 800.0873$

fy = 0.04491794
af = 0.38744444
b = 400.00
h = 750.00
From EC8 A.4.4.3(6), pf = 2tf/bw = 0.005
bw = 400.00
effective stress from (A.35), ff,e = 765.1636

R = 40.00
Effective FRP thickness, tf = NL*t*Cos(b1) = 1.00
fu,f = 840.00
Ef = 82000.00
u,f = 0.015
ase ((5.4d), TBDY) = (ase1*Aext+ase2*Aint)/Asec = 0.12601038
ase1 = 0.12601038
bo_1 = 690.00
ho_1 = 340.00
bi2_1 = 1.1834E+006
ase2 = Max(ase1,ase2) = 0.12601038
bo_2 = 542.00
ho_2 = 192.00
bi2_2 = 661256.00
psh,min*Fywe = Min(psh,x*Fywe , psh,y*Fywe) = 1.64062

Expression ((5.4d), TBDY) for psh,min*Fywe has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh_x*Fywe = psh1*Fywe1+ps2*Fywe2 = 1.64062
ps1 (external) = (Ash1*h1/s1)/Asec = 0.0020944
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 400.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00026808
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 200.00

psh_y*Fywe = psh1*Fywe1+ps2*Fywe2 = 3.23907
ps1 (external) = (Ash1*h1/s1)/Asec = 0.00392699
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 750.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00073723
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/d = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered

characteristic value $f_{sy1} = f_{s1}/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $f_{s1} = (f_{s,jacket} \cdot A_{s,ten,jacket} + f_{s,core} \cdot A_{s,ten,core})/A_{s,ten} = 389.0139$

with $E_{s1} = (E_{s,jacket} \cdot A_{s,ten,jacket} + E_{s,core} \cdot A_{s,ten,core})/A_{s,ten} = 200000.00$

$y_2 = 0.00140044$

$sh_2 = 0.0044814$

$ft_2 = 466.8167$

$fy_2 = 389.0139$

$su_2 = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

$l_o/l_{ou,min} = l_b/l_{d,min} = 0.30$

$su_2 = 0.4 \cdot esu_{2,nominal} ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu_{2,nominal} = 0.08$,

For calculation of $esu_{2,nominal}$ and y_2, sh_2, ft_2, fy_2 , it is considered

characteristic value $f_{sy2} = f_{s2}/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $f_{s2} = (f_{s,jacket} \cdot A_{s,com,jacket} + f_{s,core} \cdot A_{s,com,core})/A_{s,com} = 389.0139$

with $E_{s2} = (E_{s,jacket} \cdot A_{s,com,jacket} + E_{s,core} \cdot A_{s,com,core})/A_{s,com} = 200000.00$

$y_v = 0.00140044$

$sh_v = 0.0044814$

$ft_v = 466.8167$

$fy_v = 389.0139$

$su_v = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

$l_o/l_{ou,min} = l_b/l_d = 0.30$

$su_v = 0.4 \cdot esuv_{nominal} ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esuv_{nominal} = 0.08$,

considering characteristic value $f_{syv} = f_{sv}/1.2$, from table 5.1, TBDY

For calculation of $esuv_{nominal}$ and y_v, sh_v, ft_v, fy_v , it is considered

characteristic value $f_{syv} = f_{sv}/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $f_{sv} = (f_{s,jacket} \cdot A_{s,mid,jacket} + f_{s,mid} \cdot A_{s,mid,core})/A_{s,mid} = 389.0139$

with $E_{sv} = (E_{s,jacket} \cdot A_{s,mid,jacket} + E_{s,mid} \cdot A_{s,mid,core})/A_{s,mid} = 200000.00$

$1 = A_{s,ten}/(b \cdot d) \cdot (f_{s1}/f_c) = 0.0500703$

$2 = A_{s,com}/(b \cdot d) \cdot (f_{s2}/f_c) = 0.0500703$

$v = A_{s,mid}/(b \cdot d) \cdot (f_{sv}/f_c) = 0.01770442$

and confined core properties:

$b = 690.00$

$d = 327.00$

$d' = 13.00$

$f_{cc} (5A.2, TBDY) = 34.43547$

$cc (5A.5, TBDY) = 0.00243499$

$c = \text{confinement factor} = 1.0435$

$1 = A_{s,ten}/(b \cdot d) \cdot (f_{s1}/f_c) = 0.05941729$

$2 = A_{s,com}/(b \cdot d) \cdot (f_{s2}/f_c) = 0.05941729$

$v = A_{s,mid}/(b \cdot d) \cdot (f_{sv}/f_c) = 0.02100943$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

---->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

---->

$su (4.9) = 0.16886055$

$Mu = MRc (4.14) = 1.8158E+008$

$u = su (4.1) = 1.7255512E-005$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of Mu_1 -

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$$u = 1.7255512E-005$$

$$\mu = 1.8158E+008$$

with full section properties:

$$b = 750.00$$

$$d = 357.00$$

$$d' = 43.00$$

$$v = 0.00124684$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$c_o(5A.5, TBDY) = 0.002$$

$$\text{Final value of } \phi_u: \phi_u^* = \text{shear_factor} * \text{Max}(\phi_u, \phi_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \phi_u = 0.01006403$$

$$\omega_e(5.4c, TBDY) = a_{se} * s_{h, \min} * f_{ywe} / f_{ce} + \text{Min}(\phi_x, \phi_y) = 0.02692906$$

where $\phi = a_f * p_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\phi_x = 0.02504965$$

$$a_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f / b_w = 0.00266667$$

$$b_w = 750.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 800.0873$$

$$\phi_y = 0.04491794$$

$$a_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f / b_w = 0.005$$

$$b_w = 400.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L * t * \text{Cos}(b_1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{,f} = 0.015$$

$$a_{se}((5.4d), TBDY) = (a_{se1} * A_{ext} + a_{se2} * A_{int}) / A_{sec} = 0.12601038$$

$$a_{se1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i2,1} = 1.1834E+006$$

$$a_{se2} = \text{Max}(a_{se1}, a_{se2}) = 0.12601038$$

$$b_{o,2} = 542.00$$

$$h_{o,2} = 192.00$$

$$b_{i2,2} = 661256.00$$

$$p_{sh, \min} * F_{ywe} = \text{Min}(p_{sh,x} * F_{ywe}, p_{sh,y} * F_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $p_{sh, \min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$p_{sh,x} * F_{ywe} = p_{sh1} * F_{ywe1} + p_{sh2} * F_{ywe2} = 1.64062$$

$$p_{sh1}(\text{external}) = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$$

$$A_{sh1} = A_{stir,1} * n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

$$h_1 = 400.00$$

$$p_{sh2}(\text{internal}) = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00026808$$

$$A_{sh2} = A_{stir,2} * n_{s,2} = 100.531$$

$$\text{No stirrups, } n_{s,2} = 2.00$$

$$h_2 = 200.00$$

$$p_{sh,y} * F_{ywe} = p_{sh1} * F_{ywe1} + p_{sh2} * F_{ywe2} = 3.23907$$

$$p_{sh1}(\text{external}) = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.00392699$$

$$\text{Ash1} = \text{Astir}_1 * \text{ns}_1 = 157.0796$$

$$\text{No stirups, ns}_1 = 2.00$$

$$h1 = 750.00$$

$$\text{ps2 (internal)} = (\text{Ash2} * h2 / s2) / \text{Asec} = 0.00073723$$

$$\text{Ash2} = \text{Astir}_2 * \text{ns}_2 = 100.531$$

$$\text{No stirups, ns}_2 = 2.00$$

$$h2 = 550.00$$

$$\text{Asec} = 300000.00$$

$$s1 = 100.00$$

$$s2 = 250.00$$

$$\text{fywe1} = 694.45$$

$$\text{fywe2} = 694.45$$

$$\text{fce} = 33.00$$

$$\text{From } ((5.A.5), \text{TBDY}), \text{TBDY: } \text{cc} = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$y1 = 0.00140044$$

$$\text{sh1} = 0.0044814$$

$$\text{ft1} = 466.8167$$

$$\text{fy1} = 389.0139$$

$$\text{su1} = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$\text{lo/lou,min} = \text{lb/ld} = 0.30$$

$$\text{su1} = 0.4 * \text{esu1_nominal} ((5.5), \text{TBDY}) = 0.032$$

$$\text{From table 5A.1, TBDY: esu1_nominal} = 0.08,$$

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

$$y1, \text{sh1,ft1,fy1, are also multiplied by } \text{Min}(1, 1.25 * (\text{lb/ld})^{2/3}), \text{ from 10.3.5, ASCE 41-17.}$$

$$\text{with fs1} = (\text{fs,jacket} * \text{Asl,ten,jacket} + \text{fs,core} * \text{Asl,ten,core}) / \text{Asl,ten} = 389.0139$$

$$\text{with Es1} = (\text{Es,jacket} * \text{Asl,ten,jacket} + \text{Es,core} * \text{Asl,ten,core}) / \text{Asl,ten} = 200000.00$$

$$y2 = 0.00140044$$

$$\text{sh2} = 0.0044814$$

$$\text{ft2} = 466.8167$$

$$\text{fy2} = 389.0139$$

$$\text{su2} = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$\text{lo/lou,min} = \text{lb/lb,min} = 0.30$$

$$\text{su2} = 0.4 * \text{esu2_nominal} ((5.5), \text{TBDY}) = 0.032$$

$$\text{From table 5A.1, TBDY: esu2_nominal} = 0.08,$$

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

$$y1, \text{sh1,ft1,fy1, are also multiplied by } \text{Min}(1, 1.25 * (\text{lb/ld})^{2/3}), \text{ from 10.3.5, ASCE 41-17.}$$

$$\text{with fs2} = (\text{fs,jacket} * \text{Asl,com,jacket} + \text{fs,core} * \text{Asl,com,core}) / \text{Asl,com} = 389.0139$$

$$\text{with Es2} = (\text{Es,jacket} * \text{Asl,com,jacket} + \text{Es,core} * \text{Asl,com,core}) / \text{Asl,com} = 200000.00$$

$$yv = 0.00140044$$

$$\text{shv} = 0.0044814$$

$$\text{ftv} = 466.8167$$

$$\text{fyv} = 389.0139$$

$$\text{suv} = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$\text{lo/lou,min} = \text{lb/ld} = 0.30$$

$$\text{suv} = 0.4 * \text{esuv_nominal} ((5.5), \text{TBDY}) = 0.032$$

$$\text{From table 5A.1, TBDY: esuv_nominal} = 0.08,$$

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

$$y1, \text{sh1,ft1,fy1, are also multiplied by } \text{Min}(1, 1.25 * (\text{lb/ld})^{2/3}), \text{ from 10.3.5, ASCE 41-17.}$$

$$\text{with fsv} = (\text{fs,jacket} * \text{Asl,mid,jacket} + \text{fs,mid} * \text{Asl,mid,core}) / \text{Asl,mid} = 389.0139$$

$$\text{with Esv} = (\text{Es,jacket} * \text{Asl,mid,jacket} + \text{Es,mid} * \text{Asl,mid,core}) / \text{Asl,mid} = 200000.00$$

$$1 = \text{Asl,ten} / (\text{b} * \text{d}) * (\text{fs1} / \text{fc}) = 0.0500703$$

$$2 = \text{Asl,com} / (\text{b} * \text{d}) * (\text{fs2} / \text{fc}) = 0.0500703$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.01770442$$

and confined core properties:

$$b = 690.00$$

$$d = 327.00$$

$$d' = 13.00$$

$$f_{cc} (5A.2, TBDY) = 34.43547$$

$$c_c (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.05941729$$

$$2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.05941729$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.02100943$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$s_u (4.9) = 0.16886055$$

$$\mu_u = M_{Rc} (4.14) = 1.8158E+008$$

$$u = s_u (4.1) = 1.7255512E-005$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of μ_{u2+}

Calculation of ultimate curvature μ_u according to 4.1, Biskinis/Fardis 2013:

$$u = 1.7255512E-005$$

$$\mu_u = 1.8158E+008$$

with full section properties:

$$b = 750.00$$

$$d = 357.00$$

$$d' = 43.00$$

$$v = 0.00124684$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$c_o (5A.5, TBDY) = 0.002$$

$$\text{Final value of } c_u: c_u^* = \text{shear_factor} * \text{Max}(c_u, c_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } c_u = 0.01006403$$

$$w_e ((5.4c), TBDY) = a_{se} * s_{h,min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$$

where $f = a_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.02504965$$

$$a_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.00266667$$

$$b_w = 750.00$$

$$\text{effective stress from (A.35), } f_{fe} = 800.0873$$

$$f_y = 0.04491794$$

$$a_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.005$$

$$b_w = 400.00$$

$$\text{effective stress from (A.35), } f_{fe} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L^* t \text{Cos}(b_1) = 1.00$$

$f_{u,f} = 840.00$
 $E_f = 82000.00$
 $u,f = 0.015$
 $ase((5.4d), TBDY) = (ase1 \cdot A_{ext} + ase2 \cdot A_{int}) / A_{sec} = 0.12601038$
 $ase1 = 0.12601038$
 $bo_1 = 690.00$
 $ho_1 = 340.00$
 $bi2_1 = 1.1834E+006$
 $ase2 = \text{Max}(ase1, ase2) = 0.12601038$
 $bo_2 = 542.00$
 $ho_2 = 192.00$
 $bi2_2 = 661256.00$

$psh_{min} \cdot F_{ywe} = \text{Min}(psh_x \cdot F_{ywe}, psh_y \cdot F_{ywe}) = 1.64062$

Expression ((5.4d), TBDY) for $psh_{min} \cdot F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh_x \cdot F_{ywe} = psh1 \cdot F_{ywe1} + ps2 \cdot F_{ywe2} = 1.64062$
 $ps1 \text{ (external)} = (Ash1 \cdot h1 / s1) / A_{sec} = 0.0020944$
 $Ash1 = Astir_1 \cdot ns_1 = 157.0796$
 No stirrups, $ns_1 = 2.00$
 $h1 = 400.00$
 $ps2 \text{ (internal)} = (Ash2 \cdot h2 / s2) / A_{sec} = 0.00026808$
 $Ash2 = Astir_2 \cdot ns_2 = 100.531$
 No stirrups, $ns_2 = 2.00$
 $h2 = 200.00$

$psh_y \cdot F_{ywe} = psh1 \cdot F_{ywe1} + ps2 \cdot F_{ywe2} = 3.23907$
 $ps1 \text{ (external)} = (Ash1 \cdot h1 / s1) / A_{sec} = 0.00392699$
 $Ash1 = Astir_1 \cdot ns_1 = 157.0796$
 No stirrups, $ns_1 = 2.00$
 $h1 = 750.00$
 $ps2 \text{ (internal)} = (Ash2 \cdot h2 / s2) / A_{sec} = 0.00073723$
 $Ash2 = Astir_2 \cdot ns_2 = 100.531$
 No stirrups, $ns_2 = 2.00$
 $h2 = 550.00$

$A_{sec} = 300000.00$
 $s1 = 100.00$
 $s2 = 250.00$
 $f_{ywe1} = 694.45$
 $f_{ywe2} = 694.45$
 $f_{ce} = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00243499$
 $c = \text{confinement factor} = 1.0435$

$y1 = 0.00140044$
 $sh1 = 0.0044814$

$ft1 = 466.8167$
 $fy1 = 389.0139$

$su1 = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$lo/lo_{min} = lb/ld = 0.30$

$su1 = 0.4 \cdot esu1_{nominal}((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu1_{nominal} = 0.08$,

For calculation of $esu1_{nominal}$ and $y1, sh1, ft1, fy1$, it is considered characteristic value $fsy1 = fs1 / 1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = (fs_{jacket} \cdot A_{sl,ten,jacket} + fs_{core} \cdot A_{sl,ten,core}) / A_{sl,ten} = 389.0139$

with $Es1 = (Es_{jacket} \cdot A_{sl,ten,jacket} + Es_{core} \cdot A_{sl,ten,core}) / A_{sl,ten} = 200000.00$

$y2 = 0.00140044$
 $sh2 = 0.0044814$

$ft2 = 466.8167$
 $fy2 = 389.0139$

$su2 = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044

shv = 0.0044814

ftv = 466.8167

fyv = 389.0139

suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.0500703

2 = Asl,com/(b*d)*(fs2/fc) = 0.0500703

v = Asl,mid/(b*d)*(fsv/fc) = 0.01770442

and confined core properties:

b = 690.00

d = 327.00

d' = 13.00

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

1 = Asl,ten/(b*d)*(fs1/fc) = 0.05941729

2 = Asl,com/(b*d)*(fs2/fc) = 0.05941729

v = Asl,mid/(b*d)*(fsv/fc) = 0.02100943

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

v < vs,y2 - LHS eq.(4.5) is satisfied

--->

su (4.9) = 0.16886055

Mu = MRc (4.14) = 1.8158E+008

u = su (4.1) = 1.7255512E-005

Calculation of ratio lb/ld

Inadequate Lap Length with lb/ld = 0.30

Calculation of Mu2-

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

u = 1.7255512E-005

Mu = 1.8158E+008

with full section properties:

b = 750.00

d = 357.00

d' = 43.00

$$v = 0.00124684$$

$$N = 11016.808$$

$$fc = 33.00$$

$$co \text{ (5A.5, TBDY)} = 0.002$$

$$\text{Final value of } cu: cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } cu = 0.01006403$$

$$\text{we ((5.4c), TBDY)} = ase * sh_{min} * fy_{we} / f_{ce} + \text{Min}(fx, fy) = 0.02692906$$

where $f = af * pf * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$fx = 0.02504965$$

$$af = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } pf = 2tf/bw = 0.00266667$$

$$bw = 750.00$$

$$\text{effective stress from (A.35), } ff_e = 800.0873$$

$$fy = 0.04491794$$

$$af = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } pf = 2tf/bw = 0.005$$

$$bw = 400.00$$

$$\text{effective stress from (A.35), } ff_e = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } tf = NL * t * \text{Cos}(b1) = 1.00$$

$$fu_f = 840.00$$

$$Ef = 82000.00$$

$$u_f = 0.015$$

$$ase \text{ ((5.4d), TBDY)} = (ase1 * A_{ext} + ase2 * A_{int}) / A_{sec} = 0.12601038$$

$$ase1 = 0.12601038$$

$$bo_1 = 690.00$$

$$ho_1 = 340.00$$

$$bi2_1 = 1.1834E+006$$

$$ase2 = \text{Max}(ase1, ase2) = 0.12601038$$

$$bo_2 = 542.00$$

$$ho_2 = 192.00$$

$$bi2_2 = 661256.00$$

$$psh_{min} * Fy_{we} = \text{Min}(psh_x * Fy_{we}, psh_y * Fy_{we}) = 1.64062$$

Expression ((5.4d), TBDY) for $psh_{min} * Fy_{we}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$psh_x * Fy_{we} = psh1 * Fy_{we1} + ps2 * Fy_{we2} = 1.64062$$

$$ps1 \text{ (external)} = (Ash1 * h1 / s1) / A_{sec} = 0.0020944$$

$$Ash1 = Astir_1 * ns_1 = 157.0796$$

$$\text{No stirrups, } ns_1 = 2.00$$

$$h1 = 400.00$$

$$ps2 \text{ (internal)} = (Ash2 * h2 / s2) / A_{sec} = 0.00026808$$

$$Ash2 = Astir_2 * ns_2 = 100.531$$

$$\text{No stirrups, } ns_2 = 2.00$$

$$h2 = 200.00$$

$$psh_y * Fy_{we} = psh1 * Fy_{we1} + ps2 * Fy_{we2} = 3.23907$$

$$ps1 \text{ (external)} = (Ash1 * h1 / s1) / A_{sec} = 0.00392699$$

$$Ash1 = Astir_1 * ns_1 = 157.0796$$

$$\text{No stirrups, } ns_1 = 2.00$$

$$h1 = 750.00$$

$$ps2 \text{ (internal)} = (Ash2 * h2 / s2) / A_{sec} = 0.00073723$$

$$Ash2 = Astir_2 * ns_2 = 100.531$$

$$\text{No stirrups, } ns_2 = 2.00$$

$$h2 = 550.00$$

$$A_{sec} = 300000.00$$

$$s1 = 100.00$$

$$s2 = 250.00$$

fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A.5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044
sh2 = 0.0044814
ft2 = 466.8167
fy2 = 389.0139
su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044
shv = 0.0044814
ftv = 466.8167
fyv = 389.0139
suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.0500703

2 = Asl,com/(b*d)*(fs2/fc) = 0.0500703

v = Asl,mid/(b*d)*(fsv/fc) = 0.01770442

and confined core properties:

b = 690.00
d = 327.00
d' = 13.00

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

1 = Asl,ten/(b*d)*(fs1/fc) = 0.05941729

2 = Asl,com/(b*d)*(fs2/fc) = 0.05941729

v = Asl,mid/(b*d)*(fsv/fc) = 0.02100943

Case/Assumption: Unconfined full section - Steel rupture
' satisfies Eq. (4.3)

--->
v < vs,y2 - LHS eq.(4.5) is satisfied

---->
su (4.9) = 0.16886055
Mu = MRc (4.14) = 1.8158E+008
u = su (4.1) = 1.7255512E-005

Calculation of ratio lb/l_d

Inadequate Lap Length with lb/l_d = 0.30

Calculation of Shear Strength Vr = Min(Vr1,Vr2) = 848607.437

Calculation of Shear Strength at edge 1, Vr1 = 848607.437
Vr1 = VCol ((10.3), ASCE 41-17) = knl*VCol0
VCol0 = 848607.437
knl = 1 (zero step-static loading)

NOTE: In expression (10-3) 'Vs = Av*fy*d/s' is replaced by 'Vs+ f*Vf'
where Vf is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)
Mean concrete strength: fc' = (fc'_jacket*Area_jacket + fc'_core*Area_core)/Area_section = 33.00, but fc'^0.5 <= 8.3
MPa (22.5.3.1, ACI 318-14)

M/Vd = 2.00
Mu = 3.5345895E-012
Vu = 3.4586346E-031
d = 0.8*h = 320.00
Nu = 11016.808
Ag = 300000.00
From (11.5.4.8), ACI 318-14: Vs = Vs1 + Vs2 = 279254.914

where:

Vs1 = 279254.914 is calculated for jacket, with:

d = 320.00
Av = 157079.633
fy = 555.56
s = 100.00

Vs1 is multiplied by Col1 = 1.00

s/d = 0.3125

Vs2 = 0.00 is calculated for core, with:

d = 160.00
Av = 100530.965
fy = 555.56
s = 250.00

Vs2 is multiplied by Col2 = 0.00

s/d = 1.5625

Vf ((11-3)-(11.4), ACI 440) = 234192.00

f = 0.95, for fully-wrapped sections

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) sin + cos is replaced with (cot + cota)sina which is more a generalised expression,

where is the angle of the crack direction (see KANEPE).

This later relation, considered as a function Vf(,), is implemented for every different fiber orientation ai,
as well as for 2 crack directions, = 45° and = -45° to take into consideration the cyclic seismic loading.

orientation 1: 1 = b1 + 90° = 90.00

Vf = Min(|Vf(45, 1)|, |Vf(-45,a1)|), with:

total thickness per orientation, tf1 = NL*t/NoDir = 1.00

dfv = d (figure 11.2, ACI 440) = 357.00

ffe ((11-5), ACI 440) = 328.00

Ef = 82000.00

fe = 0.004, from (11.6a), ACI 440

with fu = 0.009

From (11-11), ACI 440: $V_s + V_f \leq 915872.391$
 $bw = 750.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 848607.437$

$V_{r2} = V_{Col} ((10.3), ASCE 41-17) = knl * V_{Col0}$

$V_{Col0} = 848607.437$

$knl = 1$ (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v * f_y * d / s$ ' is replaced by ' $V_s + f * V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

$= 1$ (normal-weight concrete)

Mean concrete strength: $f_c' = (f_c'_{jacket} * Area_{jacket} + f_c'_{core} * Area_{core}) / Area_{section} = 33.00$, but $f_c'^{0.5} \leq 8.3$
MPa (22.5.3.1, ACI 318-14)

$M / Vd = 2.00$

$\mu_u = 3.5345895E-012$

$V_u = 3.4586346E-031$

$d = 0.8 * h = 320.00$

$N_u = 11016.808$

$A_g = 300000.00$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 279254.914$

where:

$V_{s1} = 279254.914$ is calculated for jacket, with:

$d = 320.00$

$A_v = 157079.633$

$f_y = 555.56$

$s = 100.00$

V_{s1} is multiplied by $Col1 = 1.00$

$s/d = 0.3125$

$V_{s2} = 0.00$ is calculated for core, with:

$d = 160.00$

$A_v = 100530.965$

$f_y = 555.56$

$s = 250.00$

V_{s2} is multiplied by $Col2 = 0.00$

$s/d = 1.5625$

$V_f ((11-3)-(11.4), ACI 440) = 234192.00$

$f = 0.95$, for fully-wrapped sections

$w_f / s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot) \sin \alpha$ which is more a generalised expression,
where α is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\alpha)$, is implemented for every different fiber orientation α_i ,
as well as for 2 crack directions, $\alpha = 45^\circ$ and $\alpha = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\alpha = 45^\circ$

$V_f = \text{Min}(|V_f(45, \alpha)|, |V_f(-45, \alpha)|)$, with:

total thickness per orientation, $t_{f1} = NL * t / N_{Dir} = 1.00$

$d_{fv} = d$ (figure 11.2, ACI 440) = 357.00

$f_{fe} ((11-5), ACI 440) = 328.00$

$E_f = 82000.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.009$

From (11-11), ACI 440: $V_s + V_f \leq 915872.391$

$bw = 750.00$

End Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At local axis: 3

Start Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At Shear local axis: 2

(Bending local axis: 3)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$

Mean strength values are used for both shear and moment calculations.

Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Existing Column

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of moment strengths,

the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

Jacket

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

Existing Column

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

#####

External Height, $H = 400.00$

External Width, $W = 750.00$

Internal Height, $H = 200.00$

Internal Width, $W = 550.00$

Cover Thickness, $c = 25.00$

Mean Confinement Factor overall section = 1.0435

Element Length, $L = 3000.00$

Primary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Inadequate Lap Length with $l_o/l_{ou, \min} = 0.30$

FRP Wrapping Data

Type: Carbon

Dry properties (design values)

Thickness, $t = 1.00$

Tensile Strength, $f_{fu} = 840.00$

Tensile Modulus, $E_f = 82000.00$

Elongation, $e_{fu} = 0.009$

Number of directions, $N_{oDir} = 1$

Fiber orientations, $b_i = 0.00^\circ$

Number of layers, $N_L = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 2

EDGE -A-

Shear Force, $V_a = 2.9582284E-031$

EDGE -B-

Shear Force, $V_b = -2.9582284E-031$

BOTH EDGES

Axial Force, $F = -11016.808$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $As_c = 2676.637$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{t, \text{ten}} = 1137.257$

-Compression: $As_{c, \text{com}} = 1137.257$

-Middle: $As_{c, \text{mid}} = 402.1239$

Calculation of Shear Capacity ratio , $V_e/V_r = 0.19671905$

Member Controlled by Flexure ($V_e/V_r < 1$)

Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 248405.514$

with

$M_{pr1} = \text{Max}(M_{u1+}, M_{u1-}) = 3.7261E+008$

$M_{u1+} = 3.7261E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination

$M_{u1-} = 3.7261E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination

$M_{pr2} = \text{Max}(M_{u2+}, M_{u2-}) = 3.7261E+008$

$M_{u2+} = 3.7261E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the the static loading combination

$M_{u2-} = 3.7261E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the the static loading combination

Calculation of M_{u1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$\phi_u = 8.5888248E-006$

$M_u = 3.7261E+008$

with full section properties:

$b = 400.00$

$d = 707.00$

$d' = 43.00$

$v = 0.00118049$

$N = 11016.808$

$f_c = 33.00$

α_1 (5A.5, TBDY) = 0.002

Final value of ϕ_{cu} : $\phi_{cu}^* = \text{shear_factor} * \text{Max}(\phi_{cu}, \phi_{cc}) = 0.01006403$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $\phi_{cu} = 0.01006403$

ω_e ((5.4c), TBDY) = $\alpha_{se} * \text{sh}_{\min} * f_{ywe} / f_{ce} + \text{Min}(\phi_x, \phi_y) = 0.02692906$

where $\phi = \alpha_f * \rho_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

 $\phi_x = 0.02504965$

$\alpha_f = 0.38744444$

$b = 750.00$

$h = 400.00$

From EC8 A.4.4.3(6), $\rho_f = 2t_f/bw = 0.00266667$

$bw = 750.00$

effective stress from (A.35), $f_{fe} = 800.0873$

 $\phi_y = 0.04491794$

$\alpha_f = 0.38744444$

$b = 400.00$

$h = 750.00$

From EC8 A.4.4.3(6), $\rho_f = 2t_f/bw = 0.005$

$bw = 400.00$

effective stress from (A.35), $f_{fe} = 765.1636$

 $R = 40.00$

Effective FRP thickness, $t_f = N L * t * \text{Cos}(b1) = 1.00$

$f_{u,f} = 840.00$

$E_f = 82000.00$

$u_{,f} = 0.015$

α_{se} ((5.4d), TBDY) = $(\alpha_{se1} * A_{ext} + \alpha_{se2} * A_{int}) / A_{sec} = 0.12601038$

$\alpha_{se1} = 0.12601038$

$b_{o,1} = 690.00$

$h_{o,1} = 340.00$

$b_{i2,1} = 1.1834E+006$

$\alpha_{se2} = \text{Max}(\alpha_{se1}, \alpha_{se2}) = 0.12601038$

bo_2 = 542.00

ho_2 = 192.00

bi2_2 = 661256.00

psh,min*Fywe = Min(psh,x*Fywe , psh,y*Fywe) = 1.64062

Expression ((5.4d), TBDY) for psh,min*Fywe has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh_x*Fywe = psh1*Fywe1+ps2*Fywe2 = 1.64062

ps1 (external) = (Ash1*h1/s1)/Asec = 0.0020944

Ash1 = Astir_1*ns_1 = 157.0796

No stirrups, ns_1 = 2.00

h1 = 400.00

ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00026808

Ash2 = Astir_2*ns_2 = 100.531

No stirrups, ns_2 = 2.00

h2 = 200.00

psh_y*Fywe = psh1*Fywe1+ps2*Fywe2 = 3.23907

ps1 (external) = (Ash1*h1/s1)/Asec = 0.00392699

Ash1 = Astir_1*ns_1 = 157.0796

No stirrups, ns_1 = 2.00

h1 = 750.00

ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00073723

Ash2 = Astir_2*ns_2 = 100.531

No stirrups, ns_2 = 2.00

h2 = 550.00

Asec = 300000.00

s1 = 100.00

s2 = 250.00

fywe1 = 694.45

fywe2 = 694.45

fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499

c = confinement factor = 1.0435

y1 = 0.00140044

sh1 = 0.0044814

ft1 = 466.8167

fy1 = 389.0139

su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

lo/lou,min = lb/d = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^2/3), from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044

sh2 = 0.0044814

ft2 = 466.8167

fy2 = 389.0139

su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^2/3), from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044
shv = 0.0044814
ftv = 466.8167
fyv = 389.0139
suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.04740567

2 = Asl,com/(b*d)*(fs2/fc) = 0.04740567

v = Asl,mid/(b*d)*(fsv/fc) = 0.01676222

and confined core properties:

b = 340.00

d = 677.00

d' = 13.00

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

1 = Asl,ten/(b*d)*(fs1/fc) = 0.05824278

2 = Asl,com/(b*d)*(fs2/fc) = 0.05824278

v = Asl,mid/(b*d)*(fsv/fc) = 0.02059413

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

v < vs,y2 - LHS eq.(4.5) is satisfied

--->

su (4.9) = 0.15682678

Mu = MRc (4.14) = 3.7261E+008

u = su (4.1) = 8.5888248E-006

Calculation of ratio lb/ld

Inadequate Lap Length with lb/ld = 0.30

Calculation of Mu1-

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

u = 8.5888248E-006

Mu = 3.7261E+008

with full section properties:

b = 400.00

d = 707.00

d' = 43.00

v = 0.00118049

N = 11016.808

fc = 33.00

co (5A.5, TBDY) = 0.002

Final value of cu: cu* = shear_factor * Max(cu, cc) = 0.01006403

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: cu = 0.01006403

we ((5.4c), TBDY) = ase* sh,min*fywe/fce+ Min(fx, fy) = 0.02692906

where f = af*pf*ffe/fce is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

fx = 0.02504965
af = 0.38744444
b = 750.00
h = 400.00
From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.00266667$
bw = 750.00
effective stress from (A.35), $ff,e = 800.0873$

fy = 0.04491794
af = 0.38744444
b = 400.00
h = 750.00
From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.005$
bw = 400.00
effective stress from (A.35), $ff,e = 765.1636$

R = 40.00
Effective FRP thickness, $tf = NL*t*\text{Cos}(b1) = 1.00$
fu,f = 840.00
Ef = 82000.00
u,f = 0.015
ase ((5.4d), TBDY) = $(ase1*Aext+ase2*Aint)/Asec = 0.12601038$
ase1 = 0.12601038
bo_1 = 690.00
ho_1 = 340.00
bi2_1 = 1.1834E+006
ase2 = $\text{Max}(ase1,ase2) = 0.12601038$
bo_2 = 542.00
ho_2 = 192.00
bi2_2 = 661256.00

psh,min*Fywe = $\text{Min}(psh,x*Fywe, psh,y*Fywe) = 1.64062$
Expression ((5.4d), TBDY) for psh,min*Fywe has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh_x*Fywe = $psh1*Fywe1+ps2*Fywe2 = 1.64062$
ps1 (external) = $(Ash1*h1/s1)/Asec = 0.0020944$
Ash1 = $Astir_1*ns_1 = 157.0796$
No stirups, ns_1 = 2.00
h1 = 400.00
ps2 (internal) = $(Ash2*h2/s2)/Asec = 0.00026808$
Ash2 = $Astir_2*ns_2 = 100.531$
No stirups, ns_2 = 2.00
h2 = 200.00

psh_y*Fywe = $psh1*Fywe1+ps2*Fywe2 = 3.23907$
ps1 (external) = $(Ash1*h1/s1)/Asec = 0.00392699$
Ash1 = $Astir_1*ns_1 = 157.0796$
No stirups, ns_1 = 2.00
h1 = 750.00
ps2 (internal) = $(Ash2*h2/s2)/Asec = 0.00073723$
Ash2 = $Astir_2*ns_2 = 100.531$
No stirups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139

su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044

sh2 = 0.0044814

ft2 = 466.8167

fy2 = 389.0139

su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044

shv = 0.0044814

ftv = 466.8167

fyv = 389.0139

suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.04740567

2 = Asl,com/(b*d)*(fs2/fc) = 0.04740567

v = Asl,mid/(b*d)*(fsv/fc) = 0.01676222

and confined core properties:

b = 340.00

d = 677.00

d' = 13.00

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

1 = Asl,ten/(b*d)*(fs1/fc) = 0.05824278

2 = Asl,com/(b*d)*(fs2/fc) = 0.05824278

v = Asl,mid/(b*d)*(fsv/fc) = 0.02059413

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

v < vs,y2 - LHS eq.(4.5) is satisfied

--->

su (4.9) = 0.15682678

Mu = MRc (4.14) = 3.7261E+008

u = su (4.1) = 8.5888248E-006

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of μ_{2+}

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:

$\mu = 8.5888248E-006$

$\mu_{2+} = 3.7261E+008$

with full section properties:

$b = 400.00$

$d = 707.00$

$d' = 43.00$

$v = 0.00118049$

$N = 11016.808$

$f_c = 33.00$

ω (5A.5, TBDY) = 0.002

Final value of μ_c : $\mu_c^* = \text{shear_factor} * \text{Max}(\mu_c, \mu_{cc}) = 0.01006403$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $\mu_c = 0.01006403$

μ_{we} ((5.4c), TBDY) = $\text{ase} * \text{sh}_{\min} * f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$

where $f = \text{af} * \text{pf} * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.02504965$

$\text{af} = 0.38744444$

$b = 750.00$

$h = 400.00$

From EC8 A.4.4.3(6), $\text{pf} = 2t_f/b_w = 0.00266667$

$b_w = 750.00$

effective stress from (A.35), $f_{fe} = 800.0873$

$f_y = 0.04491794$

$\text{af} = 0.38744444$

$b = 400.00$

$h = 750.00$

From EC8 A.4.4.3(6), $\text{pf} = 2t_f/b_w = 0.005$

$b_w = 400.00$

effective stress from (A.35), $f_{fe} = 765.1636$

$R = 40.00$

Effective FRP thickness, $t_f = N L * t * \text{Cos}(b_1) = 1.00$

$f_{u,f} = 840.00$

$E_f = 82000.00$

$u_{f} = 0.015$

ase ((5.4d), TBDY) = $(\text{ase}_1 * A_{\text{ext}} + \text{ase}_2 * A_{\text{int}}) / A_{\text{sec}} = 0.12601038$

$\text{ase}_1 = 0.12601038$

$b_{o,1} = 690.00$

$h_{o,1} = 340.00$

$b_{i,1} = 1.1834E+006$

$\text{ase}_2 = \text{Max}(\text{ase}_1, \text{ase}_2) = 0.12601038$

$b_{o,2} = 542.00$

$h_{o,2} = 192.00$

$b_{i,2} = 661256.00$

$\text{psh}_{\min} * f_{ywe} = \text{Min}(\text{psh}_x * f_{ywe}, \text{psh}_y * f_{ywe}) = 1.64062$

Expression ((5.4d), TBDY) for $\text{psh}_{\min} * f_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$\text{psh}_x * f_{ywe} = \text{psh}_1 * f_{ywe1} + \text{ps}_2 * f_{ywe2} = 1.64062$

ps_1 (external) = $(A_{sh1} * h_1 / s_1) / A_{\text{sec}} = 0.0020944$

$A_{sh1} = A_{\text{stir}_1} * n_{s,1} = 157.0796$

No stirrups, $n_{s,1} = 2.00$

h1 = 400.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00026808
Ash2 = Astir_2*ns_2 = 100.531
No stirups, ns_2 = 2.00
h2 = 200.00

psh_y*Fywe = psh1*Fywe1+ps2*Fywe2 = 3.23907
ps1 (external) = (Ash1*h1/s1)/Asec = 0.00392699
Ash1 = Astir_1*ns_1 = 157.0796
No stirups, ns_1 = 2.00
h1 = 750.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00073723
Ash2 = Astir_2*ns_2 = 100.531
No stirups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/d = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^ 2/3), from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044
sh2 = 0.0044814
ft2 = 466.8167
fy2 = 389.0139
su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^ 2/3), from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044
shv = 0.0044814
ftv = 466.8167
fyv = 389.0139
suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/d = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value $f_{sv} = f_{sv}/1.2$, from table 5.1, TBDY
For calculation of $e_{suv_nominal}$ and y_v , sh_v, ft_v, fy_v , it is considered characteristic value $f_{sv} = f_{sv}/1.2$, from table 5.1, TBDY.

y_1 , sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $f_{sv} = (f_{s,jacket} \cdot A_{s,mid,jacket} + f_{s,mid} \cdot A_{s,mid,core}) / A_{s,mid} = 389.0139$

with $E_{sv} = (E_{s,jacket} \cdot A_{s,mid,jacket} + E_{s,mid} \cdot A_{s,mid,core}) / A_{s,mid} = 200000.00$

$1 = A_{s,ten} / (b \cdot d) \cdot (f_{s1} / f_c) = 0.04740567$

$2 = A_{s,com} / (b \cdot d) \cdot (f_{s2} / f_c) = 0.04740567$

$v = A_{s,mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.01676222$

and confined core properties:

$b = 340.00$

$d = 677.00$

$d' = 13.00$

f_{cc} (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

$1 = A_{s,ten} / (b \cdot d) \cdot (f_{s1} / f_c) = 0.05824278$

$2 = A_{s,com} / (b \cdot d) \cdot (f_{s2} / f_c) = 0.05824278$

$v = A_{s,mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.02059413$

Case/Assumption: Unconfined full section - Steel rupture
' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

su (4.9) = 0.15682678

$Mu = MRc$ (4.14) = 3.7261E+008

$u = su$ (4.1) = 8.5888248E-006

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of Mu_2 -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 8.5888248E-006$

$Mu = 3.7261E+008$

with full section properties:

$b = 400.00$

$d = 707.00$

$d' = 43.00$

$v = 0.00118049$

$N = 11016.808$

$f_c = 33.00$

co (5A.5, TBDY) = 0.002

Final value of cu : $cu^* = \text{shear_factor} \cdot \text{Max}(cu, cc) = 0.01006403$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $cu = 0.01006403$

w_e ((5.4c), TBDY) = $ase \cdot sh_{,min} \cdot fy_{we} / f_{ce} + \text{Min}(fx, fy) = 0.02692906$

where $f = af \cdot pf \cdot f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

 $fx = 0.02504965$

$af = 0.38744444$

$b = 750.00$

$h = 400.00$

From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.00266667$

$bw = 750.00$

effective stress from (A.35), $ff_{,e} = 800.0873$

$fy = 0.04491794$

$af = 0.38744444$

b = 400.00
h = 750.00
From EC8 A 4.4.3(6), $pf = 2tf/bw = 0.005$
bw = 400.00
effective stress from (A.35), $ff,e = 765.1636$

R = 40.00
Effective FRP thickness, $tf = NL*t*\text{Cos}(b1) = 1.00$
 $f_{u,f} = 840.00$
 $E_f = 82000.00$
 $u_{,f} = 0.015$
 $ase((5.4d), \text{TBDY}) = (ase1*A_{ext} + ase2*A_{int})/A_{sec} = 0.12601038$
ase1 = 0.12601038
bo_1 = 690.00
ho_1 = 340.00
bi2_1 = 1.1834E+006
ase2 = $\text{Max}(ase1, ase2) = 0.12601038$
bo_2 = 542.00
ho_2 = 192.00
bi2_2 = 661256.00
 $psh_{,min}*F_{ywe} = \text{Min}(psh_{,x}*F_{ywe}, psh_{,y}*F_{ywe}) = 1.64062$

Expression ((5.4d), TBDY) for $psh_{,min}*F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh_{,x}*F_{ywe} = psh1*F_{ywe1} + ps2*F_{ywe2} = 1.64062$
ps1 (external) = $(Ash1*h1/s1)/A_{sec} = 0.0020944$
Ash1 = $A_{stir_1}*ns_1 = 157.0796$
No stirrups, $ns_1 = 2.00$
h1 = 400.00
ps2 (internal) = $(Ash2*h2/s2)/A_{sec} = 0.00026808$
Ash2 = $A_{stir_2}*ns_2 = 100.531$
No stirrups, $ns_2 = 2.00$
h2 = 200.00

$psh_{,y}*F_{ywe} = psh1*F_{ywe1} + ps2*F_{ywe2} = 3.23907$
ps1 (external) = $(Ash1*h1/s1)/A_{sec} = 0.00392699$
Ash1 = $A_{stir_1}*ns_1 = 157.0796$
No stirrups, $ns_1 = 2.00$
h1 = 750.00
ps2 (internal) = $(Ash2*h2/s2)/A_{sec} = 0.00073723$
Ash2 = $A_{stir_2}*ns_2 = 100.531$
No stirrups, $ns_2 = 2.00$
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: $cc = 0.00243499$
c = confinement factor = 1.0435

$y1 = 0.00140044$
 $sh1 = 0.0044814$
 $ft1 = 466.8167$
 $fy1 = 389.0139$
 $su1 = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$lo/lou_{,min} = lb/d = 0.30$

$su1 = 0.4*esu1_{nominal}((5.5), \text{TBDY}) = 0.032$

From table 5A.1, TBDY: $esu1_{nominal} = 0.08$,

For calculation of $esu1_{nominal}$ and $y1, sh1, ft1, fy1$, it is considered
characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25*(lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = (fs_{jacket}*A_{sl,ten,jacket} + fs_{core}*A_{sl,ten,core})/A_{sl,ten} = 389.0139$

with $E_{s1} = (E_{s,jacket} \cdot A_{s,ten,jacket} + E_{s,core} \cdot A_{s,ten,core}) / A_{s,ten} = 200000.00$
 $y_2 = 0.00140044$
 $sh_2 = 0.0044814$
 $ft_2 = 466.8167$
 $fy_2 = 389.0139$
 $su_2 = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_0/l_{ou,min} = l_b/l_{b,min} = 0.30$
 $su_2 = 0.4 \cdot esu_{2,nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu_{2,nominal} = 0.08$,
 For calculation of $esu_{2,nominal}$ and y_2, sh_2, ft_2, fy_2 , it is considered
 characteristic value $fs_2 = fs_2/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs_2 = (fs_{jacket} \cdot A_{s,com,jacket} + fs_{core} \cdot A_{s,com,core}) / A_{s,com} = 389.0139$
 with $E_{s2} = (E_{s,jacket} \cdot A_{s,com,jacket} + E_{s,core} \cdot A_{s,com,core}) / A_{s,com} = 200000.00$
 $y_v = 0.00140044$
 $sh_v = 0.0044814$
 $ft_v = 466.8167$
 $fy_v = 389.0139$
 $suv = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_0/l_{ou,min} = l_b/l_d = 0.30$
 $suv = 0.4 \cdot esuv_{nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_{nominal} = 0.08$,
 considering characteristic value $fs_v = fs_v/1.2$, from table 5.1, TBDY
 For calculation of $esuv_{nominal}$ and y_v, sh_v, ft_v, fy_v , it is considered
 characteristic value $fs_v = fs_v/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs_v = (fs_{jacket} \cdot A_{s,mid,jacket} + fs_{mid} \cdot A_{s,mid,core}) / A_{s,mid} = 389.0139$
 with $E_{sv} = (E_{s,jacket} \cdot A_{s,mid,jacket} + E_{s,mid} \cdot A_{s,mid,core}) / A_{s,mid} = 200000.00$
 $1 = A_{s,ten} / (b \cdot d) \cdot (fs_1 / f_c) = 0.04740567$
 $2 = A_{s,com} / (b \cdot d) \cdot (fs_2 / f_c) = 0.04740567$
 $v = A_{s,mid} / (b \cdot d) \cdot (fs_v / f_c) = 0.01676222$

and confined core properties:

$b = 340.00$
 $d = 677.00$
 $d' = 13.00$
 $f_{cc} (5A.2, TBDY) = 34.43547$
 $cc (5A.5, TBDY) = 0.00243499$
 $c = \text{confinement factor} = 1.0435$
 $1 = A_{s,ten} / (b \cdot d) \cdot (fs_1 / f_c) = 0.05824278$
 $2 = A_{s,com} / (b \cdot d) \cdot (fs_2 / f_c) = 0.05824278$
 $v = A_{s,mid} / (b \cdot d) \cdot (fs_v / f_c) = 0.02059413$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

---->

$v < v_s, y_2$ - LHS eq.(4.5) is satisfied

---->

$su (4.9) = 0.15682678$
 $M_u = MR_c (4.14) = 3.7261E+008$
 $u = su (4.1) = 8.5888248E-006$

 Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

 Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 1.2627E+006$

 Calculation of Shear Strength at edge 1, $V_{r1} = 1.2627E+006$

Vr1 = VCol ((10.3), ASCE 41-17) = knl*VCol0

VCol0 = 1.2627E+006

kn1 = 1 (zero step-static loading)

NOTE: In expression (10-3) 'Vs = Av*fy*d/s' is replaced by 'Vs+ f*Vf'
where Vf is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: fc' = (fc'_jacket*Area_jacket + fc'_core*Area_core)/Area_section = 33.00, but fc'^0.5 <= 8.3 MPa (22.5.3.1, ACI 318-14)

M/Vd = 2.00

Mu = 2.3795529E-012

Vu = 2.9582284E-031

d = 0.8*h = 600.00

Nu = 11016.808

Ag = 300000.00

From (11.5.4.8), ACI 318-14: Vs = Vs1 + Vs2 = 621900.694

where:

Vs1 = 523602.964 is calculated for jacket, with:

d = 600.00

Av = 157079.633

fy = 555.56

s = 100.00

Vs1 is multiplied by Col1 = 1.00

s/d = 0.16666667

Vs2 = 98297.73 is calculated for core, with:

d = 440.00

Av = 100530.965

fy = 555.56

s = 250.00

Vs2 is multiplied by Col2 = 1.00

s/d = 0.56818182

Vf ((11-3)-(11.4), ACI 440) = 463792.00

f = 0.95, for fully-wrapped sections

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) sin + cos is replaced with (cot + cota)sina which is more a generalised expression,
where is the angle of the crack direction (see KANEPE).

This later relation, considered as a function Vf(,), is implemented for every different fiber orientation ai,
as well as for 2 crack directions, = 45° and = -45° to take into consideration the cyclic seismic loading.

orientation 1: 1 = b1 + 90° = 90.00

Vf = Min(|Vf(45, 1)|, |Vf(-45, a1)|), with:

total thickness per orientation, tf1 = NL*t/NoDir = 1.00

dfv = d (figure 11.2, ACI 440) = 707.00

ffe ((11-5), ACI 440) = 328.00

Ef = 82000.00

fe = 0.004, from (11.6a), ACI 440

with fu = 0.009

From (11-11), ACI 440: Vs + Vf <= 915872.391

bw = 400.00

Calculation of Shear Strength at edge 2, Vr2 = 1.2627E+006

Vr2 = VCol ((10.3), ASCE 41-17) = knl*VCol0

VCol0 = 1.2627E+006

kn1 = 1 (zero step-static loading)

NOTE: In expression (10-3) 'Vs = Av*fy*d/s' is replaced by 'Vs+ f*Vf'
where Vf is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: fc' = (fc'_jacket*Area_jacket + fc'_core*Area_core)/Area_section = 33.00, but fc'^0.5 <= 8.3 MPa (22.5.3.1, ACI 318-14)

M/Vd = 2.00

Mu = 2.3795529E-012

Vu = 2.9582284E-031

d = 0.8*h = 600.00

Nu = 11016.808

Ag = 300000.00
From (11.5.4.8), ACI 318-14: Vs = Vs1 + Vs2 = 621900.694

where:

Vs1 = 523602.964 is calculated for jacket, with:

d = 600.00

Av = 157079.633

fy = 555.56

s = 100.00

Vs1 is multiplied by Col1 = 1.00

s/d = 0.16666667

Vs2 = 98297.73 is calculated for core, with:

d = 440.00

Av = 100530.965

fy = 555.56

s = 250.00

Vs2 is multiplied by Col2 = 1.00

s/d = 0.56818182

Vf ((11-3)-(11.4), ACI 440) = 463792.00

f = 0.95, for fully-wrapped sections

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) $\sin \theta + \cos \theta$ is replaced with $(\cot \theta + \cot \alpha) \sin \alpha$ which is more a generalised expression, where θ is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, \alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, \alpha)|)$, with:

total thickness per orientation, $tf1 = NL * t / \text{NoDir} = 1.00$

$dfv = d$ (figure 11.2, ACI 440) = 707.00

ffe ((11-5), ACI 440) = 328.00

$E_f = 82000.00$

$fe = 0.004$, from (11.6a), ACI 440

with $fu = 0.009$

From (11-11), ACI 440: $V_s + V_f \leq 915872.391$

$bw = 400.00$

End Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At local axis: 2

Start Of Calculation of Chord Rotation Capacity for element: column JC1 of floor 1

At local axis: 2

Integration Section: (a)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$

Chord Rotation is generally considered as Deformation-Controlled Action according to Table C7-1, ASCE 41-17.

Mean strengths are used for Deformation-Controlled Actions according to 7.5.1.3, ASCE 41-17

Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Existing Column

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

External Height, $H = 400.00$

External Width, $W = 750.00$

Internal Height, $H = 200.00$

Internal Width, $W = 550.00$

Cover Thickness, $c = 25.00$
Element Length, $L = 3000.00$
Primary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Inadequate Lap Length with $l_b/l_d = 0.30$
FRP Wrapping Data
Type: Carbon
Dry properties (design values)
Thickness, $t = 1.00$
Tensile Strength, $f_{fu} = 840.00$
Tensile Modulus, $E_f = 82000.00$
Elongation, $e_{fu} = 0.009$
Number of directions, $N_{oDir} = 1$
Fiber orientations, $b_i = 0.00^\circ$
Number of layers, $N_L = 1$
Radius of rounding corners, $R = 40.00$

Stepwise Properties

Bending Moment, $M = 7.4177612E-010$
Shear Force, $V_2 = -3112.69$
Shear Force, $V_3 = -2.2719624E-013$
Axial Force, $F = -15947.314$
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: $A_{st} = 0.00$
-Compression: $A_{sc} = 2676.637$
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: $A_{st,ten} = 1137.257$
-Compression: $A_{sc,com} = 1137.257$
-Middle: $A_{st,mid} = 402.1239$
Longitudinal External Reinforcement Area Distribution (in 3 divisions)
-Tension: $A_{st,ten,jacket} = 829.3805$
-Compression: $A_{sc,com,jacket} = 829.3805$
-Middle: $A_{st,mid,jacket} = 402.1239$
Longitudinal Internal Reinforcement Area Distribution (in 3 divisions)
-Tension: $A_{st,ten,core} = 307.8761$
-Compression: $A_{sc,com,core} = 307.8761$
-Middle: $A_{st,mid,core} = 0.00$
Mean Diameter of Tension Reinforcement, $Db_L = 16.80$

New component: From table 7-7, ASCE 41_17: Final chord rotation Capacity $u_{,R} = 1.0^*$ $u = 0.0274056$
 $u = y + p = 0.0274056$

- Calculation of y -

$y = (M_y * L_s / 3) / E_{eff} = 0.00203441$ ((4.29), Biskinis Phd)
 $M_y = 1.3183E+008$
 $L_s = M/V$ (with $L_s > 0.1 * L$ and $L_s < 2 * L$) = 1500.00
From table 10.5, ASCE 41_17: $E_{eff} = factor * E_c * I_g = 3.2399E+013$
factor = 0.30
 $A_g = 300000.00$
Mean concrete strength: $f'_c = (f'_{c,jacket} * Area_{jacket} + f'_{c,core} * Area_{core}) / Area_{section} = 33.00$
 $N = 15947.314$
 $E_c * I_g = E_{c,jacket} * I_{g,jacket} + E_{c,core} * I_{g,core} = 1.0800E+014$

Calculation of Yielding Moment M_y

Calculation of γ and M_y according to Annex 7 -

$$y = \text{Min}(y_{\text{ten}}, y_{\text{com}})$$

$$y_{\text{ten}} = 5.6388532\text{E-}006$$

$$\text{with } ((10.1), \text{ASCE 41-17}) f_y = \text{Min}(f_y, 1.25 \cdot f_y \cdot (l_b/d)^{2/3}) = 311.2112$$

$$d = 357.00$$

$$y = 0.22702375$$

$$A = 0.01018816$$

$$B = 0.00579182$$

$$\text{with } p_t = 0.00424746$$

$$p_c = 0.00424746$$

$$p_v = 0.00150186$$

$$N = 15947.314$$

$$b = 750.00$$

$$r = 0.12044818$$

$$y_{\text{comp}} = 2.7794074\text{E-}005$$

$$\text{with } f_c \cdot (12.3, \text{ACI 440}) = 33.35499$$

$$f_c = 33.00$$

$$f_l = 0.77176471$$

$$b = 750.00$$

$$h = 400.00$$

$$A_g = 300000.00$$

$$\text{From } (12.9), \text{ACI 440: } k_a = 0.14671972$$

$$g = p_t + p_c + p_v = 0.00999678$$

$$r_c = 40.00$$

$$A_e/A_c = 0.51581153$$

$$\text{Effective FRP thickness, } t_f = N \cdot l \cdot \cos(b_1) = 1.00$$

$$\text{effective strain from } (12.5) \text{ and } (12.12), e_{fe} = 0.004$$

$$f_u = 0.009$$

$$E_f = 82000.00$$

$$E_c = 26999.444$$

$$y = 0.22410833$$

$$A = 0.00986286$$

$$B = 0.00560044$$

$$\text{with } E_s = 200000.00$$

Calculation of ratio l_b/d

Inadequate Lap Length with $l_b/d = 0.30$

- Calculation of p -

From table 10-8: $p = 0.02537119$

with:

- Columns controlled by inadequate development or splicing along the clear height because $l_b/d < 1$

$$\text{shear control ratio } V_y E / V_c O E = 0.14265244$$

$$d = d_{\text{external}} = 357.00$$

$$s = s_{\text{external}} = 0.00$$

$$t = s_1 + s_2 + 2 \cdot t_f / b_w \cdot (f_{fe} / f_s) = 0.00393687$$

$$\text{jacket: } s_1 = A_{v1} \cdot h_1 / (s_1 \cdot A_g) = 0.0020944$$

$A_{v1} = 157.0796$, is the total area of all stirrups parallel to loading (shear) direction

$$h_1 = 400.00$$

$$s_1 = 100.00$$

$$\text{core: } s_2 = A_{v2} \cdot h_2 / (s_2 \cdot A_g) = 0.00026808$$

$A_{v2} = 100.531$, is the total area of all stirrups parallel to loading (shear) direction

$$h_2 = 200.00$$

$$s_2 = 250.00$$

The term $2 \cdot t_f / b_w \cdot (f_{fe} / f_s)$ is implemented to account for FRP contribution

where $f = 2 \cdot t_f / b_w$ is FRP ratio (EC8 - 3, A.4.4.3(6)) and f_{fe} / f_s normalises f to steel strength

All these variables have already been given in Shear control ratio calculation.

For the normalisation f_s of jacket is used.

$$N_{UD} = 15947.314$$

$$A_g = 300000.00$$

$$f_c E = (f_{c_jacket} \cdot \text{Area}_{\text{jacket}} + f_{c_core} \cdot \text{Area}_{\text{core}}) / \text{section_area} = 33.00$$

$$f_{yE} = (f_{y_ext_Long_Reinf} * Area_ext_Long_Reinf + f_{y_int_Long_Reinf} * Area_int_Long_Reinf) / Area_Tot_Long_Rein = 555.56$$

$$f_{tE} = (f_{y_ext_Trans_Reinf} * s_1 + f_{y_int_Trans_Reinf} * s_2) / (s_1 + s_2) = 555.56$$

$$\rho_l = Area_Tot_Long_Rein / (b * d) = 0.00999678$$

$$b = 750.00$$

$$d = 357.00$$

$$f_{cE} = 33.00$$

End Of Calculation of Chord Rotation Capacity for element: column JC1 of floor 1

At local axis: 2

Integration Section: (a)

Calculation No. 11

column C1, Floor 1

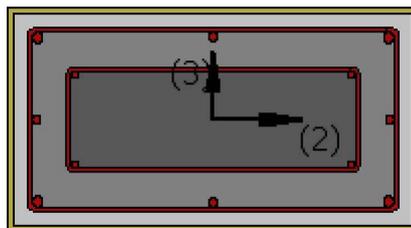
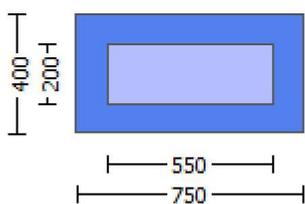
Limit State: Life Safety (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Shear capacity VRd

Edge: Start

Local Axis: (3)



Start Of Calculation of Shear Capacity for element: column JC1 of floor 1

At local axis: 3

Integration Section: (a)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$

Member Shear Force is generally considered as Force-Controlled Action according to Table C7-1, ASCE 41-17.

Lower-bound strengths are used for Force-Controlled Actions according to 7.5.1.3, ASCE 41-17

Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Primary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$
 Concrete Elasticity, $E_c = 26999.444$
 Steel Elasticity, $E_s = 200000.00$
 Existing Column
 New material of Primary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$
 New material of Primary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$
 Concrete Elasticity, $E_c = 26999.444$
 Steel Elasticity, $E_s = 200000.00$
 #####
 Note: Especially for the calculation of γ for displacement ductility demand,
 the expected (mean value) strengths are used (7.5.1.3, ASCE 41-17) because bending is considered as
 Deformation-Controlled Action (Table C7-1, ASCE 41-17).
 Jacket
 New material: Concrete Strength, $f_c = f_{cm} = 33.00$
 New material: Steel Strength, $f_s = f_{sm} = 555.56$
 Existing Column
 New material: Concrete Strength, $f_c = f_{cm} = 33.00$
 New material: Steel Strength, $f_s = f_{sm} = 555.56$
 #####
 External Height, $H = 400.00$
 External Width, $W = 750.00$
 Internal Height, $H = 200.00$
 Internal Width, $W = 550.00$
 Cover Thickness, $c = 25.00$
 Element Length, $L = 3000.00$
 Primary Member
 Ribbed Bars
 Ductile Steel
 Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
 Longitudinal Bars With Ends Lapped Starting at the End Sections
 Inadequate Lap Length with $l_o/l_{o,min} = l_b/l_d = 0.30$
 FRP Wrapping Data
 Type: Carbon
 Dry properties (design values)
 Thickness, $t = 1.00$
 Tensile Strength, $f_{fu} = 840.00$
 Tensile Modulus, $E_f = 82000.00$
 Elongation, $e_{fu} = 0.009$
 Number of directions, $NoDir = 1$
 Fiber orientations, $bi: 0.00^\circ$
 Number of layers, $NL = 1$
 Radius of rounding corners, $R = 40.00$

 Stepwise Properties

EDGE -A-
 Bending Moment, $M_a = 7.4177612E-010$
 Shear Force, $V_a = -2.2719624E-013$
 EDGE -B-
 Bending Moment, $M_b = -5.8822499E-011$
 Shear Force, $V_b = 2.2719624E-013$
 BOTH EDGES
 Axial Force, $F = -15947.314$
 Longitudinal Reinforcement Area Distribution (in 2 divisions)
 -Tension: $As_t = 0.00$
 -Compression: $As_c = 2676.637$
 Longitudinal Reinforcement Area Distribution (in 3 divisions)
 -Tension: $As_{l,ten} = 1137.257$
 -Compression: $As_{l,com} = 1137.257$
 -Middle: $As_{l,mid} = 402.1239$
 Mean Diameter of Tension Reinforcement, $Db_{L,ten} = 16.80$

 New component: From table 7-7, ASCE 41_17: Final Shear Capacity $VR = 1.0 * V_n = 776982.499$

Vn ((10.3), ASCE 41-17) = knl*VColO = 776982.499

VCol = 776982.499

knl = 1.00

displacement_ductility_demand = 0.00

NOTE: In expression (10-3) 'Vs = Av*fy*d/s' is replaced by 'Vs+ f*Vf' where Vf is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: fc' = (fc'_jacket*Area_jacket + fc'_core*Area_core)/Area_section = 25.00, but fc'^0.5 <= 8.3 MPa (22.5.3.1, ACI 318-14)

M/Vd = 2.00

Mu = 7.4177612E-010

Vu = 2.2719624E-013

d = 0.8*h = 320.00

Nu = 15947.314

Ag = 300000.00

From (11.5.4.8), ACI 318-14: Vs = Vs1 + Vs2 = 251327.412

where:

Vs1 = 251327.412 is calculated for jacket, with:

d = 320.00

Av = 157079.633

fy = 500.00

s = 100.00

Vs1 is multiplied by Col1 = 1.00

s/d = 0.3125

Vs2 = 0.00 is calculated for core, with:

d = 160.00

Av = 100530.965

fy = 500.00

s = 250.00

Vs2 is multiplied by Col2 = 0.00

s/d = 1.5625

Vf ((11-3)-(11.4), ACI 440) = 234192.00

f = 0.95, for fully-wrapped sections

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) sin + cos is replaced with (cot + cota)sina which is more a generalised expression, where is the angle of the crack direction (see KANEPE).

This later relation, considered as a function Vf(,), is implemented for every different fiber orientation ai, as well as for 2 crack directions, = 45° and = -45° to take into consideration the cyclic seismic loading.

orientation 1: 1 = b1 + 90° = 90.00

Vf = Min(|Vf(45, 1)|, |Vf(-45, a1)|), with:

total thickness per orientation, tf1 = NL*t/NoDir = 1.00

dfv = d (figure 11.2, ACI 440) = 357.00

ffe ((11-5), ACI 440) = 328.00

Ef = 82000.00

fe = 0.004, from (11.6a), ACI 440

with fu = 0.009

From (11-11), ACI 440: Vs + Vf <= 797164.595

bw = 750.00

displacement_ductility_demand is calculated as / y

- Calculation of / y for END A -

for rotation axis 2 and integ. section (a)

From analysis, chord rotation = 1.0055897E-020

y = (My*Ls/3)/Eleff = 0.00203441 ((4.29), Biskinis Phd)

My = 1.3183E+008

Ls = M/V (with Ls > 0.1*L and Ls < 2*L) = 1500.00

From table 10.5, ASCE 41_17: Eleff = factor*Ec*lg = 3.2399E+013

factor = 0.30

Ag = 300000.00

Mean concrete strength: fc' = (fc'_jacket*Area_jacket + fc'_core*Area_core)/Area_section = 33.00

N = 15947.314

$$E_c \cdot I_g = E_{c_jacket} \cdot I_{g_jacket} + E_{c_core} \cdot I_{g_core} = 1.0800E+014$$

Calculation of Yielding Moment M_y

Calculation of y and M_y according to Annex 7 -

$$y = \text{Min}(y_{ten}, y_{com})$$

$$y_{ten} = 5.6388532E-006$$

$$\text{with } ((10.1), \text{ASCE 41-17}) f_y = \text{Min}(f_y, 1.25 \cdot f_y \cdot (l_b/d)^{2/3}) = 311.2112$$

$$d = 357.00$$

$$y = 0.22702375$$

$$A = 0.01018816$$

$$B = 0.00579182$$

$$\text{with } p_t = 0.00424746$$

$$p_c = 0.00424746$$

$$p_v = 0.00150186$$

$$N = 15947.314$$

$$b = 750.00$$

$$" = 0.12044818$$

$$y_{comp} = 2.7794074E-005$$

$$\text{with } f_c^* (12.3, \text{ACI 440}) = 33.35499$$

$$f_c = 33.00$$

$$f_l = 0.77176471$$

$$b = 750.00$$

$$h = 400.00$$

$$A_g = 300000.00$$

$$\text{From } (12.9), \text{ACI 440: } k_a = 0.14671972$$

$$g = p_t + p_c + p_v = 0.00999678$$

$$r_c = 40.00$$

$$A_e/A_c = 0.51581153$$

$$\text{Effective FRP thickness, } t_f = N_L \cdot t \cdot \text{Cos}(b_1) = 1.00$$

$$\text{effective strain from } (12.5) \text{ and } (12.12), e_{fe} = 0.004$$

$$f_u = 0.009$$

$$E_f = 82000.00$$

$$E_c = 26999.444$$

$$y = 0.22410833$$

$$A = 0.00986286$$

$$B = 0.00560044$$

$$\text{with } E_s = 200000.00$$

Calculation of ratio l_b/d

Inadequate Lap Length with $l_b/d = 0.30$

End Of Calculation of Shear Capacity for element: column JC1 of floor 1

At local axis: 3

Integration Section: (a)

Calculation No. 12

column C1, Floor 1

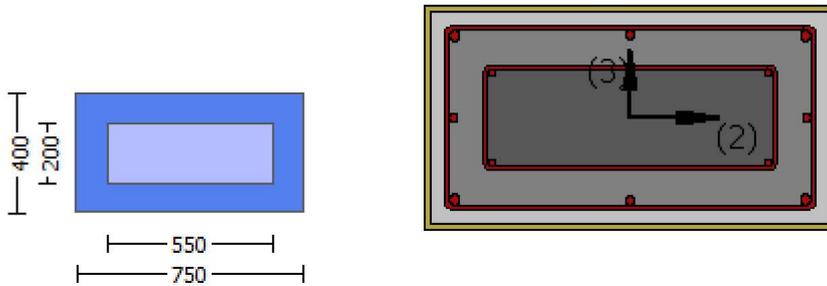
Limit State: Life Safety (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Chord rotation capacity (ϕ)

Edge: Start

Local Axis: (3)



Start Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At Shear local axis: 3

(Bending local axis: 2)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$

Mean strength values are used for both shear and moment calculations.

Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Existing Column

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of moment strengths,

the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

Jacket

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

Existing Column

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

#####

External Height, $H = 400.00$

External Width, $W = 750.00$

Internal Height, $H = 200.00$

Internal Width, $W = 550.00$

Cover Thickness, $c = 25.00$

Mean Confinement Factor overall section = 1.0435

Element Length, $L = 3000.00$

Primary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Inadequate Lap Length with $l_o/l_{ou,min} = 0.30$

FRP Wrapping Data

Type: Carbon

Dry properties (design values)

Thickness, $t = 1.00$

Tensile Strength, $f_{fu} = 840.00$

Tensile Modulus, $E_f = 82000.00$

Elongation, $e_{fu} = 0.009$

Number of directions, $NoDir = 1$

Fiber orientations, $bi = 0.00^\circ$

Number of layers, $NL = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 3

EDGE -A-

Shear Force, $V_a = 3.4586346E-031$

EDGE -B-

Shear Force, $V_b = -3.4586346E-031$

BOTH EDGES

Axial Force, $F = -11016.808$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $As_c = 2676.637$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{t,ten} = 1137.257$

-Compression: $As_{c,com} = 1137.257$

-Middle: $As_{mid} = 402.1239$

Calculation of Shear Capacity ratio , $V_e/V_r = 0.14265244$

Member Controlled by Flexure ($V_e/V_r < 1$)

Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 121055.918$

with

$M_{pr1} = \text{Max}(Mu_{1+} , Mu_{1-}) = 1.8158E+008$

$Mu_{1+} = 1.8158E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination

$Mu_{1-} = 1.8158E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination

$M_{pr2} = \text{Max}(Mu_{2+} , Mu_{2-}) = 1.8158E+008$

$Mu_{2+} = 1.8158E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the the static loading combination

$Mu_{2-} = 1.8158E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the the static loading combination

Calculation of Mu_{1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$\phi_u = 1.7255512E-005$

$M_u = 1.8158E+008$

with full section properties:

$b = 750.00$

$d = 357.00$

$d' = 43.00$

$v = 0.00124684$

$N = 11016.808$

$f_c = 33.00$

$\alpha (\text{5A.5, TBDY}) = 0.002$

Final value of c_u : $c_u^* = \text{shear_factor} * \text{Max}(c_u, c_c) = 0.01006403$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $c_u = 0.01006403$

w_e ((5.4c), TBDY) = $ase * \text{sh_min} * fy_{we} / f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$

where $f = af * pf * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

 $f_x = 0.02504965$

$af = 0.38744444$

$b = 750.00$

$h = 400.00$

From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.00266667$

$bw = 750.00$

effective stress from (A.35), $ff_e = 800.0873$

 $f_y = 0.04491794$

$af = 0.38744444$

$b = 400.00$

$h = 750.00$

From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.005$

$bw = 400.00$

effective stress from (A.35), $ff_e = 765.1636$

 $R = 40.00$

Effective FRP thickness, $tf = NL * t * \text{Cos}(b1) = 1.00$

$f_{u,f} = 840.00$

$E_f = 82000.00$

$u_{,f} = 0.015$

ase ((5.4d), TBDY) = $(ase1 * A_{ext} + ase2 * A_{int}) / A_{sec} = 0.12601038$

$ase1 = 0.12601038$

$bo_1 = 690.00$

$ho_1 = 340.00$

$bi2_1 = 1.1834E+006$

$ase2 = \text{Max}(ase1, ase2) = 0.12601038$

$bo_2 = 542.00$

$ho_2 = 192.00$

$bi2_2 = 661256.00$

$psh_{,min} * F_{ywe} = \text{Min}(psh_{,x} * F_{ywe}, psh_{,y} * F_{ywe}) = 1.64062$

Expression ((5.4d), TBDY) for $psh_{,min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

 $psh_{,x} * F_{ywe} = psh1 * F_{ywe1} + ps2 * F_{ywe2} = 1.64062$

$ps1$ (external) = $(Ash1 * h1 / s1) / A_{sec} = 0.0020944$

$Ash1 = Astir_1 * ns_1 = 157.0796$

No stirrups, $ns_1 = 2.00$

$h1 = 400.00$

$ps2$ (internal) = $(Ash2 * h2 / s2) / A_{sec} = 0.00026808$

$Ash2 = Astir_2 * ns_2 = 100.531$

No stirrups, $ns_2 = 2.00$

$h2 = 200.00$

 $psh_{,y} * F_{ywe} = psh1 * F_{ywe1} + ps2 * F_{ywe2} = 3.23907$

$ps1$ (external) = $(Ash1 * h1 / s1) / A_{sec} = 0.00392699$

$Ash1 = Astir_1 * ns_1 = 157.0796$

No stirrups, $ns_1 = 2.00$

$h1 = 750.00$

$ps2$ (internal) = $(Ash2 * h2 / s2) / A_{sec} = 0.00073723$

$Ash2 = Astir_2 * ns_2 = 100.531$

No stirrups, $ns_2 = 2.00$

$h2 = 550.00$

 $A_{sec} = 300000.00$

$s1 = 100.00$

$s2 = 250.00$

$fy_{we1} = 694.45$

$fy_{we2} = 694.45$

$f_{ce} = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00243499$

$c = \text{confinement factor} = 1.0435$
 $y1 = 0.00140044$
 $sh1 = 0.0044814$
 $ft1 = 466.8167$
 $fy1 = 389.0139$
 $su1 = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou,min = lb/ld = 0.30$
 $su1 = 0.4 * esu1_nominal ((5.5), \text{TBDY}) = 0.032$
 From table 5A.1, TBDY: $esu1_nominal = 0.08$,
 For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs1 = (fs,jacket * Asl,ten,jacket + fs,core * Asl,ten,core) / Asl,ten = 389.0139$
 with $Es1 = (Es,jacket * Asl,ten,jacket + Es,core * Asl,ten,core) / Asl,ten = 200000.00$
 $y2 = 0.00140044$
 $sh2 = 0.0044814$
 $ft2 = 466.8167$
 $fy2 = 389.0139$
 $su2 = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou,min = lb/lb,min = 0.30$
 $su2 = 0.4 * esu2_nominal ((5.5), \text{TBDY}) = 0.032$
 From table 5A.1, TBDY: $esu2_nominal = 0.08$,
 For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs2 = (fs,jacket * Asl,com,jacket + fs,core * Asl,com,core) / Asl,com = 389.0139$
 with $Es2 = (Es,jacket * Asl,com,jacket + Es,core * Asl,com,core) / Asl,com = 200000.00$
 $yv = 0.00140044$
 $shv = 0.0044814$
 $ftv = 466.8167$
 $fyv = 389.0139$
 $suv = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou,min = lb/ld = 0.30$
 $suv = 0.4 * esuv_nominal ((5.5), \text{TBDY}) = 0.032$
 From table 5A.1, TBDY: $esuv_nominal = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = (fs,jacket * Asl,mid,jacket + fs,mid * Asl,mid,core) / Asl,mid = 389.0139$
 with $Es_v = (Es,jacket * Asl,mid,jacket + Es,mid * Asl,mid,core) / Asl,mid = 200000.00$
 $1 = Asl,ten / (b * d) * (fs1 / fc) = 0.0500703$
 $2 = Asl,com / (b * d) * (fs2 / fc) = 0.0500703$
 $v = Asl,mid / (b * d) * (fsv / fc) = 0.01770442$
 and confined core properties:
 $b = 690.00$
 $d = 327.00$
 $d' = 13.00$
 $fcc (5A.2, \text{TBDY}) = 34.43547$
 $cc (5A.5, \text{TBDY}) = 0.00243499$
 $c = \text{confinement factor} = 1.0435$
 $1 = Asl,ten / (b * d) * (fs1 / fc) = 0.05941729$
 $2 = Asl,com / (b * d) * (fs2 / fc) = 0.05941729$
 $v = Asl,mid / (b * d) * (fsv / fc) = 0.02100943$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)
 --->
 $v < vs,y2$ - LHS eq.(4.5) is satisfied

--->

$$s_u(4.9) = 0.16886055$$

$$M_u = M_{Rc}(4.14) = 1.8158E+008$$

$$u = s_u(4.1) = 1.7255512E-005$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of M_{u1} -

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$$u = 1.7255512E-005$$

$$M_u = 1.8158E+008$$

with full section properties:

$$b = 750.00$$

$$d = 357.00$$

$$d' = 43.00$$

$$v = 0.00124684$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$c_o(5A.5, TBDY) = 0.002$$

$$\text{Final value of } \phi_u: \phi_u^* = \text{shear_factor} * \text{Max}(\phi_u, \phi_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \phi_u = 0.01006403$$

$$w_e(5.4c, TBDY) = a_{se} * s_{h, \min} * f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$$

where $f = a_f * p_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.02504965$$

$$a_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.00266667$$

$$b_w = 750.00$$

$$\text{effective stress from (A.35), } f_{fe} = 800.0873$$

$$f_y = 0.04491794$$

$$a_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.005$$

$$b_w = 400.00$$

$$\text{effective stress from (A.35), } f_{fe} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL * t * \text{Cos}(b_1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{f} = 0.015$$

$$a_{se}((5.4d), TBDY) = (a_{se1} * A_{ext} + a_{se2} * A_{int}) / A_{sec} = 0.12601038$$

$$a_{se1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i2,1} = 1.1834E+006$$

$$a_{se2} = \text{Max}(a_{se1}, a_{se2}) = 0.12601038$$

$$b_{o,2} = 542.00$$

$$h_{o,2} = 192.00$$

$$b_{i2,2} = 661256.00$$

$$p_{sh, \min} * F_{ywe} = \text{Min}(p_{sh,x} * F_{ywe}, p_{sh,y} * F_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $p_{sh, \min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh_x*Fywe = psh1*Fywe1+ps2*Fywe2 = 1.64062
ps1 (external) = (Ash1*h1/s1)/Asec = 0.0020944
Ash1 = Astir_1*ns_1 = 157.0796
No stirups, ns_1 = 2.00
h1 = 400.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00026808
Ash2 = Astir_2*ns_2 = 100.531
No stirups, ns_2 = 2.00
h2 = 200.00

psh_y*Fywe = psh1*Fywe1+ps2*Fywe2 = 3.23907
ps1 (external) = (Ash1*h1/s1)/Asec = 0.00392699
Ash1 = Astir_1*ns_1 = 157.0796
No stirups, ns_1 = 2.00
h1 = 750.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00073723
Ash2 = Astir_2*ns_2 = 100.531
No stirups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00

s1 = 100.00

s2 = 250.00

fywe1 = 694.45

fywe2 = 694.45

fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499

c = confinement factor = 1.0435

y1 = 0.00140044

sh1 = 0.0044814

ft1 = 466.8167

fy1 = 389.0139

su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 * (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044

sh2 = 0.0044814

ft2 = 466.8167

fy2 = 389.0139

su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 * (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044

shv = 0.0044814

ftv = 466.8167

fyv = 389.0139

suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

lo/lo_{u,min} = lb/d = 0.30

su_v = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fs_{yv} = fsv/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and y_v, sh_v,ft_v,fy_v, it is considered characteristic value fs_{yv} = fsv/1.2, from table 5.1, TBDY.

y₁, sh₁,ft₁,fy₁, are also multiplied by Min(1,1.25*(lb/d)^{2/3}), from 10.3.5, ASCE 41-17.

with fsv = (fs_{jacket}*As_{l,mid,jacket} + fs_{mid}*As_{l,mid,core})/As_{l,mid} = 389.0139

with Es_v = (Es_{jacket}*As_{l,mid,jacket} + Es_{mid}*As_{l,mid,core})/As_{l,mid} = 200000.00

1 = As_{l,ten}/(b*d)*(fs₁/fc) = 0.0500703

2 = As_{l,com}/(b*d)*(fs₂/fc) = 0.0500703

v = As_{l,mid}/(b*d)*(fsv/fc) = 0.01770442

and confined core properties:

b = 690.00

d = 327.00

d' = 13.00

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

1 = As_{l,ten}/(b*d)*(fs₁/fc) = 0.05941729

2 = As_{l,com}/(b*d)*(fs₂/fc) = 0.05941729

v = As_{l,mid}/(b*d)*(fsv/fc) = 0.02100943

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

v < v_{s,y2} - LHS eq.(4.5) is satisfied

--->

su (4.9) = 0.16886055

Mu = MRc (4.14) = 1.8158E+008

u = su (4.1) = 1.7255512E-005

Calculation of ratio lb/d

Inadequate Lap Length with lb/d = 0.30

Calculation of Mu₂₊

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

u = 1.7255512E-005

Mu = 1.8158E+008

with full section properties:

b = 750.00

d = 357.00

d' = 43.00

v = 0.00124684

N = 11016.808

fc = 33.00

co (5A.5, TBDY) = 0.002

Final value of cu: cu* = shear_factor * Max(cu, cc) = 0.01006403

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: cu = 0.01006403

we ((5.4c), TBDY) = ase* sh_{min}*fy_{we}/f_{ce}+Min(fx, fy) = 0.02692906

where f = af*pf*ffe/f_{ce} is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

fx = 0.02504965

af = 0.38744444

b = 750.00

h = 400.00

From EC8 A.4.4.3(6), pf = 2tf/bw = 0.00266667

bw = 750.00
effective stress from (A.35), $f_{f,e} = 800.0873$

fy = 0.04491794
af = 0.38744444
b = 400.00
h = 750.00
From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.005$
bw = 400.00
effective stress from (A.35), $f_{f,e} = 765.1636$

R = 40.00
Effective FRP thickness, $tf = NL*t*Cos(b1) = 1.00$
fu,f = 840.00
Ef = 82000.00
u,f = 0.015
ase ((5.4d), TBDY) = $(ase1*Aext+ase2*Aint)/Asec = 0.12601038$
ase1 = 0.12601038
bo_1 = 690.00
ho_1 = 340.00
bi2_1 = 1.1834E+006
ase2 = $Max(ase1,ase2) = 0.12601038$
bo_2 = 542.00
ho_2 = 192.00
bi2_2 = 661256.00

psh,min*Fywe = $Min(psh,x*Fywe, psh,y*Fywe) = 1.64062$
Expression ((5.4d), TBDY) for psh,min*Fywe has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh_x*Fywe = $psh1*Fywe1+ps2*Fywe2 = 1.64062$
ps1 (external) = $(Ash1*h1/s1)/Asec = 0.0020944$
Ash1 = $Astir_1*ns_1 = 157.0796$
No stirrups, ns_1 = 2.00
h1 = 400.00
ps2 (internal) = $(Ash2*h2/s2)/Asec = 0.00026808$
Ash2 = $Astir_2*ns_2 = 100.531$
No stirrups, ns_2 = 2.00
h2 = 200.00

psh_y*Fywe = $psh1*Fywe1+ps2*Fywe2 = 3.23907$
ps1 (external) = $(Ash1*h1/s1)/Asec = 0.00392699$
Ash1 = $Astir_1*ns_1 = 157.0796$
No stirrups, ns_1 = 2.00
h1 = 750.00
ps2 (internal) = $(Ash2*h2/s2)/Asec = 0.00073723$
Ash2 = $Astir_2*ns_2 = 100.531$
No stirrups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/d = 0.30
su1 = $0.4*esu1_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = (fs_jacket \cdot Asl,ten,jacket + fs_core \cdot Asl,ten,core) / Asl,ten = 389.0139$

with $Es1 = (Es_jacket \cdot Asl,ten,jacket + Es_core \cdot Asl,ten,core) / Asl,ten = 200000.00$

$y2 = 0.00140044$

$sh2 = 0.0044814$

$ft2 = 466.8167$

$fy2 = 389.0139$

$su2 = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with $shear_factor$

and also multiplied by the $shear_factor$ according to 15.7.1.4, with

$Shear_factor = 1.00$

$lo/lou,min = lb/lb,min = 0.30$

$su2 = 0.4 \cdot esu2_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = (fs_jacket \cdot Asl,com,jacket + fs_core \cdot Asl,com,core) / Asl,com = 389.0139$

with $Es2 = (Es_jacket \cdot Asl,com,jacket + Es_core \cdot Asl,com,core) / Asl,com = 200000.00$

$yv = 0.00140044$

$shv = 0.0044814$

$ftv = 466.8167$

$fyv = 389.0139$

$suv = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with $shear_factor$

and also multiplied by the $shear_factor$ according to 15.7.1.4, with

$Shear_factor = 1.00$

$lo/lou,min = lb/ld = 0.30$

$suv = 0.4 \cdot esuv_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esuv_nominal = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY

For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered

characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fsv = (fs_jacket \cdot Asl,mid,jacket + fs_mid \cdot Asl,mid,core) / Asl,mid = 389.0139$

with $Es_v = (Es_jacket \cdot Asl,mid,jacket + Es_mid \cdot Asl,mid,core) / Asl,mid = 200000.00$

$1 = Asl,ten / (b \cdot d) \cdot (fs1 / fc) = 0.0500703$

$2 = Asl,com / (b \cdot d) \cdot (fs2 / fc) = 0.0500703$

$v = Asl,mid / (b \cdot d) \cdot (fsv / fc) = 0.01770442$

and confined core properties:

$b = 690.00$

$d = 327.00$

$d' = 13.00$

$fcc (5A.2, TBDY) = 34.43547$

$cc (5A.5, TBDY) = 0.00243499$

$c = \text{confinement factor} = 1.0435$

$1 = Asl,ten / (b \cdot d) \cdot (fs1 / fc) = 0.05941729$

$2 = Asl,com / (b \cdot d) \cdot (fs2 / fc) = 0.05941729$

$v = Asl,mid / (b \cdot d) \cdot (fsv / fc) = 0.02100943$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < vs,y2$ - LHS eq.(4.5) is satisfied

--->

$su (4.9) = 0.16886055$

$Mu = MRc (4.14) = 1.8158E+008$

$u = su (4.1) = 1.7255512E-005$

Calculation of ratio lb/ld

Inadequate Lap Length with $lb/ld = 0.30$

Calculation of Mu2-

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:

$$\mu = 1.7255512E-005$$

$$\text{Mu} = 1.8158E+008$$

with full section properties:

$$b = 750.00$$

$$d = 357.00$$

$$d' = 43.00$$

$$v = 0.00124684$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$c_o (5A.5, \text{TBDY}) = 0.002$$

$$\text{Final value of } c_u: c_u^* = \text{shear_factor} * \text{Max}(c_u, c_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } c_u = 0.01006403$$

$$w_e ((5.4c), \text{TBDY}) = a_{se} * s_{h,\min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$$

where $f = a_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.02504965$$

$$a_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/bw = 0.00266667$$

$$bw = 750.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 800.0873$$

$$f_y = 0.04491794$$

$$a_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/bw = 0.005$$

$$bw = 400.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL * t * \text{Cos}(b_1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{,f} = 0.015$$

$$a_{se} ((5.4d), \text{TBDY}) = (a_{se1} * A_{ext} + a_{se2} * A_{int}) / A_{sec} = 0.12601038$$

$$a_{se1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i2,1} = 1.1834E+006$$

$$a_{se2} = \text{Max}(a_{se1}, a_{se2}) = 0.12601038$$

$$b_{o,2} = 542.00$$

$$h_{o,2} = 192.00$$

$$b_{i2,2} = 661256.00$$

$$p_{sh,\min} * F_{ywe} = \text{Min}(p_{sh,x} * F_{ywe}, p_{sh,y} * F_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $p_{sh,\min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$p_{sh,x} * F_{ywe} = p_{s1} * F_{ywe1} + p_{s2} * F_{ywe2} = 1.64062$$

$$p_{s1} (\text{external}) = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$$

$$A_{sh1} = A_{stir,1} * n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

$$h_1 = 400.00$$

$$p_{s2} (\text{internal}) = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00026808$$

$$A_{sh2} = A_{stir,2} * n_{s,2} = 100.531$$

$$\text{No stirrups, } n_{s,2} = 2.00$$

$$h_2 = 200.00$$

psh_y*Fywe = psh₁*Fywe₁+ps₂*Fywe₂ = 3.23907
ps₁ (external) = (Ash₁*h₁/s₁)/Asec = 0.00392699
Ash₁ = Astir₁*ns₁ = 157.0796
No stirups, ns₁ = 2.00
h₁ = 750.00
ps₂ (internal) = (Ash₂*h₂/s₂)/Asec = 0.00073723
Ash₂ = Astir₂*ns₂ = 100.531
No stirups, ns₂ = 2.00
h₂ = 550.00

Asec = 300000.00

s₁ = 100.00

s₂ = 250.00

fywe₁ = 694.45

fywe₂ = 694.45

fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499

c = confinement factor = 1.0435

y₁ = 0.00140044

sh₁ = 0.0044814

ft₁ = 466.8167

fy₁ = 389.0139

su₁ = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

su₁ = 0.4*esu_{1_nominal} ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu_{1_nominal} = 0.08,

For calculation of esu_{1_nominal} and y₁, sh₁,ft₁,fy₁, it is considered
characteristic value fsy₁ = fs₁/1.2, from table 5.1, TBDY.

y₁, sh₁,ft₁,fy₁, are also multiplied by Min(1,1.25*(lb/ld)^{2/3}), from 10.3.5, ASCE 41-17.

with fs₁ = (fs_{jacket}*Asl_{ten,jacket} + fs_{core}*Asl_{ten,core})/Asl_{ten} = 389.0139

with Es₁ = (Es_{jacket}*Asl_{ten,jacket} + Es_{core}*Asl_{ten,core})/Asl_{ten} = 200000.00

y₂ = 0.00140044

sh₂ = 0.0044814

ft₂ = 466.8167

fy₂ = 389.0139

su₂ = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su₂ = 0.4*esu_{2_nominal} ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu_{2_nominal} = 0.08,

For calculation of esu_{2_nominal} and y₂, sh₂,ft₂,fy₂, it is considered
characteristic value fsy₂ = fs₂/1.2, from table 5.1, TBDY.

y₂, sh₂,ft₂,fy₂, are also multiplied by Min(1,1.25*(lb/ld)^{2/3}), from 10.3.5, ASCE 41-17.

with fs₂ = (fs_{jacket}*Asl_{com,jacket} + fs_{core}*Asl_{com,core})/Asl_{com} = 389.0139

with Es₂ = (Es_{jacket}*Asl_{com,jacket} + Es_{core}*Asl_{com,core})/Asl_{com} = 200000.00

y_v = 0.00140044

sh_v = 0.0044814

ft_v = 466.8167

fy_v = 389.0139

suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

suv = 0.4*esuv_{nominal} ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_{nominal} = 0.08,

considering characteristic value fsy_v = fs_v/1.2, from table 5.1, TBDY

For calculation of esuv_{nominal} and y_v, sh_v,ft_v,fy_v, it is considered
characteristic value fsy_v = fs_v/1.2, from table 5.1, TBDY.

y₁, sh₁,ft₁,fy₁, are also multiplied by Min(1,1.25*(lb/ld)^{2/3}), from 10.3.5, ASCE 41-17.

with fs_v = (fs_{jacket}*Asl_{mid,jacket} + fs_{mid}*Asl_{mid,core})/Asl_{mid} = 389.0139

with Es_v = (Es_{jacket}*Asl_{mid,jacket} + Es_{mid}*Asl_{mid,core})/Asl_{mid} = 200000.00

$$1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.0500703$$

$$2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.0500703$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.01770442$$

and confined core properties:

$$b = 690.00$$

$$d = 327.00$$

$$d' = 13.00$$

$$f_{cc} (5A.2, TBDY) = 34.43547$$

$$c_c (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.05941729$$

$$2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.05941729$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.02100943$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$s_u (4.9) = 0.16886055$$

$$\mu_u = M/R_c (4.14) = 1.8158E+008$$

$$u = s_u (4.1) = 1.7255512E-005$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 848607.437$

Calculation of Shear Strength at edge 1, $V_{r1} = 848607.437$

$V_{r1} = V_{Col} ((10.3), ASCE 41-17) = k_{nl} * V_{Col0}$

$$V_{Col0} = 848607.437$$

$k_{nl} = 1$ (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v * f_y * d / s$ ' is replaced by ' $V_{s+} = f * V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: $f'_c = (f'_{c,jacket} * Area_{jacket} + f'_{c,core} * Area_{core}) / Area_{section} = 33.00$, but $f_c^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$$M/Vd = 2.00$$

$$\mu_u = 3.5345895E-012$$

$$V_u = 3.4586346E-031$$

$$d = 0.8 * h = 320.00$$

$$N_u = 11016.808$$

$$A_g = 300000.00$$

$$\text{From (11.5.4.8), ACI 318-14: } V_s = V_{s1} + V_{s2} = 279254.914$$

where:

$V_{s1} = 279254.914$ is calculated for jacket, with:

$$d = 320.00$$

$$A_v = 157079.633$$

$$f_y = 555.56$$

$$s = 100.00$$

V_{s1} is multiplied by $Col1 = 1.00$

$$s/d = 0.3125$$

$V_{s2} = 0.00$ is calculated for core, with:

$$d = 160.00$$

$$A_v = 100530.965$$

$$f_y = 555.56$$

$$s = 250.00$$

V_{s2} is multiplied by $Col2 = 0.00$

$$s/d = 1.5625$$

$V_f ((11-3)-(11.4), ACI 440) = 234192.00$

$f = 0.95$, for fully-wrapped sections

$wf/sf = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin \theta + \cos \theta$ is replaced with $(\cot \theta + \csc \theta) \sin \alpha$ which is more a generalised expression, where θ is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, \alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, \theta)|)$, with:

total thickness per orientation, $tf1 = NL * t / \text{NoDir} = 1.00$

$dfv = d$ (figure 11.2, ACI 440) = 357.00

ffe ((11-5), ACI 440) = 328.00

$E_f = 82000.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.009$

From (11-11), ACI 440: $V_s + V_f \leq 915872.391$

$bw = 750.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 848607.437$

$V_{r2} = V_{Co1}$ ((10.3), ASCE 41-17) = $k_n l * V_{Co1}$

$V_{Co1} = 848607.437$

$k_n l = 1$ (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v * f_y * d / s$ ' is replaced by ' $V_s + f * V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

$\lambda = 1$ (normal-weight concrete)

Mean concrete strength: $f'_c = (f'_{c_jacket} * \text{Area}_{jacket} + f'_{c_core} * \text{Area}_{core}) / \text{Area}_{section} = 33.00$, but $f'_c^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$M / Vd = 2.00$

$\mu_u = 3.5345895E-012$

$\nu_u = 3.4586346E-031$

$d = 0.8 * h = 320.00$

$N_u = 11016.808$

$A_g = 300000.00$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 279254.914$

where:

$V_{s1} = 279254.914$ is calculated for jacket, with:

$d = 320.00$

$A_v = 157079.633$

$f_y = 555.56$

$s = 100.00$

V_{s1} is multiplied by $Col1 = 1.00$

$s/d = 0.3125$

$V_{s2} = 0.00$ is calculated for core, with:

$d = 160.00$

$A_v = 100530.965$

$f_y = 555.56$

$s = 250.00$

V_{s2} is multiplied by $Col2 = 0.00$

$s/d = 1.5625$

V_f ((11-3)-(11.4), ACI 440) = 234192.00

$f = 0.95$, for fully-wrapped sections

$wf/sf = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin \theta + \cos \theta$ is replaced with $(\cot \theta + \csc \theta) \sin \alpha$ which is more a generalised expression, where θ is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, \alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, \theta)|)$, with:

total thickness per orientation, $tf1 = NL * t / \text{NoDir} = 1.00$

$dfv = d$ (figure 11.2, ACI 440) = 357.00

ffe ((11-5), ACI 440) = 328.00

$E_f = 82000.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.009$

From (11-11), ACI 440: $V_s + V_f \leq 915872.391$

$bw = 750.00$

End Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1
At local axis: 3

Start Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1
At Shear local axis: 2
(Bending local axis: 3)
Section Type: rcjrs

Constant Properties

Knowledge Factor, $\phi = 0.90$
Mean strength values are used for both shear and moment calculations.
Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$
New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$
Concrete Elasticity, $E_c = 26999.444$
Steel Elasticity, $E_s = 200000.00$

Existing Column

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$
New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$
Concrete Elasticity, $E_c = 26999.444$
Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of moment strengths,
the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

Jacket

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

Existing Column

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

#####

External Height, $H = 400.00$

External Width, $W = 750.00$

Internal Height, $H = 200.00$

Internal Width, $W = 550.00$

Cover Thickness, $c = 25.00$

Mean Confinement Factor overall section = 1.0435

Element Length, $L = 3000.00$

Primary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Inadequate Lap Length with $l_o/l_{ou, \min} = 0.30$

FRP Wrapping Data

Type: Carbon

Dry properties (design values)

Thickness, $t = 1.00$

Tensile Strength, $f_{fu} = 840.00$

Tensile Modulus, $E_f = 82000.00$

Elongation, $e_{fu} = 0.009$

Number of directions, $NoDir = 1$

Fiber orientations, $bi: 0.00^\circ$

Number of layers, $NL = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 2

EDGE -A-

Shear Force, $V_a = 2.9582284E-031$

EDGE -B-

Shear Force, $V_b = -2.9582284E-031$

BOTH EDGES

Axial Force, $F = -11016.808$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $A_{st} = 0.00$

-Compression: $A_{sc} = 2676.637$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $A_{s,ten} = 1137.257$

-Compression: $A_{s,com} = 1137.257$

-Middle: $A_{s,mid} = 402.1239$

Calculation of Shear Capacity ratio, $V_e/V_r = 0.19671905$

Member Controlled by Flexure ($V_e/V_r < 1$)

Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 248405.514$

with

$M_{pr1} = \text{Max}(M_{u1+}, M_{u1-}) = 3.7261E+008$

$M_{u1+} = 3.7261E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination

$M_{u1-} = 3.7261E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination

$M_{pr2} = \text{Max}(M_{u2+}, M_{u2-}) = 3.7261E+008$

$M_{u2+} = 3.7261E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the the static loading combination

$M_{u2-} = 3.7261E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the the static loading combination

Calculation of M_{u1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$\phi_u = 8.5888248E-006$

$M_u = 3.7261E+008$

with full section properties:

$b = 400.00$

$d = 707.00$

$d' = 43.00$

$v = 0.00118049$

$N = 11016.808$

$f_c = 33.00$

$\alpha_{co} (5A.5, TBDY) = 0.002$

Final value of ϕ_u : $\phi_u^* = \text{shear_factor} * \text{Max}(\phi_u, \phi_c) = 0.01006403$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $\phi_u = 0.01006403$

where $\phi_u (5.4c, TBDY) = \alpha_{se} * \text{sh}_{,min} * f_{ywe}/f_{ce} + \text{Min}(\phi_x, \phi_y) = 0.02692906$

where $\phi_x = \alpha_{se} * \phi_{f,ff} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$\phi_x = 0.02504965$

$\alpha_{se} = 0.38744444$

$b = 750.00$

$h = 400.00$

From EC8 A.4.4.3(6), $\phi_{f,ff} = 2t_f/b_w = 0.00266667$

$b_w = 750.00$

effective stress from (A.35), $f_{f,e} = 800.0873$

$\phi_y = 0.04491794$

$\alpha_{se} = 0.38744444$

$b = 400.00$

$h = 750.00$

From EC8 A.4.4.3(6), $\phi_{f,ff} = 2t_f/b_w = 0.005$

$b_w = 400.00$

effective stress from (A.35), $f_{f,e} = 765.1636$

R = 40.00

Effective FRP thickness, $t_f = NL * t * \cos(b_1) = 1.00$

$f_{u,f} = 840.00$

$E_f = 82000.00$

$u_{,f} = 0.015$

$ase((5.4d), TBDY) = (ase_1 * A_{ext} + ase_2 * A_{int}) / A_{sec} = 0.12601038$

$ase_1 = 0.12601038$

$bo_1 = 690.00$

$ho_1 = 340.00$

$bi_{2,1} = 1.1834E+006$

$ase_2 = \max(ase_1, ase_2) = 0.12601038$

$bo_2 = 542.00$

$ho_2 = 192.00$

$bi_{2,2} = 661256.00$

$p_{sh,min} * F_{ywe} = \min(p_{sh,x} * F_{ywe}, p_{sh,y} * F_{ywe}) = 1.64062$

Expression ((5.4d), TBDY) for $p_{sh,min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$p_{sh,x} * F_{ywe} = p_{sh1} * F_{ywe1} + ps_2 * F_{ywe2} = 1.64062$

ps_1 (external) = $(A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$

$A_{sh1} = A_{stir,1} * ns_1 = 157.0796$

No stirrups, $ns_1 = 2.00$

$h_1 = 400.00$

ps_2 (internal) = $(A_{sh2} * h_2 / s_2) / A_{sec} = 0.00026808$

$A_{sh2} = A_{stir,2} * ns_2 = 100.531$

No stirrups, $ns_2 = 2.00$

$h_2 = 200.00$

$p_{sh,y} * F_{ywe} = p_{sh1} * F_{ywe1} + ps_2 * F_{ywe2} = 3.23907$

ps_1 (external) = $(A_{sh1} * h_1 / s_1) / A_{sec} = 0.00392699$

$A_{sh1} = A_{stir,1} * ns_1 = 157.0796$

No stirrups, $ns_1 = 2.00$

$h_1 = 750.00$

ps_2 (internal) = $(A_{sh2} * h_2 / s_2) / A_{sec} = 0.00073723$

$A_{sh2} = A_{stir,2} * ns_2 = 100.531$

No stirrups, $ns_2 = 2.00$

$h_2 = 550.00$

$A_{sec} = 300000.00$

$s_1 = 100.00$

$s_2 = 250.00$

$f_{ywe1} = 694.45$

$f_{ywe2} = 694.45$

$f_{ce} = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00243499$

c = confinement factor = 1.0435

$y_1 = 0.00140044$

$sh_1 = 0.0044814$

$ft_1 = 466.8167$

$fy_1 = 389.0139$

$su_1 = 0.00512$

using (30) in Bisikinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$l_0 / l_{ou,min} = l_b / l_d = 0.30$

$su_1 = 0.4 * esu_{1,nominal}((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu_{1,nominal} = 0.08$,

For calculation of $esu_{1,nominal}$ and y_1, sh_1, ft_1, fy_1 , it is considered characteristic value $fs_1 = fs_1 / 1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\min(1, 1.25 * (l_b / l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs_1 = (f_{s,jacket} * A_{s1,ten,jacket} + f_{s,core} * A_{s1,ten,core}) / A_{s1,ten} = 389.0139$

with $Es_1 = (E_{s,jacket} * A_{s1,ten,jacket} + E_{s,core} * A_{s1,ten,core}) / A_{s1,ten} = 200000.00$

$y_2 = 0.00140044$

$sh_2 = 0.0044814$

$ft_2 = 466.8167$

$$f_y2 = 389.0139$$

$$s_u2 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$l_o/l_{o,u,min} = l_b/l_{b,min} = 0.30$$

$$s_u2 = 0.4 * e_{su2,nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $e_{su2,nominal} = 0.08$,

For calculation of $e_{su2,nominal}$ and y_2 , sh_2, ft_2, f_y2 , it is considered
characteristic value $f_{sy2} = f_s2/1.2$, from table 5.1, TBDY.

y_1 , sh_1, ft_1, f_y1 , are also multiplied by $\text{Min}(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } f_{s2} = (f_{s,jacket} * A_{s1,com,jacket} + f_{s,core} * A_{s1,com,core}) / A_{s1,com} = 389.0139$$

$$\text{with } E_{s2} = (E_{s,jacket} * A_{s1,com,jacket} + E_{s,core} * A_{s1,com,core}) / A_{s1,com} = 200000.00$$

$$y_v = 0.00140044$$

$$sh_v = 0.0044814$$

$$ft_v = 466.8167$$

$$f_{y_v} = 389.0139$$

$$s_{u_v} = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$l_o/l_{o,u,min} = l_b/l_d = 0.30$$

$$s_{u_v} = 0.4 * e_{suv,nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $e_{suv,nominal} = 0.08$,

considering characteristic value $f_{syv} = f_{sv}/1.2$, from table 5.1, TBDY

For calculation of $e_{suv,nominal}$ and y_v , sh_v, ft_v, f_{y_v} , it is considered
characteristic value $f_{syv} = f_{sv}/1.2$, from table 5.1, TBDY.

y_1 , sh_1, ft_1, f_{y1} , are also multiplied by $\text{Min}(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } f_{sv} = (f_{s,jacket} * A_{s1,mid,jacket} + f_{s,mid} * A_{s1,mid,core}) / A_{s1,mid} = 389.0139$$

$$\text{with } E_{sv} = (E_{s,jacket} * A_{s1,mid,jacket} + E_{s,mid} * A_{s1,mid,core}) / A_{s1,mid} = 200000.00$$

$$1 = A_{s1,ten} / (b * d) * (f_{s1} / f_c) = 0.04740567$$

$$2 = A_{s1,com} / (b * d) * (f_{s2} / f_c) = 0.04740567$$

$$v = A_{s1,mid} / (b * d) * (f_{sv} / f_c) = 0.01676222$$

and confined core properties:

$$b = 340.00$$

$$d = 677.00$$

$$d' = 13.00$$

$$f_{cc} (5A.2, TBDY) = 34.43547$$

$$c_c (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = A_{s1,ten} / (b * d) * (f_{s1} / f_c) = 0.05824278$$

$$2 = A_{s1,com} / (b * d) * (f_{s2} / f_c) = 0.05824278$$

$$v = A_{s1,mid} / (b * d) * (f_{sv} / f_c) = 0.02059413$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$s_u (4.9) = 0.15682678$$

$$M_u = M_{Rc} (4.14) = 3.7261E+008$$

$$u = s_u (4.1) = 8.5888248E-006$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of M_{u1} -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$$u = 8.5888248E-006$$

$$M_u = 3.7261E+008$$

with full section properties:

$$b = 400.00$$

$$d = 707.00$$

$$d' = 43.00$$

$$v = 0.00118049$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$\alpha (5A.5, TBDY) = 0.002$$

$$\text{Final value of } \alpha: \alpha^* = \text{shear_factor} * \text{Max}(\alpha, \alpha_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \alpha = 0.01006403$$

$$\alpha_{we} ((5.4c), TBDY) = \alpha_{se} * \text{sh}_{\min} * f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$$

where $f = \alpha * p_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.02504965$$

$$\alpha_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f / b_w = 0.00266667$$

$$b_w = 750.00$$

$$\text{effective stress from (A.35), } f_{fe} = 800.0873$$

$$f_y = 0.04491794$$

$$\alpha_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f / b_w = 0.005$$

$$b_w = 400.00$$

$$\text{effective stress from (A.35), } f_{fe} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL * t * \text{Cos}(b_1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{,f} = 0.015$$

$$\alpha_{se} ((5.4d), TBDY) = (\alpha_{se1} * A_{ext} + \alpha_{se2} * A_{int}) / A_{sec} = 0.12601038$$

$$\alpha_{se1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i2,1} = 1.1834E+006$$

$$\alpha_{se2} = \text{Max}(\alpha_{se1}, \alpha_{se2}) = 0.12601038$$

$$b_{o,2} = 542.00$$

$$h_{o,2} = 192.00$$

$$b_{i2,2} = 661256.00$$

$$p_{sh,\min} * F_{ywe} = \text{Min}(p_{sh,x} * F_{ywe}, p_{sh,y} * F_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $p_{sh,\min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$p_{sh,x} * F_{ywe} = p_{sh1} * F_{ywe1} + p_{sh2} * F_{ywe2} = 1.64062$$

$$p_{s1} (\text{external}) = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$$

$$A_{sh1} = A_{stir,1} * n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

$$h_1 = 400.00$$

$$p_{s2} (\text{internal}) = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00026808$$

$$A_{sh2} = A_{stir,2} * n_{s,2} = 100.531$$

$$\text{No stirrups, } n_{s,2} = 2.00$$

$$h_2 = 200.00$$

$$p_{sh,y} * F_{ywe} = p_{sh1} * F_{ywe1} + p_{sh2} * F_{ywe2} = 3.23907$$

$$p_{s1} (\text{external}) = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.00392699$$

$$A_{sh1} = A_{stir,1} * n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

$$h_1 = 750.00$$

$$p_{s2} (\text{internal}) = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00073723$$

$$A_{sh2} = A_{stir,2} * n_{s,2} = 100.531$$

$$\text{No stirrups, } n_{s,2} = 2.00$$

$$h_2 = 550.00$$

Asec = 300000.00

s1 = 100.00

s2 = 250.00

fywe1 = 694.45

fywe2 = 694.45

fce = 33.00

From ((5.A.5), TBDY), TBDY: cc = 0.00243499

c = confinement factor = 1.0435

y1 = 0.00140044

sh1 = 0.0044814

ft1 = 466.8167

fy1 = 389.0139

su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = (fsjacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044

sh2 = 0.0044814

ft2 = 466.8167

fy2 = 389.0139

su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044

shv = 0.0044814

ftv = 466.8167

fyv = 389.0139

suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered

characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.04740567

2 = Asl,com/(b*d)*(fs2/fc) = 0.04740567

v = Asl,mid/(b*d)*(fsv/fc) = 0.01676222

and confined core properties:

b = 340.00

d = 677.00

d' = 13.00

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

1 = $Asl_{ten}/(b*d)*(fs1/fc) = 0.05824278$

2 = $Asl_{com}/(b*d)*(fs2/fc) = 0.05824278$

v = $Asl_{mid}/(b*d)*(fsv/fc) = 0.02059413$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

v < $v_{s,y2}$ - LHS eq.(4.5) is satisfied

su (4.9) = 0.15682678

Mu = MRc (4.14) = 3.7261E+008

u = su (4.1) = 8.5888248E-006

Calculation of ratio lb/d

Inadequate Lap Length with lb/d = 0.30

Calculation of Mu2+

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

u = 8.5888248E-006

Mu = 3.7261E+008

with full section properties:

b = 400.00

d = 707.00

d' = 43.00

v = 0.00118049

N = 11016.808

fc = 33.00

co (5A.5, TBDY) = 0.002

Final value of cu: $cu^* = shear_factor * Max(cu, cc) = 0.01006403$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $cu = 0.01006403$

we ((5.4c), TBDY) = $ase * sh_{,min} * fy_{we}/f_{ce} + Min(f_x, f_y) = 0.02692906$

where $f = af * pf * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

 $f_x = 0.02504965$

$af = 0.38744444$

b = 750.00

h = 400.00

From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.00266667$

bw = 750.00

effective stress from (A.35), $ff_e = 800.0873$

 $f_y = 0.04491794$

$af = 0.38744444$

b = 400.00

h = 750.00

From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.005$

bw = 400.00

effective stress from (A.35), $ff_e = 765.1636$

R = 40.00

Effective FRP thickness, $tf = NL * t * Cos(b1) = 1.00$

$f_{u,f} = 840.00$

$E_f = 82000.00$

$u_{,f} = 0.015$

ase ((5.4d), TBDY) = $(ase1 * A_{ext} + ase2 * A_{int}) / A_{sec} = 0.12601038$

ase1 = 0.12601038

bo_1 = 690.00

ho_1 = 340.00

bi2_1 = 1.1834E+006
ase2 = Max(ase1,ase2) = 0.12601038
bo_2 = 542.00
ho_2 = 192.00
bi2_2 = 661256.00

psh,min*Fywe = Min(psh,x*Fywe , psh,y*Fywe) = 1.64062

Expression ((5.4d), TBDY) for psh,min*Fywe has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh_x*Fywe = psh1*Fywe1+ps2*Fywe2 = 1.64062
ps1 (external) = (Ash1*h1/s1)/Asec = 0.0020944
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 400.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00026808
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 200.00

psh_y*Fywe = psh1*Fywe1+ps2*Fywe2 = 3.23907
ps1 (external) = (Ash1*h1/s1)/Asec = 0.00392699
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 750.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00073723
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb,min)^ 2/3), from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044
sh2 = 0.0044814
ft2 = 466.8167
fy2 = 389.0139
su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb,min)^ 2/3), from 10.3.5, ASCE 41-17.

with $f_{s2} = (f_{s,jacket} \cdot A_{s,com,jacket} + f_{s,core} \cdot A_{s,com,core}) / A_{s,com} = 389.0139$
 with $E_{s2} = (E_{s,jacket} \cdot A_{s,com,jacket} + E_{s,core} \cdot A_{s,com,core}) / A_{s,com} = 200000.00$
 $y_v = 0.00140044$
 $sh_v = 0.0044814$
 $ft_v = 466.8167$
 $fy_v = 389.0139$
 $suv = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_o/l_{ou,min} = l_b/l_d = 0.30$
 $suv = 0.4 \cdot esuv_{nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_{nominal} = 0.08$,
 considering characteristic value $f_{syv} = f_{sv}/1.2$, from table 5.1, TBDY
 For calculation of $esuv_{nominal}$ and y_v, sh_v, ft_v, fy_v , it is considered
 characteristic value $f_{syv} = f_{sv}/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $f_{sv} = (f_{s,jacket} \cdot A_{s,mid,jacket} + f_{s,mid} \cdot A_{s,mid,core}) / A_{s,mid} = 389.0139$
 with $E_{sv} = (E_{s,jacket} \cdot A_{s,mid,jacket} + E_{s,mid} \cdot A_{s,mid,core}) / A_{s,mid} = 200000.00$
 $1 = A_{s,ten} / (b \cdot d) \cdot (f_{s1} / f_c) = 0.04740567$
 $2 = A_{s,com} / (b \cdot d) \cdot (f_{s2} / f_c) = 0.04740567$
 $v = A_{s,mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.01676222$

and confined core properties:

$b = 340.00$
 $d = 677.00$
 $d' = 13.00$
 $f_{cc} (5A.2, TBDY) = 34.43547$
 $cc (5A.5, TBDY) = 0.00243499$
 $c = \text{confinement factor} = 1.0435$
 $1 = A_{s,ten} / (b \cdot d) \cdot (f_{s1} / f_c) = 0.05824278$
 $2 = A_{s,com} / (b \cdot d) \cdot (f_{s2} / f_c) = 0.05824278$
 $v = A_{s,mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.02059413$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

$su (4.9) = 0.15682678$
 $Mu = MRc (4.14) = 3.7261E+008$
 $u = su (4.1) = 8.5888248E-006$

 Calculation of ratio l_b/l_d

 Inadequate Lap Length with $l_b/l_d = 0.30$

 Calculation of Mu_2 -

 Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 8.5888248E-006$
 $Mu = 3.7261E+008$

 with full section properties:

$b = 400.00$
 $d = 707.00$
 $d' = 43.00$
 $v = 0.00118049$
 $N = 11016.808$
 $f_c = 33.00$
 $co (5A.5, TBDY) = 0.002$
 Final value of cu : $cu^* = \text{shear_factor} \cdot \text{Max}(cu, cc) = 0.01006403$
 The Shear_factor is considered equal to 1 (pure moment strength)
 From (5.4b), TBDY: $cu = 0.01006403$

w_e ((5.4c), TBDY) = $a_{se} \cdot s_{h,min} \cdot f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$
where $f = a_f \cdot p_f \cdot f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.02504965$
 $a_f = 0.38744444$
 $b = 750.00$
 $h = 400.00$
From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.00266667$
 $bw = 750.00$
effective stress from (A.35), $f_{f,e} = 800.0873$

$f_y = 0.04491794$
 $a_f = 0.38744444$
 $b = 400.00$
 $h = 750.00$
From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.005$
 $bw = 400.00$
effective stress from (A.35), $f_{f,e} = 765.1636$

$R = 40.00$
Effective FRP thickness, $t_f = NL \cdot t \cdot \text{Cos}(b_1) = 1.00$
 $f_{u,f} = 840.00$
 $E_f = 82000.00$
 $u_{,f} = 0.015$
 $a_{se} \text{ ((5.4d), TBDY)} = (a_{se1} \cdot A_{ext} + a_{se2} \cdot A_{int})/A_{sec} = 0.12601038$
 $a_{se1} = 0.12601038$
 $bo_1 = 690.00$
 $ho_1 = 340.00$
 $bi2_1 = 1.1834E+006$
 $a_{se2} = \text{Max}(a_{se1}, a_{se2}) = 0.12601038$
 $bo_2 = 542.00$
 $ho_2 = 192.00$
 $bi2_2 = 661256.00$

$p_{sh,min} \cdot F_{ywe} = \text{Min}(p_{sh,x} \cdot F_{ywe}, p_{sh,y} \cdot F_{ywe}) = 1.64062$
Expression ((5.4d), TBDY) for $p_{sh,min} \cdot F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$p_{sh,x} \cdot F_{ywe} = p_{sh1} \cdot F_{ywe1} + p_{sh2} \cdot F_{ywe2} = 1.64062$
 $ps1$ (external) = $(A_{sh1} \cdot h_1/s_1)/A_{sec} = 0.0020944$
 $A_{sh1} = A_{stir_1} \cdot ns_1 = 157.0796$
No stirrups, $ns_1 = 2.00$
 $h_1 = 400.00$
 $ps2$ (internal) = $(A_{sh2} \cdot h_2/s_2)/A_{sec} = 0.00026808$
 $A_{sh2} = A_{stir_2} \cdot ns_2 = 100.531$
No stirrups, $ns_2 = 2.00$
 $h_2 = 200.00$

$p_{sh,y} \cdot F_{ywe} = p_{sh1} \cdot F_{ywe1} + p_{sh2} \cdot F_{ywe2} = 3.23907$
 $ps1$ (external) = $(A_{sh1} \cdot h_1/s_1)/A_{sec} = 0.00392699$
 $A_{sh1} = A_{stir_1} \cdot ns_1 = 157.0796$
No stirrups, $ns_1 = 2.00$
 $h_1 = 750.00$
 $ps2$ (internal) = $(A_{sh2} \cdot h_2/s_2)/A_{sec} = 0.00073723$
 $A_{sh2} = A_{stir_2} \cdot ns_2 = 100.531$
No stirrups, $ns_2 = 2.00$
 $h_2 = 550.00$

$A_{sec} = 300000.00$
 $s_1 = 100.00$
 $s_2 = 250.00$
 $f_{ywe1} = 694.45$
 $f_{ywe2} = 694.45$
 $f_{ce} = 33.00$
From ((5.A5), TBDY), TBDY: $cc = 0.00243499$
 $c = \text{confinement factor} = 1.0435$
 $y_1 = 0.00140044$
 $sh_1 = 0.0044814$

$$ft1 = 466.8167$$

$$fy1 = 389.0139$$

$$su1 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lou,min = lb/ld = 0.30$$

$$su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25*(lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139$$

$$\text{with } Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00$$

$$y2 = 0.00140044$$

$$sh2 = 0.0044814$$

$$ft2 = 466.8167$$

$$fy2 = 389.0139$$

$$su2 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lou,min = lb/lb,min = 0.30$$

$$su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25*(lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139$$

$$\text{with } Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00$$

$$yv = 0.00140044$$

$$shv = 0.0044814$$

$$ftv = 466.8167$$

$$fyv = 389.0139$$

$$suv = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lou,min = lb/ld = 0.30$$

$$suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25*(lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139$$

$$\text{with } Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00$$

$$1 = Asl,ten/(b*d)*(fs1/fc) = 0.04740567$$

$$2 = Asl,com/(b*d)*(fs2/fc) = 0.04740567$$

$$v = Asl,mid/(b*d)*(fsv/fc) = 0.01676222$$

and confined core properties:

$$b = 340.00$$

$$d = 677.00$$

$$d' = 13.00$$

$$fcc (5A.2, TBDY) = 34.43547$$

$$cc (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = Asl,ten/(b*d)*(fs1/fc) = 0.05824278$$

$$2 = Asl,com/(b*d)*(fs2/fc) = 0.05824278$$

$$v = Asl,mid/(b*d)*(fsv/fc) = 0.02059413$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

v < vs,y2 - LHS eq.(4.5) is satisfied

--->

$$su (4.9) = 0.15682678$$

$$Mu = MRc (4.14) = 3.7261E+008$$

$$u = s_u(4.1) = 8.5888248E-006$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 1.2627E+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 1.2627E+006$

$V_{r1} = V_{Col}((10.3), \text{ASCE 41-17}) = k_{nl} * V_{Col0}$

$$V_{Col0} = 1.2627E+006$$

$k_{nl} = 1$ (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v * f_y * d / s$ ' is replaced by ' $V_s + f * V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

$= 1$ (normal-weight concrete)

Mean concrete strength: $f_c' = (f_c'_{jacket} * \text{Area}_{jacket} + f_c'_{core} * \text{Area}_{core}) / \text{Area}_{section} = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$$M/Vd = 2.00$$

$$M_u = 2.3795529E-012$$

$$V_u = 2.9582284E-031$$

$$d = 0.8 * h = 600.00$$

$$N_u = 11016.808$$

$$A_g = 300000.00$$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 621900.694$

where:

$V_{s1} = 523602.964$ is calculated for jacket, with:

$$d = 600.00$$

$$A_v = 157079.633$$

$$f_y = 555.56$$

$$s = 100.00$$

V_{s1} is multiplied by $Col1 = 1.00$

$$s/d = 0.16666667$$

$V_{s2} = 98297.73$ is calculated for core, with:

$$d = 440.00$$

$$A_v = 100530.965$$

$$f_y = 555.56$$

$$s = 250.00$$

V_{s2} is multiplied by $Col2 = 1.00$

$$s/d = 0.56818182$$

V_f ((11-3)-(11.4), ACI 440) = 463792.00

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \csc) \sin \alpha$ which is more a generalised expression, where α is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\alpha, \theta)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, \theta)|)$, with:

total thickness per orientation, $t_{f1} = N_L * t / \text{NoDir} = 1.00$

$d_{fv} = d$ (figure 11.2, ACI 440) = 707.00

f_{fe} ((11-5), ACI 440) = 328.00

$$E_f = 82000.00$$

$$f_e = 0.004, \text{ from (11.6a), ACI 440}$$

$$\text{with } f_u = 0.009$$

From (11-11), ACI 440: $V_s + V_f \leq 915872.391$

$$b_w = 400.00$$

Calculation of Shear Strength at edge 2, $V_{r2} = 1.2627E+006$

$V_{r2} = V_{Col}((10.3), \text{ASCE 41-17}) = k_{nl} * V_{Col0}$

$$V_{Col0} = 1.2627E+006$$

knl = 1 (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v \cdot f_y \cdot d / s$ ' is replaced by ' $V_s + f \cdot V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: $f_c' = (f_c'_{\text{jacket}} \cdot \text{Area}_{\text{jacket}} + f_c'_{\text{core}} \cdot \text{Area}_{\text{core}}) / \text{Area}_{\text{section}} = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$M / Vd = 2.00$

$\mu_u = 2.3795529E-012$

$\nu_u = 2.9582284E-031$

$d = 0.8 \cdot h = 600.00$

$N_u = 11016.808$

$A_g = 300000.00$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 621900.694$

where:

$V_{s1} = 523602.964$ is calculated for jacket, with:

$d = 600.00$

$A_v = 157079.633$

$f_y = 555.56$

$s = 100.00$

V_{s1} is multiplied by $\text{Col1} = 1.00$

$s/d = 0.16666667$

$V_{s2} = 98297.73$ is calculated for core, with:

$d = 440.00$

$A_v = 100530.965$

$f_y = 555.56$

$s = 250.00$

V_{s2} is multiplied by $\text{Col2} = 1.00$

$s/d = 0.56818182$

V_f ((11-3)-(11.4), ACI 440) = 463792.00

$f = 0.95$, for fully-wrapped sections

$w_f / s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression, where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, a)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, \theta)|)$, with:

total thickness per orientation, $t_{f1} = N_L \cdot t / \text{NoDir} = 1.00$

$d_{fv} = d$ (figure 11.2, ACI 440) = 707.00

f_{fe} ((11-5), ACI 440) = 328.00

$E_f = 82000.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.009$

From (11-11), ACI 440: $V_s + V_f \leq 915872.391$

$b_w = 400.00$

End Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At local axis: 2

Start Of Calculation of Chord Rotation Capacity for element: column JC1 of floor 1

At local axis: 3

Integration Section: (a)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$

Chord Rotation is generally considered as Deformation-Controlled Action according to Table C7-1, ASCE 41-17.

Mean strengths are used for Deformation-Controlled Actions according to 7.5.1.3, ASCE 41-17

Consequently:

Jacket

· New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$
 · New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$
 · Concrete Elasticity, $E_c = 26999.444$
 · Steel Elasticity, $E_s = 200000.00$
 · Existing Column
 · New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$
 · New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$
 · Concrete Elasticity, $E_c = 26999.444$
 · Steel Elasticity, $E_s = 200000.00$
 · External Height, $H = 400.00$
 · External Width, $W = 750.00$
 · Internal Height, $H = 200.00$
 · Internal Width, $W = 550.00$
 · Cover Thickness, $c = 25.00$
 · Element Length, $L = 3000.00$
 · Primary Member
 · Ribbed Bars
 · Ductile Steel
 · Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
 · Longitudinal Bars With Ends Lapped Starting at the End Sections
 · Inadequate Lap Length with $l_b/l_d = 0.30$
 · FRP Wrapping Data
 · Type: Carbon
 · Dry properties (design values)
 · Thickness, $t = 1.00$
 · Tensile Strength, $f_{fu} = 840.00$
 · Tensile Modulus, $E_f = 82000.00$
 · Elongation, $e_{fu} = 0.009$
 · Number of directions, $N_{oDir} = 1$
 · Fiber orientations, $b_i: 0.00^\circ$
 · Number of layers, $N_L = 1$
 · Radius of rounding corners, $R = 40.00$

 · Stepwise Properties

· Bending Moment, $M = -1.0982E+007$
 · Shear Force, $V_2 = -3112.69$
 · Shear Force, $V_3 = -2.2719624E-013$
 · Axial Force, $F = -15947.314$
 · Longitudinal Reinforcement Area Distribution (in 2 divisions)
 · -Tension: $A_{sl} = 0.00$
 · -Compression: $A_{sl,c} = 2676.637$
 · Longitudinal Reinforcement Area Distribution (in 3 divisions)
 · -Tension: $A_{sl,ten} = 1137.257$
 · -Compression: $A_{sl,com} = 1137.257$
 · -Middle: $A_{sl,mid} = 402.1239$
 · Longitudinal External Reinforcement Area Distribution (in 3 divisions)
 · -Tension: $A_{sl,ten,jacket} = 829.3805$
 · -Compression: $A_{sl,com,jacket} = 829.3805$
 · -Middle: $A_{sl,mid,jacket} = 402.1239$
 · Longitudinal Internal Reinforcement Area Distribution (in 3 divisions)
 · -Tension: $A_{sl,ten,core} = 307.8761$
 · -Compression: $A_{sl,com,core} = 307.8761$
 · -Middle: $A_{sl,mid,core} = 0.00$
 · Mean Diameter of Tension Reinforcement, $DbL = 16.80$

 · New component: From table 7-7, ASCE 41_17: Final chord rotation Capacity $u_{,R} = 1.0^*$ $u = 0.03271107$
 $u = y + p = 0.03271107$

 · - Calculation of y -

 $y = (M_y * L_s / 3) / E_{eff} = 0.00271107 ((4.29), Biskinis Phd)$

$M_y = 2.6257E+008$
 $L_s = M/V$ (with $L_s > 0.1*L$ and $L_s < 2*L$) = 3528.173
 From table 10.5, ASCE 41_17: $E_{eff} = factor * E_c * I_g = 1.1390E+014$
 $factor = 0.30$
 $A_g = 300000.00$
 Mean concrete strength: $f_c' = (f_c'_{jacket} * Area_{jacket} + f_c'_{core} * Area_{core}) / Area_{section} = 33.00$
 $N = 15947.314$
 $E_c * I_g = E_{c,jacket} * I_{g,jacket} + E_{c,core} * I_{g,core} = 3.7968E+014$

Calculation of Yielding Moment M_y

Calculation of y and M_y according to Annex 7 -

$y = \text{Min}(y_{ten}, y_{com})$
 $y_{ten} = 2.8044129E-006$
 with ((10.1), ASCE 41-17) $f_y = \text{Min}(f_y, 1.25 * f_y * (I_b / I_d)^{2/3}) = 311.2112$
 $d = 707.00$
 $y = 0.21519134$
 $A = 0.00964597$
 $B = 0.00520141$
 with $pt = 0.00402142$
 $pc = 0.00402142$
 $pv = 0.00142194$
 $N = 15947.314$
 $b = 400.00$
 $" = 0.06082037$
 $y_{comp} = 1.4823586E-005$
 with f_c^* (12.3, (ACI 440)) = 33.35516
 $f_c = 33.00$
 $f_l = 0.77176471$
 $b = 400.00$
 $h = 750.00$
 $A_g = 300000.00$
 From (12.9), ACI 440: $k_a = 0.14679369$
 $g = pt + pc + pv = 0.00946477$
 $rc = 40.00$
 $A_e / A_c = 0.51607158$
 Effective FRP thickness, $t_f = N_L * t * \text{Cos}(b_1) = 1.00$
 effective strain from (12.5) and (12.12), $e_{fe} = 0.004$
 $f_u = 0.009$
 $E_f = 82000.00$
 $E_c = 26999.444$
 $y = 0.21218177$
 $A = 0.00933798$
 $B = 0.00502021$
 with $E_s = 200000.00$

Calculation of ratio I_b / I_d

Inadequate Lap Length with $I_b / I_d = 0.30$

- Calculation of p -

From table 10-8: $p = 0.03$

with:

- Columns controlled by inadequate development or splicing along the clear height because $I_b / I_d < 1$
 shear control ratio $V_y E / C_o I_{OE} = 0.19671905$
 $d = d_{external} = 707.00$
 $s = s_{external} = 0.00$
 $t = s_1 + s_2 + 2 * t_f / b_w * (f_{fe} / f_s) = 0.00761619$
 jacket: $s_1 = A_v * h_1 / (s_1 * A_g) = 0.00392699$
 $A_v = 157.0796$, is the total area of all stirrups parallel to loading (shear) direction

$$h1 = 750.00$$

$$s1 = 100.00$$

$$\text{core: } s2 = Av2 \cdot h2 / (s2 \cdot Ag) = 0.00073723$$

$Av2 = 100.531$, is the total area of all stirrups parallel to loading (shear) direction

$$h2 = 550.00$$

$$s2 = 250.00$$

The term $2 \cdot tf/bw \cdot (ffe/fs)$ is implemented to account for FRP contribution where $f = 2 \cdot tf/bw$ is FRP ratio (EC8 - 3, A.4.4.3(6)) and ffe/fs normalises f to steel strength. All these variables have already been given in Shear control ratio calculation.

For the normalisation fs of jacket is used.

$$NUD = 15947.314$$

$$Ag = 300000.00$$

$$fcE = (fc_jacket \cdot Area_jacket + fc_core \cdot Area_core) / section_area = 33.00$$

$$fyIE = (fy_ext_Long_Reinf \cdot Area_ext_Long_Reinf + fy_int_Long_Reinf \cdot Area_int_Long_Reinf) / Area_Tot_Long_Rein = 555.56$$

$$fytE = (fy_ext_Trans_Reinf \cdot s1 + fy_int_Trans_Reinf \cdot s2) / (s1 + s2) = 555.56$$

$$pl = Area_Tot_Long_Rein / (b \cdot d) = 0.00946477$$

$$b = 400.00$$

$$d = 707.00$$

$$fcE = 33.00$$

End Of Calculation of Chord Rotation Capacity for element: column JC1 of floor 1

At local axis: 3

Integration Section: (a)

Calculation No. 13

column C1, Floor 1

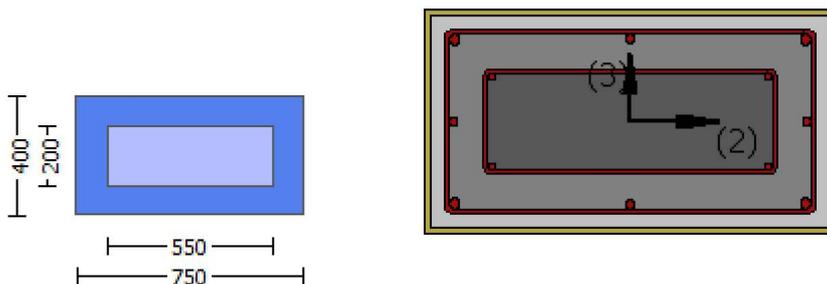
Limit State: Life Safety (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Shear capacity VRd

Edge: End

Local Axis: (2)



```

Start Of Calculation of Shear Capacity for element: column JC1 of floor 1
At local axis: 2
Integration Section: (b)
Section Type: rcjrs

Constant Properties
-----
Knowledge Factor,   = 0.90
Member Shear Force is generally considered as Force-Controlled Action according to Table C7-1, ASCE 41-17.
Lower-bound strengths are used for Force-Controlled Actions according to 7.5.1.3, ASCE 41-17
Consequently:
Jacket
New material of Primary Member: Concrete Strength, fc = fc_lower_bound = 25.00
New material of Primary Member: Steel Strength, fs = fs_lower_bound = 500.00
Concrete Elasticity, Ec = 26999.444
Steel Elasticity, Es = 200000.00
Existing Column
New material of Primary Member: Concrete Strength, fc = fc_lower_bound = 25.00
New material of Primary Member: Steel Strength, fs = fs_lower_bound = 500.00
Concrete Elasticity, Ec = 26999.444
Steel Elasticity, Es = 200000.00
#####
Note: Especially for the calculation of  $\gamma$  for displacement ductility demand,
the expected (mean value) strengths are used (7.5.1.3, ASCE 41-17) because bending is considered as
Deformation-Controlled Action (Table C7-1, ASCE 41-17).
Jacket
New material: Concrete Strength, fc = fcm = 33.00
New material: Steel Strength, fs = fsm = 555.56
Existing Column
New material: Concrete Strength, fc = fcm = 33.00
New material: Steel Strength, fs = fsm = 555.56
#####
External Height, H = 400.00
External Width, W = 750.00
Internal Height, H = 200.00
Internal Width, W = 550.00
Cover Thickness, c = 25.00
Element Length, L = 3000.00
Primary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Inadequate Lap Length with  $l_o/l_{ou,min} = l_b/d = 0.30$ 
FRP Wrapping Data
Type: Carbon
Dry properties (design values)
Thickness, t = 1.00
Tensile Strength,  $f_{fu} = 840.00$ 
Tensile Modulus,  $E_f = 82000.00$ 
Elongation,  $e_{fu} = 0.009$ 
Number of directions, NoDir = 1
Fiber orientations,  $b_i: 0.00^\circ$ 
Number of layers, NL = 1
Radius of rounding corners, R = 40.00
-----

Stepwise Properties
-----
EDGE -A-
Bending Moment,  $M_a = -1.0982E+007$ 
Shear Force,  $V_a = -3112.69$ 
EDGE -B-
Bending Moment,  $M_b = 1.6361E+006$ 
Shear Force,  $V_b = 3112.69$ 
BOTH EDGES
Axial Force,  $F = -15947.314$ 

```

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $As_c = 2676.637$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{t,ten} = 1137.257$

-Compression: $As_{c,com} = 1137.257$

-Middle: $As_{mid} = 402.1239$

Mean Diameter of Tension Reinforcement, $Db_{L,ten} = 16.80$

New component: From table 7-7, ASCE 41_17: Final Shear Capacity $VR = 1.0 \cdot V_n = 1.1003E+006$

V_n ((10.3), ASCE 41-17) = $kn_l \cdot V_{CoI} = 1.1003E+006$

$V_{CoI} = 1.1003E+006$

$kn_l = 1.00$

displacement_ductility_demand = 0.04602302

NOTE: In expression (10-3) ' $V_s = A_v \cdot f_y \cdot d / s$ ' is replaced by ' $V_s + f \cdot V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: $f_c' = (f_c'_{jacket} \cdot Area_{jacket} + f_c'_{core} \cdot Area_{core}) / Area_{section} = 25.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$M/Vd = 2.00$

$M_u = 1.6361E+006$

$V_u = 3112.69$

$d = 0.8 \cdot h = 600.00$

$N_u = 15947.314$

$A_g = 300000.00$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 559706.147$

where:

$V_{s1} = 471238.898$ is calculated for jacket, with:

$d = 600.00$

$A_v = 157079.633$

$f_y = 500.00$

$s = 100.00$

V_{s1} is multiplied by $Col1 = 1.00$

$s/d = 0.16666667$

$V_{s2} = 88467.249$ is calculated for core, with:

$d = 440.00$

$A_v = 100530.965$

$f_y = 500.00$

$s = 250.00$

V_{s2} is multiplied by $Col2 = 1.00$

$s/d = 0.56818182$

V_f ((11-3)-(11.4), ACI 440) = 463792.00

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \csc) \sin \alpha$ which is more a generalised expression,

where α is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\alpha = 45^\circ$ and $\alpha = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\alpha_1 = b_1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \alpha_1)|, |V_f(-45, \alpha_1)|)$, with:

total thickness per orientation, $t_{f1} = NL \cdot t / NoDir = 1.00$

$d_{fv} = d$ (figure 11.2, ACI 440) = 707.00

f_{fe} ((11-5), ACI 440) = 328.00

$E_f = 82000.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.009$

From (11-11), ACI 440: $V_s + V_f \leq 797164.595$

$b_w = 400.00$

displacement_ductility_demand is calculated as δ / y

- Calculation of ϕ_y for END B -
for rotation axis 3 and integ. section (b)

From analysis, chord rotation $\theta = 1.8588678E-005$
 $y = (M_y * L_s / 3) / E_{eff} = 0.0004039$ ((4.29), Biskinis Phd))
 $M_y = 2.6257E+008$
 $L_s = M/V$ (with $L_s > 0.1 * L$ and $L_s < 2 * L$) = 525.6338
From table 10.5, ASCE 41_17: $E_{eff} = factor * E_c * I_g = 1.1390E+014$
 $factor = 0.30$
 $A_g = 300000.00$
Mean concrete strength: $f_c' = (f_c'_{jacket} * Area_{jacket} + f_c'_{core} * Area_{core}) / Area_{section} = 33.00$
 $N = 15947.314$
 $E_c * I_g = E_c_{jacket} * I_{g,jacket} + E_c_{core} * I_{g,core} = 3.7968E+014$

Calculation of Yielding Moment M_y

Calculation of ϕ_y and M_y according to Annex 7 -

 $y = \text{Min}(y_{ten}, y_{com})$
 $y_{ten} = 2.8044129E-006$
with ((10.1), ASCE 41-17) $f_y = \text{Min}(f_y, 1.25 * f_y * (l_b / d)^{2/3}) = 311.2112$
 $d = 707.00$
 $y = 0.21519134$
 $A = 0.00964597$
 $B = 0.00520141$
with $p_t = 0.00402142$
 $p_c = 0.00402142$
 $p_v = 0.00142194$
 $N = 15947.314$
 $b = 400.00$
 $\lambda = 0.06082037$
 $y_{comp} = 1.4823586E-005$
with $f_c' (12.3, (ACI 440)) = 33.35516$
 $f_c = 33.00$
 $f_l = 0.77176471$
 $b = 400.00$
 $h = 750.00$
 $A_g = 300000.00$
From (12.9), ACI 440: $k_a = 0.14679369$
 $g = p_t + p_c + p_v = 0.00946477$
 $r_c = 40.00$
 $A_e / A_c = 0.51607158$
Effective FRP thickness, $t_f = N_L * t * \text{Cos}(\theta) = 1.00$
effective strain from (12.5) and (12.12), $\epsilon_{fe} = 0.004$
 $f_u = 0.009$
 $E_f = 82000.00$
 $E_c = 26999.444$
 $y = 0.21218177$
 $A = 0.00933798$
 $B = 0.00502021$
with $E_s = 200000.00$

Calculation of ratio l_b / d

Inadequate Lap Length with $l_b / d = 0.30$

End Of Calculation of Shear Capacity for element: column JC1 of floor 1

At local axis: 2

Integration Section: (b)

Calculation No. 14

column C1, Floor 1

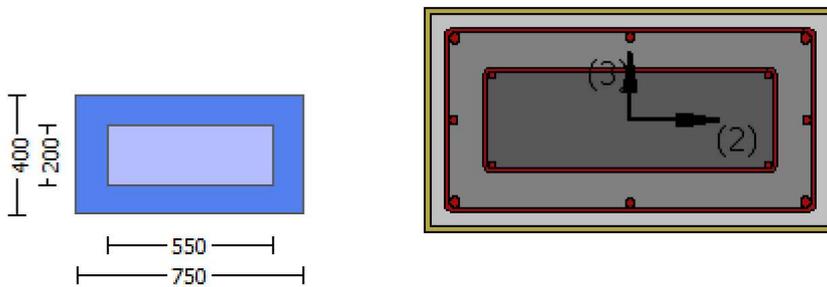
Limit State: Life Safety (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Chord rotation capacity (θ)

Edge: End

Local Axis: (2)



Start Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At Shear local axis: 3

(Bending local axis: 2)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\phi = 0.90$

Mean strength values are used for both shear and moment calculations.

Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Existing Column

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of moment strengths,

the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

Jacket

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

Existing Column

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

#####

External Height, $H = 400.00$

External Width, $W = 750.00$

Internal Height, H = 200.00
Internal Width, W = 550.00
Cover Thickness, c = 25.00
Mean Confinement Factor overall section = 1.0435
Element Length, L = 3000.00
Primary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Inadequate Lap Length with $l_o/l_{o,min}$ = 0.30
FRP Wrapping Data
Type: Carbon
Dry properties (design values)
Thickness, t = 1.00
Tensile Strength, f_{fu} = 840.00
Tensile Modulus, E_f = 82000.00
Elongation, $ε_{fu}$ = 0.009
Number of directions, NoDir = 1
Fiber orientations, $β_i$: 0.00°
Number of layers, NL = 1
Radius of rounding corners, R = 40.00

Stepwise Properties

At local axis: 3
EDGE -A-
Shear Force, V_a = 3.4586346E-031
EDGE -B-
Shear Force, V_b = -3.4586346E-031
BOTH EDGES
Axial Force, F = -11016.808
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: A_{st} = 0.00
-Compression: A_{sc} = 2676.637
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: $A_{st,ten}$ = 1137.257
-Compression: $A_{st,com}$ = 1137.257
-Middle: $A_{st,mid}$ = 402.1239

Calculation of Shear Capacity ratio, V_e/V_r = 0.14265244

Member Controlled by Flexure ($V_e/V_r < 1$)

Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 121055.918$
with

$M_{pr1} = \text{Max}(M_{u1+}, M_{u1-}) = 1.8158E+008$

$M_{u1+} = 1.8158E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination

$M_{u1-} = 1.8158E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination

$M_{pr2} = \text{Max}(M_{u2+}, M_{u2-}) = 1.8158E+008$

$M_{u2+} = 1.8158E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the the static loading combination

$M_{u2-} = 1.8158E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the the static loading combination

Calculation of M_{u1+}

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 1.7255512E-005$

$M_u = 1.8158E+008$

with full section properties:

$$b = 750.00$$

$$d = 357.00$$

$$d' = 43.00$$

$$v = 0.00124684$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$\alpha (5A.5, \text{TBDY}) = 0.002$$

$$\text{Final value of } \alpha: \alpha^* = \text{shear_factor} * \text{Max}(\alpha, \alpha_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \alpha_c = 0.01006403$$

$$\omega_e ((5.4c), \text{TBDY}) = \alpha^* \cdot \text{sh}_{\min} \cdot f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$$

where $f = \alpha^* \cdot p_f \cdot f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.02504965$$

$$\alpha_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/bw = 0.00266667$$

$$bw = 750.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 800.0873$$

$$f_y = 0.04491794$$

$$\alpha_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/bw = 0.005$$

$$bw = 400.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL \cdot t \cdot \text{Cos}(b_1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{,f} = 0.015$$

$$\alpha_{se} ((5.4d), \text{TBDY}) = (\alpha_{se1} \cdot A_{ext} + \alpha_{se2} \cdot A_{int}) / A_{sec} = 0.12601038$$

$$\alpha_{se1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i2,1} = 1.1834E+006$$

$$\alpha_{se2} = \text{Max}(\alpha_{se1}, \alpha_{se2}) = 0.12601038$$

$$b_{o,2} = 542.00$$

$$h_{o,2} = 192.00$$

$$b_{i2,2} = 661256.00$$

$$p_{sh,\min} \cdot F_{ywe} = \text{Min}(p_{sh,x} \cdot F_{ywe}, p_{sh,y} \cdot F_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $p_{sh,\min} \cdot F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$p_{sh,x} \cdot F_{ywe} = p_{sh1} \cdot F_{ywe1} + p_{sh2} \cdot F_{ywe2} = 1.64062$$

$$p_{s1} (\text{external}) = (A_{sh1} \cdot h_1 / s_1) / A_{sec} = 0.0020944$$

$$A_{sh1} = A_{stir,1} \cdot n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

$$h_1 = 400.00$$

$$p_{s2} (\text{internal}) = (A_{sh2} \cdot h_2 / s_2) / A_{sec} = 0.00026808$$

$$A_{sh2} = A_{stir,2} \cdot n_{s,2} = 100.531$$

$$\text{No stirrups, } n_{s,2} = 2.00$$

$$h_2 = 200.00$$

$$p_{sh,y} \cdot F_{ywe} = p_{sh1} \cdot F_{ywe1} + p_{sh2} \cdot F_{ywe2} = 3.23907$$

$$p_{s1} (\text{external}) = (A_{sh1} \cdot h_1 / s_1) / A_{sec} = 0.00392699$$

$$A_{sh1} = A_{stir,1} \cdot n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

$$h_1 = 750.00$$

$$p_{s2} (\text{internal}) = (A_{sh2} \cdot h_2 / s_2) / A_{sec} = 0.00073723$$

$$A_{sh2} = A_{stir,2} \cdot n_{s,2} = 100.531$$

$$\text{No stirrups, } n_{s,2} = 2.00$$

h2 = 550.00

Asec = 300000.00

s1 = 100.00

s2 = 250.00

fywe1 = 694.45

fywe2 = 694.45

fce = 33.00

From ((5.A.5), TBDY), TBDY: cc = 0.00243499

c = confinement factor = 1.0435

y1 = 0.00140044

sh1 = 0.0044814

ft1 = 466.8167

fy1 = 389.0139

su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044

sh2 = 0.0044814

ft2 = 466.8167

fy2 = 389.0139

su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044

shv = 0.0044814

ftv = 466.8167

fyv = 389.0139

suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.0500703

2 = Asl,com/(b*d)*(fs2/fc) = 0.0500703

v = Asl,mid/(b*d)*(fsv/fc) = 0.01770442

and confined core properties:

b = 690.00

d = 327.00

d' = 13.00

fcc (5A.2, TBDY) = 34.43547

$$cc (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = \text{Asl,ten}/(b*d)*(fs1/fc) = 0.05941729$$

$$2 = \text{Asl,com}/(b*d)*(fs2/fc) = 0.05941729$$

$$v = \text{Asl,mid}/(b*d)*(fsv/fc) = 0.02100943$$

Case/Assumption: Unconfined full section - Steel rupture
' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$su (4.9) = 0.16886055$$

$$Mu = MRc (4.14) = 1.8158E+008$$

$$u = su (4.1) = 1.7255512E-005$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of $Mu1$ -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$$u = 1.7255512E-005$$

$$Mu = 1.8158E+008$$

with full section properties:

$$b = 750.00$$

$$d = 357.00$$

$$d' = 43.00$$

$$v = 0.00124684$$

$$N = 11016.808$$

$$fc = 33.00$$

$$co (5A.5, TBDY) = 0.002$$

$$\text{Final value of } cu: cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } cu = 0.01006403$$

$$we ((5.4c), TBDY) = ase * sh_{,min} * fy_{we}/f_{ce} + \text{Min}(fx, fy) = 0.02692906$$

where $f = af * pf * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$fx = 0.02504965$$

$$af = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } pf = 2tf/bw = 0.00266667$$

$$bw = 750.00$$

$$\text{effective stress from (A.35), } ff,e = 800.0873$$

$$fy = 0.04491794$$

$$af = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } pf = 2tf/bw = 0.005$$

$$bw = 400.00$$

$$\text{effective stress from (A.35), } ff,e = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } tf = NL * t * \text{Cos}(b1) = 1.00$$

$$fu,f = 840.00$$

$$Ef = 82000.00$$

$$u,f = 0.015$$

$$\text{ase ((5.4d), TBDY) } = (ase1 * A_{ext} + ase2 * A_{int}) / A_{sec} = 0.12601038$$

$$ase1 = 0.12601038$$

$$bo_1 = 690.00$$

ho_1 = 340.00
bi2_1 = 1.1834E+006
ase2 = Max(ase1,ase2) = 0.12601038
bo_2 = 542.00
ho_2 = 192.00
bi2_2 = 661256.00

psh,min*Fywe = Min(psh,x*Fywe , psh,y*Fywe) = 1.64062

Expression ((5.4d), TBDY) for psh,min*Fywe has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh_x*Fywe = psh1*Fywe1+ps2*Fywe2 = 1.64062
ps1 (external) = (Ash1*h1/s1)/Asec = 0.0020944
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 400.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00026808
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 200.00

psh_y*Fywe = psh1*Fywe1+ps2*Fywe2 = 3.23907
ps1 (external) = (Ash1*h1/s1)/Asec = 0.00392699
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 750.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00073723
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/d = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^ 2/3), from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044
sh2 = 0.0044814
ft2 = 466.8167
fy2 = 389.0139
su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs_2 = (fs_{jacket} \cdot A_{s1,com,jacket} + fs_{core} \cdot A_{s1,com,core}) / A_{s1,com} = 389.0139$

with $Es_2 = (Es_{jacket} \cdot A_{s1,com,jacket} + Es_{core} \cdot A_{s1,com,core}) / A_{s1,com} = 200000.00$

$y_v = 0.00140044$

$sh_v = 0.0044814$

$ft_v = 466.8167$

$fy_v = 389.0139$

$suv = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

$l_o/l_{o,min} = l_b/l_d = 0.30$

$suv = 0.4 \cdot esuv_{nominal} ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esuv_{nominal} = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY

For calculation of $esuv_{nominal}$ and y_v, sh_v, ft_v, fy_v , it is considered

characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fsv = (fs_{jacket} \cdot A_{s,mid,jacket} + fs_{mid} \cdot A_{s,mid,core}) / A_{s,mid} = 389.0139$

with $Es_v = (Es_{jacket} \cdot A_{s,mid,jacket} + Es_{mid} \cdot A_{s,mid,core}) / A_{s,mid} = 200000.00$

$1 = A_{s,ten} / (b \cdot d) \cdot (fs_1 / fc) = 0.0500703$

$2 = A_{s,com} / (b \cdot d) \cdot (fs_2 / fc) = 0.0500703$

$v = A_{s,mid} / (b \cdot d) \cdot (fsv / fc) = 0.01770442$

and confined core properties:

$b = 690.00$

$d = 327.00$

$d' = 13.00$

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

$1 = A_{s,ten} / (b \cdot d) \cdot (fs_1 / fc) = 0.05941729$

$2 = A_{s,com} / (b \cdot d) \cdot (fs_2 / fc) = 0.05941729$

$v = A_{s,mid} / (b \cdot d) \cdot (fsv / fc) = 0.02100943$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

su (4.9) = 0.16886055

$Mu = MRc$ (4.14) = 1.8158E+008

$u = su$ (4.1) = 1.7255512E-005

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of Mu_{2+}

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 1.7255512E-005$

$Mu = 1.8158E+008$

with full section properties:

$b = 750.00$

$d = 357.00$

$d' = 43.00$

$v = 0.00124684$

$N = 11016.808$

$fc = 33.00$

co (5A.5, TBDY) = 0.002

Final value of cu : $cu^* = \text{shear_factor} \cdot \text{Max}(cu, cc) = 0.01006403$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $cu = 0.01006403$

w_e ((5.4c), TBDY) = $ase * sh_{min} * fy_{we} / f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$

where $f = af * pf * ff_e / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.02504965$

$af = 0.38744444$

$b = 750.00$

$h = 400.00$

From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.00266667$

$bw = 750.00$

effective stress from (A.35), $ff_e = 800.0873$

$f_y = 0.04491794$

$af = 0.38744444$

$b = 400.00$

$h = 750.00$

From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.005$

$bw = 400.00$

effective stress from (A.35), $ff_e = 765.1636$

$R = 40.00$

Effective FRP thickness, $tf = NL * t * \text{Cos}(b_1) = 1.00$

$f_{u,f} = 840.00$

$E_f = 82000.00$

$u_{,f} = 0.015$

ase ((5.4d), TBDY) = $(ase_1 * A_{ext} + ase_2 * A_{int}) / A_{sec} = 0.12601038$

$ase_1 = 0.12601038$

$bo_1 = 690.00$

$ho_1 = 340.00$

$bi_{2,1} = 1.1834E+006$

$ase_2 = \text{Max}(ase_1, ase_2) = 0.12601038$

$bo_2 = 542.00$

$ho_2 = 192.00$

$bi_{2,2} = 661256.00$

$psh_{min} * Fy_{we} = \text{Min}(psh_x * Fy_{we}, psh_y * Fy_{we}) = 1.64062$

Expression ((5.4d), TBDY) for $psh_{min} * Fy_{we}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh_x * Fy_{we} = psh_1 * Fy_{we1} + ps_2 * Fy_{we2} = 1.64062$

ps_1 (external) = $(Ash_1 * h_1 / s_1) / A_{sec} = 0.0020944$

$Ash_1 = Astir_1 * ns_1 = 157.0796$

No stirrups, $ns_1 = 2.00$

$h_1 = 400.00$

ps_2 (internal) = $(Ash_2 * h_2 / s_2) / A_{sec} = 0.00026808$

$Ash_2 = Astir_2 * ns_2 = 100.531$

No stirrups, $ns_2 = 2.00$

$h_2 = 200.00$

$psh_y * Fy_{we} = psh_1 * Fy_{we1} + ps_2 * Fy_{we2} = 3.23907$

ps_1 (external) = $(Ash_1 * h_1 / s_1) / A_{sec} = 0.00392699$

$Ash_1 = Astir_1 * ns_1 = 157.0796$

No stirrups, $ns_1 = 2.00$

$h_1 = 750.00$

ps_2 (internal) = $(Ash_2 * h_2 / s_2) / A_{sec} = 0.00073723$

$Ash_2 = Astir_2 * ns_2 = 100.531$

No stirrups, $ns_2 = 2.00$

$h_2 = 550.00$

$A_{sec} = 300000.00$

$s_1 = 100.00$

$s_2 = 250.00$

$fy_{we1} = 694.45$

$fy_{we2} = 694.45$

$f_{ce} = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00243499$

$c =$ confinement factor = 1.0435

$y_1 = 0.00140044$

$sh1 = 0.0044814$
 $ft1 = 466.8167$
 $fy1 = 389.0139$
 $su1 = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $Shear_factor = 1.00$
 $lo/lou,min = lb/ld = 0.30$
 $su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu1_nominal = 0.08$,
 For calculation of $esu1_nominal$ and $y1, sh1,ft1,fy1$, it is considered
 characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.
 $y1, sh1,ft1,fy1$, are also multiplied by $Min(1,1.25*(lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139$
 with $Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00$
 $y2 = 0.00140044$
 $sh2 = 0.0044814$
 $ft2 = 466.8167$
 $fy2 = 389.0139$
 $su2 = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $Shear_factor = 1.00$
 $lo/lou,min = lb/lb,min = 0.30$
 $su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu2_nominal = 0.08$,
 For calculation of $esu2_nominal$ and $y2, sh2,ft2,fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.
 $y1, sh1,ft1,fy1$, are also multiplied by $Min(1,1.25*(lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139$
 with $Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00$
 $yv = 0.00140044$
 $shv = 0.0044814$
 $ftv = 466.8167$
 $fyv = 389.0139$
 $suv = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $Shear_factor = 1.00$
 $lo/lou,min = lb/ld = 0.30$
 $suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_nominal = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv, shv,ftv,fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 $y1, sh1,ft1,fy1$, are also multiplied by $Min(1,1.25*(lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139$
 with $Es_v = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00$
 $1 = Asl,ten/(b*d)*(fs1/fc) = 0.0500703$
 $2 = Asl,com/(b*d)*(fs2/fc) = 0.0500703$
 $v = Asl,mid/(b*d)*(fsv/fc) = 0.01770442$
 and confined core properties:
 $b = 690.00$
 $d = 327.00$
 $d' = 13.00$
 $fcc (5A.2, TBDY) = 34.43547$
 $cc (5A.5, TBDY) = 0.00243499$
 $c = \text{confinement factor} = 1.0435$
 $1 = Asl,ten/(b*d)*(fs1/fc) = 0.05941729$
 $2 = Asl,com/(b*d)*(fs2/fc) = 0.05941729$
 $v = Asl,mid/(b*d)*(fsv/fc) = 0.02100943$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)
 --->
 $v < vs,y2$ - LHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.16886055$

$$\begin{aligned} \mu &= MRC(4.14) = 1.8158E+008 \\ u &= su(4.1) = 1.7255512E-005 \end{aligned}$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of μ_2

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:

$$\begin{aligned} u &= 1.7255512E-005 \\ \mu &= 1.8158E+008 \end{aligned}$$

with full section properties:

$$\begin{aligned} b &= 750.00 \\ d &= 357.00 \\ d' &= 43.00 \\ v &= 0.00124684 \\ N &= 11016.808 \\ f_c &= 33.00 \\ c_o(5A.5, TBDY) &= 0.002 \\ \text{Final value of } \mu: \mu^* &= \text{shear_factor} * \text{Max}(\mu_c, \mu_{cc}) = 0.01006403 \\ \text{The Shear_factor is considered equal to 1 (pure moment strength)} \\ \text{From (5.4b), TBDY: } \mu_c &= 0.01006403 \\ \mu_{we}((5.4c), TBDY) &= a_{se} * s_{h,min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.02692906 \\ \text{where } f &= a_f * p_f * f_{fe}/f_{ce} \text{ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)} \end{aligned}$$

$$\begin{aligned} f_x &= 0.02504965 \\ a_f &= 0.38744444 \\ b &= 750.00 \\ h &= 400.00 \\ \text{From EC8 A.4.4.3(6), } p_f &= 2t_f/b_w = 0.00266667 \\ b_w &= 750.00 \\ \text{effective stress from (A.35), } f_{f,e} &= 800.0873 \end{aligned}$$

$$\begin{aligned} f_y &= 0.04491794 \\ a_f &= 0.38744444 \\ b &= 400.00 \\ h &= 750.00 \\ \text{From EC8 A.4.4.3(6), } p_f &= 2t_f/b_w = 0.005 \\ b_w &= 400.00 \\ \text{effective stress from (A.35), } f_{f,e} &= 765.1636 \end{aligned}$$

$$\begin{aligned} R &= 40.00 \\ \text{Effective FRP thickness, } t_f &= NL * t * \text{Cos}(b_1) = 1.00 \\ f_{u,f} &= 840.00 \\ E_f &= 82000.00 \\ u_{f} &= 0.015 \\ a_{se}((5.4d), TBDY) &= (a_{se1} * A_{ext} + a_{se2} * A_{int}) / A_{sec} = 0.12601038 \\ a_{se1} &= 0.12601038 \\ b_{o_1} &= 690.00 \\ h_{o_1} &= 340.00 \\ b_{i2_1} &= 1.1834E+006 \\ a_{se2} &= \text{Max}(a_{se1}, a_{se2}) = 0.12601038 \\ b_{o_2} &= 542.00 \\ h_{o_2} &= 192.00 \\ b_{i2_2} &= 661256.00 \end{aligned}$$

$p_{sh,min} * F_{ywe} = \text{Min}(p_{sh,x} * F_{ywe}, p_{sh,y} * F_{ywe}) = 1.64062$
Expression ((5.4d), TBDY) for $p_{sh,min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$p_{sh,x} * F_{ywe} = p_{sh1} * F_{ywe1} + p_{sh2} * F_{ywe2} = 1.64062$$

$$ps1 \text{ (external)} = (Ash1 \cdot h1 / s1) / Asec = 0.0020944$$

$$Ash1 = Astir_1 \cdot ns_1 = 157.0796$$

$$\text{No stirups, } ns_1 = 2.00$$

$$h1 = 400.00$$

$$ps2 \text{ (internal)} = (Ash2 \cdot h2 / s2) / Asec = 0.00026808$$

$$Ash2 = Astir_2 \cdot ns_2 = 100.531$$

$$\text{No stirups, } ns_2 = 2.00$$

$$h2 = 200.00$$

$$psh_y \cdot Fywe = psh1 \cdot Fywe1 + ps2 \cdot Fywe2 = 3.23907$$

$$ps1 \text{ (external)} = (Ash1 \cdot h1 / s1) / Asec = 0.00392699$$

$$Ash1 = Astir_1 \cdot ns_1 = 157.0796$$

$$\text{No stirups, } ns_1 = 2.00$$

$$h1 = 750.00$$

$$ps2 \text{ (internal)} = (Ash2 \cdot h2 / s2) / Asec = 0.00073723$$

$$Ash2 = Astir_2 \cdot ns_2 = 100.531$$

$$\text{No stirups, } ns_2 = 2.00$$

$$h2 = 550.00$$

$$Asec = 300000.00$$

$$s1 = 100.00$$

$$s2 = 250.00$$

$$fywe1 = 694.45$$

$$fywe2 = 694.45$$

$$fce = 33.00$$

$$\text{From } ((5.A.5), \text{ TBDY}), \text{ TBDY: } cc = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$y1 = 0.00140044$$

$$sh1 = 0.0044814$$

$$ft1 = 466.8167$$

$$fy1 = 389.0139$$

$$su1 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lou, \text{min} = lb/ld = 0.30$$

$$su1 = 0.4 \cdot esu1_{\text{nominal}} ((5.5), \text{ TBDY}) = 0.032$$

From table 5A.1, TBDY: $esu1_{\text{nominal}} = 0.08$,

For calculation of $esu1_{\text{nominal}}$ and $y1, sh1, ft1, fy1$, it is considered
characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fs1 = (fs_{\text{jacket}} \cdot Asl, \text{ten, jacket} + fs_{\text{core}} \cdot Asl, \text{ten, core}) / Asl, \text{ten} = 389.0139$$

$$\text{with } Es1 = (Es_{\text{jacket}} \cdot Asl, \text{ten, jacket} + Es_{\text{core}} \cdot Asl, \text{ten, core}) / Asl, \text{ten} = 200000.00$$

$$y2 = 0.00140044$$

$$sh2 = 0.0044814$$

$$ft2 = 466.8167$$

$$fy2 = 389.0139$$

$$su2 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lou, \text{min} = lb/lb, \text{min} = 0.30$$

$$su2 = 0.4 \cdot esu2_{\text{nominal}} ((5.5), \text{ TBDY}) = 0.032$$

From table 5A.1, TBDY: $esu2_{\text{nominal}} = 0.08$,

For calculation of $esu2_{\text{nominal}}$ and $y2, sh2, ft2, fy2$, it is considered
characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fs2 = (fs_{\text{jacket}} \cdot Asl, \text{com, jacket} + fs_{\text{core}} \cdot Asl, \text{com, core}) / Asl, \text{com} = 389.0139$$

$$\text{with } Es2 = (Es_{\text{jacket}} \cdot Asl, \text{com, jacket} + Es_{\text{core}} \cdot Asl, \text{com, core}) / Asl, \text{com} = 200000.00$$

$$yv = 0.00140044$$

$$shv = 0.0044814$$

$$ftv = 466.8167$$

$$fyv = 389.0139$$

$$suv = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$l_0/l_{ou,min} = l_b/l_d = 0.30$$

$$s_{uv} = 0.4 * e_{suv_nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $e_{suv_nominal} = 0.08$,

considering characteristic value $f_{sv} = f_{sv}/1.2$, from table 5.1, TBDY

For calculation of $e_{suv_nominal}$ and γ_v , sh_v, ft_v, fy_v , it is considered

characteristic value $f_{sv} = f_{sv}/1.2$, from table 5.1, TBDY.

$$\gamma_1, sh_1, ft_1, fy_1, \text{ are also multiplied by } \text{Min}(1, 1.25 * (l_b/l_d)^{2/3}), \text{ from 10.3.5, ASCE 41-17.}$$

$$\text{with } f_{sv} = (f_{s,jacket} * A_{s,mid,jacket} + f_{s,mid} * A_{s,mid,core}) / A_{s,mid} = 389.0139$$

$$\text{with } E_{sv} = (E_{s,jacket} * A_{s,mid,jacket} + E_{s,mid} * A_{s,mid,core}) / A_{s,mid} = 200000.00$$

$$1 = A_{s,ten} / (b * d) * (f_{s1} / f_c) = 0.0500703$$

$$2 = A_{s,com} / (b * d) * (f_{s2} / f_c) = 0.0500703$$

$$v = A_{s,mid} / (b * d) * (f_{sv} / f_c) = 0.01770442$$

and confined core properties:

$$b = 690.00$$

$$d = 327.00$$

$$d' = 13.00$$

$$f_{cc} (5A.2, TBDY) = 34.43547$$

$$c_c (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = A_{s,ten} / (b * d) * (f_{s1} / f_c) = 0.05941729$$

$$2 = A_{s,com} / (b * d) * (f_{s2} / f_c) = 0.05941729$$

$$v = A_{s,mid} / (b * d) * (f_{sv} / f_c) = 0.02100943$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$s_u (4.9) = 0.16886055$$

$$M_u = M_{Rc} (4.14) = 1.8158E+008$$

$$u = s_u (4.1) = 1.7255512E-005$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 848607.437$

Calculation of Shear Strength at edge 1, $V_{r1} = 848607.437$

$V_{r1} = V_{Col} ((10.3), ASCE 41-17) = k_{nl} * V_{Col0}$

$$V_{Col0} = 848607.437$$

$$k_{nl} = 1 \text{ (zero step-static loading)}$$

NOTE: In expression (10-3) ' $V_s = A_v * f_y * d / s$ ' is replaced by ' $V_{s+} = f * V_f$ '

where V_f is the contribution of FRPs (11.3), ACI 440).

$$= 1 \text{ (normal-weight concrete)}$$

Mean concrete strength: $f_c' = (f_c'_{jacket} * Area_{jacket} + f_c'_{core} * Area_{core}) / Area_{section} = 33.00$, but $f_c'^{0.5} < = 8.3$ MPa (22.5.3.1, ACI 318-14)

$$M/Vd = 2.00$$

$$M_u = 3.5345895E-012$$

$$V_u = 3.4586346E-031$$

$$d = 0.8 * h = 320.00$$

$$N_u = 11016.808$$

$$A_g = 300000.00$$

$$\text{From (11.5.4.8), ACI 318-14: } V_s = V_{s1} + V_{s2} = 279254.914$$

where:

$V_{s1} = 279254.914$ is calculated for jacket, with:

$$d = 320.00$$

$$A_v = 157079.633$$

$$f_y = 555.56$$

$$s = 100.00$$

V_{s1} is multiplied by $Col1 = 1.00$

$$s/d = 0.3125$$

Vs2 = 0.00 is calculated for core, with:

$$d = 160.00$$

$$A_v = 100530.965$$

$$f_y = 555.56$$

$$s = 250.00$$

Vs2 is multiplied by Col2 = 0.00

$$s/d = 1.5625$$

Vf ((11-3)-(11.4), ACI 440) = 234192.00

$$f = 0.95, \text{ for fully-wrapped sections}$$

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) $\sin^2 + \cos^2$ is replaced with $(\cot^2 + \csc^2)\sin^2\alpha$ which is more a generalised expression,

where α is the angle of the crack direction (see KANEPE).

This later relation, considered as a function Vf(α), is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\alpha = 45^\circ$ and $\alpha = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\alpha = b_1 + 90^\circ = 90.00$

Vf = Min(|Vf(45, α)|, |Vf(-45, α)|), with:

total thickness per orientation, $t_{f1} = N_L * t / N_{oDir} = 1.00$

dfv = d (figure 11.2, ACI 440) = 357.00

ffe ((11-5), ACI 440) = 328.00

$$E_f = 82000.00$$

$$f_e = 0.004, \text{ from (11.6a), ACI 440}$$

$$\text{with } f_u = 0.009$$

From (11-11), ACI 440: Vs + Vf <= 915872.391

$$b_w = 750.00$$

Calculation of Shear Strength at edge 2, Vr2 = 848607.437

Vr2 = VCol ((10.3), ASCE 41-17) = knl * VCol0

$$V_{Col0} = 848607.437$$

knl = 1 (zero step-static loading)

NOTE: In expression (10-3) 'Vs = Av*fy*d/s' is replaced by 'Vs+ f*Vf' where Vf is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: $f_c' = (f_c'_{jacket} * Area_{jacket} + f_c'_{core} * Area_{core}) / Area_{section} = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$$M/Vd = 2.00$$

$$\mu_u = 3.5345895E-012$$

$$V_u = 3.4586346E-031$$

$$d = 0.8 * h = 320.00$$

$$N_u = 11016.808$$

$$A_g = 300000.00$$

From (11.5.4.8), ACI 318-14: Vs = Vs1 + Vs2 = 279254.914

where:

Vs1 = 279254.914 is calculated for jacket, with:

$$d = 320.00$$

$$A_v = 157079.633$$

$$f_y = 555.56$$

$$s = 100.00$$

Vs1 is multiplied by Col1 = 1.00

$$s/d = 0.3125$$

Vs2 = 0.00 is calculated for core, with:

$$d = 160.00$$

$$A_v = 100530.965$$

$$f_y = 555.56$$

$$s = 250.00$$

Vs2 is multiplied by Col2 = 0.00

$$s/d = 1.5625$$

Vf ((11-3)-(11.4), ACI 440) = 234192.00

$$f = 0.95, \text{ for fully-wrapped sections}$$

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) $\sin^2 + \cos^2$ is replaced with $(\cot^2 + \csc^2)\sin^2\alpha$ which is more a generalised expression, where α is the angle of the crack direction (see KANEPE).

This later relation, considered as a function Vf(α), is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\alpha = 45^\circ$ and $\alpha = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\alpha = b_1 + 90^\circ = 90.00$

Vf = Min(|Vf(45, 1)|,|Vf(-45,a1)|), with:
total thickness per orientation, tf1 = NL*t/NoDir = 1.00
dfv = d (figure 11.2, ACI 440) = 357.00
ffe ((11-5), ACI 440) = 328.00
Ef = 82000.00
fe = 0.004, from (11.6a), ACI 440
with fu = 0.009
From (11-11), ACI 440: Vs + Vf <= 915872.391
bw = 750.00

End Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1
At local axis: 3

Start Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1
At Shear local axis: 2
(Bending local axis: 3)
Section Type: rcjrs

Constant Properties

Knowledge Factor, = 0.90
Mean strength values are used for both shear and moment calculations.
Consequently:

Jacket

New material of Primary Member: Concrete Strength, fc = fcm = 33.00
New material of Primary Member: Steel Strength, fs = fsm = 555.56
Concrete Elasticity, Ec = 26999.444
Steel Elasticity, Es = 200000.00

Existing Column

New material of Primary Member: Concrete Strength, fc = fcm = 33.00
New material of Primary Member: Steel Strength, fs = fsm = 555.56
Concrete Elasticity, Ec = 26999.444
Steel Elasticity, Es = 200000.00

#####

Note: Especially for the calculation of moment strengths,
the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

Jacket

New material: Steel Strength, fs = 1.25*fsm = 694.45

Existing Column

New material: Steel Strength, fs = 1.25*fsm = 694.45

#####

External Height, H = 400.00

External Width, W = 750.00

Internal Height, H = 200.00

Internal Width, W = 550.00

Cover Thickness, c = 25.00

Mean Confinement Factor overall section = 1.0435

Element Length, L = 3000.00

Primary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Inadequate Lap Length with lo/lo,min = 0.30

FRP Wrapping Data

Type: Carbon

Dry properties (design values)

Thickness, t = 1.00

Tensile Strength, ffu = 840.00

Tensile Modulus, Ef = 82000.00

Elongation, efu = 0.009

Number of directions, NoDir = 1

Fiber orientations, bi: 0.00°

Number of layers, NL = 1
Radius of rounding corners, R = 40.00

Stepwise Properties

At local axis: 2
EDGE -A-
Shear Force, Va = 2.9582284E-031
EDGE -B-
Shear Force, Vb = -2.9582284E-031
BOTH EDGES
Axial Force, F = -11016.808
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: Aslt = 0.00
-Compression: Aslc = 2676.637
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: Asl,ten = 1137.257
-Compression: Asl,com = 1137.257
-Middle: Asl,mid = 402.1239

Calculation of Shear Capacity ratio , $V_e/V_r = 0.19671905$
Member Controlled by Flexure ($V_e/V_r < 1$)
Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 248405.514$
with
 $M_{pr1} = \text{Max}(\mu_{1+}, \mu_{1-}) = 3.7261E+008$
 $\mu_{1+} = 3.7261E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction
which is defined for the static loading combination
 $\mu_{1-} = 3.7261E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment
direction which is defined for the static loading combination
 $M_{pr2} = \text{Max}(\mu_{2+}, \mu_{2-}) = 3.7261E+008$
 $\mu_{2+} = 3.7261E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction
which is defined for the the static loading combination
 $\mu_{2-} = 3.7261E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment
direction which is defined for the the static loading combination

Calculation of μ_{1+}

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:
 $\mu = 8.5888248E-006$
 $M_u = 3.7261E+008$

with full section properties:

$$b = 400.00$$

$$d = 707.00$$

$$d' = 43.00$$

$$v = 0.00118049$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$c_o \text{ (5A.5, TBDY)} = 0.002$$

$$\text{Final value of } \mu_c: \mu_c^* = \text{shear_factor} * \text{Max}(\mu_c, c_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \mu_c = 0.01006403$$

$$\mu_e \text{ ((5.4c), TBDY)} = a_s e^* s_{h,\min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$$

where $f = a_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.02504965$$

$$a_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.00266667$$

bw = 750.00
effective stress from (A.35), $f_{f,e} = 800.0873$

fy = 0.04491794
af = 0.38744444
b = 400.00
h = 750.00
From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.005$
bw = 400.00
effective stress from (A.35), $f_{f,e} = 765.1636$

R = 40.00
Effective FRP thickness, $tf = NL*t*Cos(b1) = 1.00$
fu,f = 840.00
Ef = 82000.00
u,f = 0.015
ase ((5.4d), TBDY) = $(ase1*Aext+ase2*Aint)/Asec = 0.12601038$
ase1 = 0.12601038
bo_1 = 690.00
ho_1 = 340.00
bi2_1 = 1.1834E+006
ase2 = $Max(ase1,ase2) = 0.12601038$
bo_2 = 542.00
ho_2 = 192.00
bi2_2 = 661256.00

$psh_{min}*Fywe = Min(psh_x*Fywe, psh_y*Fywe) = 1.64062$

Expression ((5.4d), TBDY) for $psh_{min}*Fywe$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh_x*Fywe = psh1*Fywe1+ps2*Fywe2 = 1.64062$
ps1 (external) = $(Ash1*h1/s1)/Asec = 0.0020944$
Ash1 = $Astir_1*ns_1 = 157.0796$
No stirrups, $ns_1 = 2.00$
h1 = 400.00
ps2 (internal) = $(Ash2*h2/s2)/Asec = 0.00026808$
Ash2 = $Astir_2*ns_2 = 100.531$
No stirrups, $ns_2 = 2.00$
h2 = 200.00

$psh_y*Fywe = psh1*Fywe1+ps2*Fywe2 = 3.23907$
ps1 (external) = $(Ash1*h1/s1)/Asec = 0.00392699$
Ash1 = $Astir_1*ns_1 = 157.0796$
No stirrups, $ns_1 = 2.00$
h1 = 750.00
ps2 (internal) = $(Ash2*h2/s2)/Asec = 0.00073723$
Ash2 = $Astir_2*ns_2 = 100.531$
No stirrups, $ns_2 = 2.00$
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: $cc = 0.00243499$
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
 $lo/lou_{min} = lb/ld = 0.30$
 $su1 = 0.4*esu1_{nominal} ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = (fs_jacket \cdot Asl,ten,jacket + fs_core \cdot Asl,ten,core) / Asl,ten = 389.0139$

with $Es1 = (Es_jacket \cdot Asl,ten,jacket + Es_core \cdot Asl,ten,core) / Asl,ten = 200000.00$

$y2 = 0.00140044$

$sh2 = 0.0044814$

$ft2 = 466.8167$

$fy2 = 389.0139$

$su2 = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with $shear_factor$

and also multiplied by the $shear_factor$ according to 15.7.1.4, with

$Shear_factor = 1.00$

$lo/lou,min = lb/lb,min = 0.30$

$su2 = 0.4 \cdot esu2_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = (fs_jacket \cdot Asl,com,jacket + fs_core \cdot Asl,com,core) / Asl,com = 389.0139$

with $Es2 = (Es_jacket \cdot Asl,com,jacket + Es_core \cdot Asl,com,core) / Asl,com = 200000.00$

$yv = 0.00140044$

$shv = 0.0044814$

$ftv = 466.8167$

$fyv = 389.0139$

$suv = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with $shear_factor$

and also multiplied by the $shear_factor$ according to 15.7.1.4, with

$Shear_factor = 1.00$

$lo/lou,min = lb/ld = 0.30$

$suv = 0.4 \cdot esuv_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esuv_nominal = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY

For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered

characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fsv = (fs_jacket \cdot Asl,mid,jacket + fs_mid \cdot Asl,mid,core) / Asl,mid = 389.0139$

with $Esv = (Es_jacket \cdot Asl,mid,jacket + Es_mid \cdot Asl,mid,core) / Asl,mid = 200000.00$

$1 = Asl,ten / (b \cdot d) \cdot (fs1 / fc) = 0.04740567$

$2 = Asl,com / (b \cdot d) \cdot (fs2 / fc) = 0.04740567$

$v = Asl,mid / (b \cdot d) \cdot (fsv / fc) = 0.01676222$

and confined core properties:

$b = 340.00$

$d = 677.00$

$d' = 13.00$

$fcc (5A.2, TBDY) = 34.43547$

$cc (5A.5, TBDY) = 0.00243499$

$c = \text{confinement factor} = 1.0435$

$1 = Asl,ten / (b \cdot d) \cdot (fs1 / fc) = 0.05824278$

$2 = Asl,com / (b \cdot d) \cdot (fs2 / fc) = 0.05824278$

$v = Asl,mid / (b \cdot d) \cdot (fsv / fc) = 0.02059413$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < vs,y2$ - LHS eq.(4.5) is satisfied

--->

$su (4.9) = 0.15682678$

$Mu = MRc (4.14) = 3.7261E+008$

$u = su (4.1) = 8.5888248E-006$

Calculation of ratio lb/ld

Inadequate Lap Length with $lb/ld = 0.30$

Calculation of Mu1-

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:

$$\mu = 8.5888248E-006$$

$$\text{Mu} = 3.7261E+008$$

with full section properties:

$$b = 400.00$$

$$d = 707.00$$

$$d' = 43.00$$

$$v = 0.00118049$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$c_o (5A.5, \text{TBDY}) = 0.002$$

$$\text{Final value of } c_u: c_u^* = \text{shear_factor} * \text{Max}(c_u, c_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } c_u = 0.01006403$$

$$w_e ((5.4c), \text{TBDY}) = a_{se} * s_{h,\min} * f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$$

where $f = a_f * p_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.02504965$$

$$a_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f / b_w = 0.00266667$$

$$b_w = 750.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 800.0873$$

$$f_y = 0.04491794$$

$$a_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f / b_w = 0.005$$

$$b_w = 400.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L * t * \text{Cos}(b_1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{,f} = 0.015$$

$$a_{se} ((5.4d), \text{TBDY}) = (a_{se1} * A_{ext} + a_{se2} * A_{int}) / A_{sec} = 0.12601038$$

$$a_{se1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i,2,1} = 1.1834E+006$$

$$a_{se2} = \text{Max}(a_{se1}, a_{se2}) = 0.12601038$$

$$b_{o,2} = 542.00$$

$$h_{o,2} = 192.00$$

$$b_{i,2,2} = 661256.00$$

$$p_{sh,\min} * F_{ywe} = \text{Min}(p_{sh,x} * F_{ywe}, p_{sh,y} * F_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $p_{sh,\min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$p_{sh,x} * F_{ywe} = p_{sh1} * F_{ywe1} + p_{sh2} * F_{ywe2} = 1.64062$$

$$p_{sh1} (\text{external}) = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$$

$$A_{sh1} = A_{stir,1} * n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

$$h_1 = 400.00$$

$$p_{sh2} (\text{internal}) = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00026808$$

$$A_{sh2} = A_{stir,2} * n_{s,2} = 100.531$$

$$\text{No stirrups, } n_{s,2} = 2.00$$

$$h_2 = 200.00$$

psh_y*Fywe = psh₁*Fywe₁+ps₂*Fywe₂ = 3.23907
ps₁ (external) = (Ash₁*h₁/s₁)/Asec = 0.00392699
Ash₁ = Astir₁*ns₁ = 157.0796
No stirups, ns₁ = 2.00
h₁ = 750.00
ps₂ (internal) = (Ash₂*h₂/s₂)/Asec = 0.00073723
Ash₂ = Astir₂*ns₂ = 100.531
No stirups, ns₂ = 2.00
h₂ = 550.00

Asec = 300000.00

s₁ = 100.00

s₂ = 250.00

fywe₁ = 694.45

fywe₂ = 694.45

fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499

c = confinement factor = 1.0435

y₁ = 0.00140044

sh₁ = 0.0044814

ft₁ = 466.8167

fy₁ = 389.0139

su₁ = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

su₁ = 0.4*esu_{1_nominal} ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu_{1_nominal} = 0.08,

For calculation of esu_{1_nominal} and y₁, sh₁,ft₁,fy₁, it is considered
characteristic value fsy₁ = fs₁/1.2, from table 5.1, TBDY.

y₁, sh₁,ft₁,fy₁, are also multiplied by Min(1,1.25*(lb/ld)^{2/3}), from 10.3.5, ASCE 41-17.

with fs₁ = (fs_{jacket}*Asl_{ten,jacket} + fs_{core}*Asl_{ten,core})/Asl_{ten} = 389.0139

with Es₁ = (Es_{jacket}*Asl_{ten,jacket} + Es_{core}*Asl_{ten,core})/Asl_{ten} = 200000.00

y₂ = 0.00140044

sh₂ = 0.0044814

ft₂ = 466.8167

fy₂ = 389.0139

su₂ = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su₂ = 0.4*esu_{2_nominal} ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu_{2_nominal} = 0.08,

For calculation of esu_{2_nominal} and y₂, sh₂,ft₂,fy₂, it is considered
characteristic value fsy₂ = fs₂/1.2, from table 5.1, TBDY.

y₂, sh₂,ft₂,fy₂, are also multiplied by Min(1,1.25*(lb/ld)^{2/3}), from 10.3.5, ASCE 41-17.

with fs₂ = (fs_{jacket}*Asl_{com,jacket} + fs_{core}*Asl_{com,core})/Asl_{com} = 389.0139

with Es₂ = (Es_{jacket}*Asl_{com,jacket} + Es_{core}*Asl_{com,core})/Asl_{com} = 200000.00

y_v = 0.00140044

sh_v = 0.0044814

ft_v = 466.8167

fy_v = 389.0139

suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

suv = 0.4*esuv_{nominal} ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_{nominal} = 0.08,

considering characteristic value fsy_v = fs_v/1.2, from table 5.1, TBDY

For calculation of esuv_{nominal} and y_v, sh_v,ft_v,fy_v, it is considered
characteristic value fsy_v = fs_v/1.2, from table 5.1, TBDY.

y₁, sh₁,ft₁,fy₁, are also multiplied by Min(1,1.25*(lb/ld)^{2/3}), from 10.3.5, ASCE 41-17.

with fs_v = (fs_{jacket}*Asl_{mid,jacket} + fs_{mid}*Asl_{mid,core})/Asl_{mid} = 389.0139

with Es_v = (Es_{jacket}*Asl_{mid,jacket} + Es_{mid}*Asl_{mid,core})/Asl_{mid} = 200000.00

$$1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.04740567$$

$$2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.04740567$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.01676222$$

and confined core properties:

$$b = 340.00$$

$$d = 677.00$$

$$d' = 13.00$$

$$f_{cc} (5A.2, TBDY) = 34.43547$$

$$c_c (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.05824278$$

$$2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.05824278$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.02059413$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$s_u (4.9) = 0.15682678$$

$$\mu_u = M R_c (4.14) = 3.7261E+008$$

$$u = s_u (4.1) = 8.5888248E-006$$

Calculation of ratio l_b/d

Inadequate Lap Length with $l_b/d = 0.30$

Calculation of μ_{u2+}

Calculation of ultimate curvature μ_u according to 4.1, Biskinis/Fardis 2013:

$$u = 8.5888248E-006$$

$$\mu_u = 3.7261E+008$$

with full section properties:

$$b = 400.00$$

$$d = 707.00$$

$$d' = 43.00$$

$$v = 0.00118049$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$c_c (5A.5, TBDY) = 0.002$$

$$\text{Final value of } c_u: c_u^* = \text{shear_factor} * \text{Max}(c_u, c_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } c_u = 0.01006403$$

$$w_e ((5.4c), TBDY) = a_{se} * s_{h,min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$$

where $f = a_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.02504965$$

$$a_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.00266667$$

$$b_w = 750.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 800.0873$$

$$f_y = 0.04491794$$

$$a_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.005$$

$$b_w = 400.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 765.1636$$

$R = 40.00$
 Effective FRP thickness, $t_f = NL * t * \cos(b_1) = 1.00$
 $f_{u,f} = 840.00$
 $E_f = 82000.00$
 $u_{,f} = 0.015$
 $ase((5.4d), TBDY) = (ase_1 * A_{ext} + ase_2 * A_{int}) / A_{sec} = 0.12601038$
 $ase_1 = 0.12601038$
 $bo_1 = 690.00$
 $ho_1 = 340.00$
 $bi_{2,1} = 1.1834E+006$
 $ase_2 = \text{Max}(ase_1, ase_2) = 0.12601038$
 $bo_2 = 542.00$
 $ho_2 = 192.00$
 $bi_{2,2} = 661256.00$

$psh_{,min} * F_{ywe} = \text{Min}(psh_{,x} * F_{ywe}, psh_{,y} * F_{ywe}) = 1.64062$
 Expression ((5.4d), TBDY) for $psh_{,min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh_{,x} * F_{ywe} = psh_1 * F_{ywe1} + ps_2 * F_{ywe2} = 1.64062$
 $ps_1 \text{ (external)} = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$
 $A_{sh1} = A_{stir_1} * ns_1 = 157.0796$
 No stirrups, $ns_1 = 2.00$
 $h_1 = 400.00$
 $ps_2 \text{ (internal)} = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00026808$
 $A_{sh2} = A_{stir_2} * ns_2 = 100.531$
 No stirrups, $ns_2 = 2.00$
 $h_2 = 200.00$

$psh_{,y} * F_{ywe} = psh_1 * F_{ywe1} + ps_2 * F_{ywe2} = 3.23907$
 $ps_1 \text{ (external)} = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.00392699$
 $A_{sh1} = A_{stir_1} * ns_1 = 157.0796$
 No stirrups, $ns_1 = 2.00$
 $h_1 = 750.00$
 $ps_2 \text{ (internal)} = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00073723$
 $A_{sh2} = A_{stir_2} * ns_2 = 100.531$
 No stirrups, $ns_2 = 2.00$
 $h_2 = 550.00$

$A_{sec} = 300000.00$
 $s_1 = 100.00$
 $s_2 = 250.00$
 $f_{ywe1} = 694.45$
 $f_{ywe2} = 694.45$
 $f_{ce} = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00243499$
 $c = \text{confinement factor} = 1.0435$

$y_1 = 0.00140044$
 $sh_1 = 0.0044814$
 $ft_1 = 466.8167$
 $fy_1 = 389.0139$
 $su_1 = 0.00512$

using (30) in Bisikinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lo_{,min} = lb/ld = 0.30$
 $su_1 = 0.4 * esu_{1,nominal}((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu_{1,nominal} = 0.08$,

For calculation of $esu_{1,nominal}$ and y_1, sh_1, ft_1, fy_1 , it is considered
 characteristic value $fsy_1 = fs_1 / 1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs_1 = (f_{s,jacket} * A_{sl,ten,jacket} + f_{s,core} * A_{sl,ten,core}) / A_{sl,ten} = 389.0139$

with $Es_1 = (E_{s,jacket} * A_{sl,ten,jacket} + E_{s,core} * A_{sl,ten,core}) / A_{sl,ten} = 200000.00$

$y_2 = 0.00140044$
 $sh_2 = 0.0044814$
 $ft_2 = 466.8167$
 $fy_2 = 389.0139$
 $su_2 = 0.00512$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$l_o/l_{o,u,min} = l_b/l_{b,min} = 0.30$$

$$s_u2 = 0.4 * e_{su2,nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $e_{su2,nominal} = 0.08$,

For calculation of $e_{su2,nominal}$ and y_2, sh_2, ft_2, fy_2 , it is considered
characteristic value $f_{sy2} = f_{s2}/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } f_{s2} = (f_{s,jacket} * A_{s1,com,jacket} + f_{s,core} * A_{s1,com,core}) / A_{s1,com} = 389.0139$$

$$\text{with } E_{s2} = (E_{s,jacket} * A_{s1,com,jacket} + E_{s,core} * A_{s1,com,core}) / A_{s1,com} = 200000.00$$

$$y_v = 0.00140044$$

$$sh_v = 0.0044814$$

$$ft_v = 466.8167$$

$$fy_v = 389.0139$$

$$s_{uv} = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$l_o/l_{o,u,min} = l_b/l_d = 0.30$$

$$s_{uv} = 0.4 * e_{suv,nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $e_{suv,nominal} = 0.08$,

considering characteristic value $f_{sv} = f_{sv}/1.2$, from table 5.1, TBDY

For calculation of $e_{suv,nominal}$ and y_v, sh_v, ft_v, fy_v , it is considered
characteristic value $f_{sv} = f_{sv}/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } f_{sv} = (f_{s,jacket} * A_{s1,mid,jacket} + f_{s,mid} * A_{s1,mid,core}) / A_{s1,mid} = 389.0139$$

$$\text{with } E_{sv} = (E_{s,jacket} * A_{s1,mid,jacket} + E_{s,mid} * A_{s1,mid,core}) / A_{s1,mid} = 200000.00$$

$$1 = A_{s1,ten} / (b * d) * (f_{s1} / f_c) = 0.04740567$$

$$2 = A_{s1,com} / (b * d) * (f_{s2} / f_c) = 0.04740567$$

$$v = A_{s1,mid} / (b * d) * (f_{sv} / f_c) = 0.01676222$$

and confined core properties:

$$b = 340.00$$

$$d = 677.00$$

$$d' = 13.00$$

$$f_{cc} (5A.2, TBDY) = 34.43547$$

$$cc (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = A_{s1,ten} / (b * d) * (f_{s1} / f_c) = 0.05824278$$

$$2 = A_{s1,com} / (b * d) * (f_{s2} / f_c) = 0.05824278$$

$$v = A_{s1,mid} / (b * d) * (f_{sv} / f_c) = 0.02059413$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$s_u (4.9) = 0.15682678$$

$$\mu_u = M_{Rc} (4.14) = 3.7261E+008$$

$$u = s_u (4.1) = 8.5888248E-006$$

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of μ_u -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$$u = 8.5888248E-006$$

$$\mu_u = 3.7261E+008$$

with full section properties:

$$b = 400.00$$

$$d = 707.00$$

$$d' = 43.00$$

$$v = 0.00118049$$

$$N = 11016.808$$

$$fc = 33.00$$

$$co (5A.5, TBDY) = 0.002$$

$$\text{Final value of } cu: cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } cu = 0.01006403$$

$$\text{we ((5.4c), TBDY) } = ase^* sh_{,min} * fy_{we} / f_{ce} + \text{Min}(fx, fy) = 0.02692906$$

where $f = af * pf * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$fx = 0.02504965$$

$$af = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } pf = 2tf/bw = 0.00266667$$

$$bw = 750.00$$

$$\text{effective stress from (A.35), } ff_{,e} = 800.0873$$

$$fy = 0.04491794$$

$$af = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } pf = 2tf/bw = 0.005$$

$$bw = 400.00$$

$$\text{effective stress from (A.35), } ff_{,e} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } tf = NL * t * \text{Cos}(b1) = 1.00$$

$$fu_{,f} = 840.00$$

$$Ef = 82000.00$$

$$u_{,f} = 0.015$$

$$ase \text{ ((5.4d), TBDY) } = (ase1 * A_{ext} + ase2 * A_{int}) / A_{sec} = 0.12601038$$

$$ase1 = 0.12601038$$

$$bo_{,1} = 690.00$$

$$ho_{,1} = 340.00$$

$$bi2_{,1} = 1.1834E+006$$

$$ase2 = \text{Max}(ase1, ase2) = 0.12601038$$

$$bo_{,2} = 542.00$$

$$ho_{,2} = 192.00$$

$$bi2_{,2} = 661256.00$$

$$psh_{,min} * Fy_{we} = \text{Min}(psh_{,x} * Fy_{we}, psh_{,y} * Fy_{we}) = 1.64062$$

Expression ((5.4d), TBDY) for $psh_{,min} * Fy_{we}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$psh_{,x} * Fy_{we} = psh1 * Fy_{we1} + ps2 * Fy_{we2} = 1.64062$$

$$ps1 \text{ (external) } = (Ash1 * h1 / s1) / A_{sec} = 0.0020944$$

$$Ash1 = Astir_{,1} * ns_{,1} = 157.0796$$

$$\text{No stirrups, } ns_{,1} = 2.00$$

$$h1 = 400.00$$

$$ps2 \text{ (internal) } = (Ash2 * h2 / s2) / A_{sec} = 0.00026808$$

$$Ash2 = Astir_{,2} * ns_{,2} = 100.531$$

$$\text{No stirrups, } ns_{,2} = 2.00$$

$$h2 = 200.00$$

$$psh_{,y} * Fy_{we} = psh1 * Fy_{we1} + ps2 * Fy_{we2} = 3.23907$$

$$ps1 \text{ (external) } = (Ash1 * h1 / s1) / A_{sec} = 0.00392699$$

$$Ash1 = Astir_{,1} * ns_{,1} = 157.0796$$

$$\text{No stirrups, } ns_{,1} = 2.00$$

$$h1 = 750.00$$

$$ps2 \text{ (internal) } = (Ash2 * h2 / s2) / A_{sec} = 0.00073723$$

$$Ash2 = Astir_{,2} * ns_{,2} = 100.531$$

$$\text{No stirrups, } ns_{,2} = 2.00$$

$$h2 = 550.00$$

$$A_{sec} = 300000.00$$

s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A.5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044
sh2 = 0.0044814
ft2 = 466.8167
fy2 = 389.0139
su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044
shv = 0.0044814
ftv = 466.8167
fyv = 389.0139
suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.04740567

2 = Asl,com/(b*d)*(fs2/fc) = 0.04740567

v = Asl,mid/(b*d)*(fsv/fc) = 0.01676222

and confined core properties:

b = 340.00
d = 677.00
d' = 13.00

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

1 = Asl,ten/(b*d)*(fs1/fc) = 0.05824278

$$2 = A_{sl,com}/(b*d)*(f_s2/f_c) = 0.05824278$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.02059413$$

Case/Assumption: Unconfined full section - Steel rupture
' satisfies Eq. (4.3)

---->
v < v_{s,y2} - LHS eq.(4.5) is satisfied

---->
su (4.9) = 0.15682678
Mu = MRc (4.14) = 3.7261E+008
u = su (4.1) = 8.5888248E-006

Calculation of ratio lb/ld

Inadequate Lap Length with lb/ld = 0.30

Calculation of Shear Strength Vr = Min(Vr1,Vr2) = 1.2627E+006

Calculation of Shear Strength at edge 1, Vr1 = 1.2627E+006

Vr1 = VCol ((10.3), ASCE 41-17) = knl*VColO

$$V_{ColO} = 1.2627E+006$$

knl = 1 (zero step-static loading)

NOTE: In expression (10-3) 'Vs = Av*fy*d/s' is replaced by 'Vs+ f*Vf'
where Vf is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: fc' = (fc'_jacket*Area_jacket + fc'_core*Area_core)/Area_section = 33.00, but fc'^0.5 <= 8.3
MPa (22.5.3.1, ACI 318-14)

$$M/Vd = 2.00$$

$$Mu = 2.3795529E-012$$

$$Vu = 2.9582284E-031$$

$$d = 0.8*h = 600.00$$

$$Nu = 11016.808$$

$$Ag = 300000.00$$

$$\text{From (11.5.4.8), ACI 318-14: } V_s = V_{s1} + V_{s2} = 621900.694$$

where:

Vs1 = 523602.964 is calculated for jacket, with:

$$d = 600.00$$

$$A_v = 157079.633$$

$$f_y = 555.56$$

$$s = 100.00$$

Vs1 is multiplied by Col1 = 1.00

$$s/d = 0.16666667$$

Vs2 = 98297.73 is calculated for core, with:

$$d = 440.00$$

$$A_v = 100530.965$$

$$f_y = 555.56$$

$$s = 250.00$$

Vs2 is multiplied by Col2 = 1.00

$$s/d = 0.56818182$$

Vf ((11-3)-(11.4), ACI 440) = 463792.00

f = 0.95, for fully-wrapped sections

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) sin + cos is replaced with (cot + cota)sina which is more a generalised expression,
where is the angle of the crack direction (see KANEPE).

This later relation, considered as a function Vf(,), is implemented for every different fiber orientation ai,
as well as for 2 crack directions, = 45° and = -45° to take into consideration the cyclic seismic loading.

orientation 1: 1 = b1 + 90° = 90.00

Vf = Min(|Vf(45, 1)|, |Vf(-45,a1)|), with:

total thickness per orientation, tf1 = NL*t/NoDir = 1.00

dfv = d (figure 11.2, ACI 440) = 707.00

ffe ((11-5), ACI 440) = 328.00

$$E_f = 82000.00$$

$f_e = 0.004$, from (11.6a), ACI 440
with $f_u = 0.009$
From (11-11), ACI 440: $V_s + V_f \leq 915872.391$
 $bw = 400.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 1.2627E+006$
 $V_{r2} = V_{Col} ((10.3), ASCE 41-17) = knl * V_{Col0}$
 $V_{Col0} = 1.2627E+006$
 $knl = 1$ (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v * f_y * d / s$ ' is replaced by ' $V_s + f * V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

 $= 1$ (normal-weight concrete)
Mean concrete strength: $f_c' = (f_c'_{jacket} * Area_{jacket} + f_c'_{core} * Area_{core}) / Area_{section} = 33.00$, but $f_c'^{0.5} \leq 8.3$
MPa (22.5.3.1, ACI 318-14)

$M / Vd = 2.00$
 $\mu_u = 2.3795529E-012$
 $V_u = 2.9582284E-031$
 $d = 0.8 * h = 600.00$
 $N_u = 11016.808$
 $A_g = 300000.00$
From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 621900.694$

where:

$V_{s1} = 523602.964$ is calculated for jacket, with:

$d = 600.00$
 $A_v = 157079.633$
 $f_y = 555.56$
 $s = 100.00$

V_{s1} is multiplied by $Col1 = 1.00$

$s/d = 0.16666667$

$V_{s2} = 98297.73$ is calculated for core, with:

$d = 440.00$
 $A_v = 100530.965$
 $f_y = 555.56$
 $s = 250.00$

V_{s2} is multiplied by $Col2 = 1.00$

$s/d = 0.56818182$

$V_f ((11-3)-(11.4), ACI 440) = 463792.00$

$f = 0.95$, for fully-wrapped sections

$w_f / s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,
where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, a)$, is implemented for every different fiber orientation a_i ,
as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, a1)|)$, with:

total thickness per orientation, $t_{f1} = NL * t / \text{NoDir} = 1.00$

$d_{fv} = d$ (figure 11.2, ACI 440) = 707.00

$f_{fe} ((11-5), ACI 440) = 328.00$

$E_f = 82000.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.009$

From (11-11), ACI 440: $V_s + V_f \leq 915872.391$

$bw = 400.00$

End Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At local axis: 2

Start Of Calculation of Chord Rotation Capacity for element: column JC1 of floor 1

At local axis: 2

Integration Section: (b)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$

Chord Rotation is generally considered as Deformation-Controlled Action according to Table C7-1, ASCE 41-17.

Mean strengths are used for Deformation-Controlled Actions according to 7.5.1.3, ASCE 41-17

Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Existing Column

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

External Height, $H = 400.00$

External Width, $W = 750.00$

Internal Height, $H = 200.00$

Internal Width, $W = 550.00$

Cover Thickness, $c = 25.00$

Element Length, $L = 3000.00$

Primary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Inadequate Lap Length with $l_b/l_d = 0.30$

FRP Wrapping Data

Type: Carbon

Dry properties (design values)

Thickness, $t = 1.00$

Tensile Strength, $f_{fu} = 840.00$

Tensile Modulus, $E_f = 82000.00$

Elongation, $e_{fu} = 0.009$

Number of directions, $NoDir = 1$

Fiber orientations, $bi = 0.00^\circ$

Number of layers, $NL = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

Bending Moment, $M = -5.8822499E-011$

Shear Force, $V_2 = 3112.69$

Shear Force, $V_3 = 2.2719624E-013$

Axial Force, $F = -15947.314$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $As_c = 2676.637$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{t,ten} = 1137.257$

-Compression: $As_{l,com} = 1137.257$

-Middle: $As_{l,mid} = 402.1239$

Longitudinal External Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{l,ten,jacket} = 829.3805$

-Compression: $As_{l,com,jacket} = 829.3805$

-Middle: $As_{l,mid,jacket} = 402.1239$

Longitudinal Internal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{l,ten,core} = 307.8761$

-Compression: $As_{l,com,core} = 307.8761$

-Middle: $As_{l,mid,core} = 0.00$

Mean Diameter of Tension Reinforcement, $DbL = 16.80$

New component: From table 7-7, ASCE 41_17: Final chord rotation Capacity $u, R = 1.0^* u = 0.0274056$
 $u = y + p = 0.0274056$

- Calculation of y -

$y = (M_y * L_s / 3) / E_{eff} = 0.00203441$ ((4.29), Biskinis Phd))
 $M_y = 1.3183E+008$
 $L_s = M/V$ (with $L_s > 0.1 * L$ and $L_s < 2 * L$) = 1500.00
From table 10.5, ASCE 41_17: $E_{eff} = factor * E_c * I_g = 3.2399E+013$
 $factor = 0.30$
 $A_g = 300000.00$
Mean concrete strength: $f_c' = (f_c'_{jacket} * Area_{jacket} + f_c'_{core} * Area_{core}) / Area_{section} = 33.00$
 $N = 15947.314$
 $E_c * I_g = E_c_{jacket} * I_{g,jacket} + E_c_{core} * I_{g,core} = 1.0800E+014$

Calculation of Yielding Moment M_y

Calculation of y and M_y according to Annex 7 -

$y = \text{Min}(y_{ten}, y_{com})$
 $y_{ten} = 5.6388532E-006$
with ((10.1), ASCE 41-17) $f_y = \text{Min}(f_y, 1.25 * f_y * (l_b / d)^{2/3}) = 311.2112$
 $d = 357.00$
 $y = 0.22702375$
 $A = 0.01018816$
 $B = 0.00579182$
with $p_t = 0.00424746$
 $p_c = 0.00424746$
 $p_v = 0.00150186$
 $N = 15947.314$
 $b = 750.00$
 $" = 0.12044818$
 $y_{comp} = 2.7794074E-005$
with f_c^* (12.3, (ACI 440)) = 33.35499
 $f_c = 33.00$
 $f_l = 0.77176471$
 $b = 750.00$
 $h = 400.00$
 $A_g = 300000.00$
From (12.9), ACI 440: $k_a = 0.14671972$
 $g = p_t + p_c + p_v = 0.00999678$
 $rc = 40.00$
 $A_e / A_c = 0.51581153$
Effective FRP thickness, $t_f = NL * t * \text{Cos}(b_1) = 1.00$
effective strain from (12.5) and (12.12), $e_{fe} = 0.004$
 $f_u = 0.009$
 $E_f = 82000.00$
 $E_c = 26999.444$
 $y = 0.22410833$
 $A = 0.00986286$
 $B = 0.00560044$
with $E_s = 200000.00$

Calculation of ratio l_b / d

Inadequate Lap Length with $l_b / d = 0.30$

- Calculation of p -

From table 10-8: $p = 0.02537119$

with:

- Columns controlled by inadequate development or splicing along the clear height because $l_b/l_d < 1$

shear control ratio $V_{yE}/V_{CoIE} = 0.14265244$

$d = d_{\text{external}} = 357.00$

$s = s_{\text{external}} = 0.00$

$t = s_1 + s_2 + 2 \cdot t_f/bw \cdot (f_{fe}/f_s) = 0.00393687$

jacket: $s_1 = A_{v1} \cdot h_1 / (s_1 \cdot A_g) = 0.0020944$

$A_{v1} = 157.0796$, is the total area of all stirrups parallel to loading (shear) direction

$h_1 = 400.00$

$s_1 = 100.00$

core: $s_2 = A_{v2} \cdot h_2 / (s_2 \cdot A_g) = 0.00026808$

$A_{v2} = 100.531$, is the total area of all stirrups parallel to loading (shear) direction

$h_2 = 200.00$

$s_2 = 250.00$

The term $2 \cdot t_f/bw \cdot (f_{fe}/f_s)$ is implemented to account for FRP contribution

where $f = 2 \cdot t_f/bw$ is FRP ratio (EC8 - 3, A.4.4.3(6)) and f_{fe}/f_s normalises f to steel strength

All these variables have already been given in Shear control ratio calculation.

For the normalisation f_s of jacket is used.

$NUD = 15947.314$

$A_g = 300000.00$

$f_{cE} = (f_{c_jacket} \cdot \text{Area_jacket} + f_{c_core} \cdot \text{Area_core}) / \text{section_area} = 33.00$

$f_{yIE} = (f_{y_ext_Long_Reinf} \cdot \text{Area_ext_Long_Reinf} + f_{y_int_Long_Reinf} \cdot \text{Area_int_Long_Reinf}) / \text{Area_Tot_Long_Rein} = 555.56$

$f_{yIE} = (f_{y_ext_Trans_Reinf} \cdot s_1 + f_{y_int_Trans_Reinf} \cdot s_2) / (s_1 + s_2) = 555.56$

$p_l = \text{Area_Tot_Long_Rein} / (b \cdot d) = 0.00999678$

$b = 750.00$

$d = 357.00$

$f_{cE} = 33.00$

End Of Calculation of Chord Rotation Capacity for element: column JC1 of floor 1

At local axis: 2

Integration Section: (b)

Calculation No. 15

column C1, Floor 1

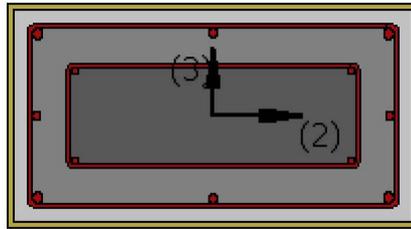
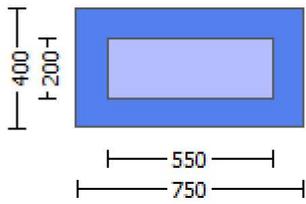
Limit State: Life Safety (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Shear capacity V_{Rd}

Edge: End

Local Axis: (3)



Start Of Calculation of Shear Capacity for element: column JC1 of floor 1

At local axis: 3

Integration Section: (b)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$

Member Shear Force is generally considered as Force-Controlled Action according to Table C7-1, ASCE 41-17.

Lower-bound strengths are used for Force-Controlled Actions according to 7.5.1.3, ASCE 41-17

Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Primary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Existing Column

New material of Primary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Primary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of γ for displacement ductility demand, the expected (mean value) strengths are used (7.5.1.3, ASCE 41-17) because bending is considered as Deformation-Controlled Action (Table C7-1, ASCE 41-17).

Jacket

New material: Concrete Strength, $f_c = f_{cm} = 33.00$

New material: Steel Strength, $f_s = f_{sm} = 555.56$

Existing Column

New material: Concrete Strength, $f_c = f_{cm} = 33.00$

New material: Steel Strength, $f_s = f_{sm} = 555.56$

#####

External Height, $H = 400.00$

External Width, $W = 750.00$

Internal Height, $H = 200.00$

Internal Width, $W = 550.00$

Cover Thickness, $c = 25.00$

Element Length, $L = 3000.00$

Primary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Inadequate Lap Length with $l_o/l_{o,min} = l_b/l_d = 0.30$

FRP Wrapping Data

Type: Carbon

Dry properties (design values)

Thickness, $t = 1.00$

Tensile Strength, $f_{fu} = 840.00$

Tensile Modulus, $E_f = 82000.00$

Elongation, $e_{fu} = 0.009$

Number of directions, $NoDir = 1$

Fiber orientations, b_i : 0.00°
Number of layers, NL = 1
Radius of rounding corners, R = 40.00

Stepwise Properties

EDGE -A-
Bending Moment, M_a = 7.4177612E-010
Shear Force, V_a = -2.2719624E-013
EDGE -B-
Bending Moment, M_b = -5.8822499E-011
Shear Force, V_b = 2.2719624E-013
BOTH EDGES
Axial Force, F = -15947.314
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: As_t = 0.00
-Compression: As_c = 2676.637
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: $As_{t,ten}$ = 1137.257
-Compression: $As_{c,com}$ = 1137.257
-Middle: As_{mid} = 402.1239
Mean Diameter of Tension Reinforcement, $Db_{L,ten}$ = 16.80

New component: From table 7-7, ASCE 41_17: Final Shear Capacity $V_R = 1.0 \cdot V_n = 776982.499$
 V_n ((10.3), ASCE 41-17) = $k_n \cdot V_{CoI0} = 776982.499$
 $V_{CoI} = 776982.499$
 $k_n = 1.00$
displacement_ductility_demand = 0.00

NOTE: In expression (10-3) ' $V_s = A_v \cdot f_y \cdot d / s$ ' is replaced by ' $V_s + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)
Mean concrete strength: $f'_c = (f'_{c,jacket} \cdot Area_{jacket} + f'_{c,core} \cdot Area_{core}) / Area_{section} = 25.00$, but $f_c^{0.5} \leq 8.3$
MPa (22.5.3.1, ACI 318-14)
 $M / Vd = 2.00$
 $M_u = 5.8822499E-011$
 $V_u = 2.2719624E-013$
 $d = 0.8 \cdot h = 320.00$
 $N_u = 15947.314$
 $A_g = 300000.00$
From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 251327.412$
where:
 $V_{s1} = 251327.412$ is calculated for jacket, with:
 $d = 320.00$
 $A_v = 157079.633$
 $f_y = 500.00$
 $s = 100.00$
 V_{s1} is multiplied by $Col1 = 1.00$
 $s/d = 0.3125$
 $V_{s2} = 0.00$ is calculated for core, with:
 $d = 160.00$
 $A_v = 100530.965$
 $f_y = 500.00$
 $s = 250.00$
 V_{s2} is multiplied by $Col2 = 0.00$
 $s/d = 1.5625$
 V_f ((11-3)-(11.4), ACI 440) = 234192.00
 $f = 0.95$, for fully-wrapped sections
 $w_f / s_f = 1$ (FRP strips adjacent to one another).
In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,
where a is the angle of the crack direction (see KANEPE).
This later relation, considered as a function $V_f(\theta, \dots)$, is implemented for every different fiber orientation a_i ,

as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta_1 = b_1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta_1)|, |V_f(-45, \theta_1)|)$, with:

total thickness per orientation, $t_{f1} = NL \cdot t / \text{NoDir} = 1.00$

$d_{fv} = d$ (figure 11.2, ACI 440) = 357.00

f_{fe} ((11-5), ACI 440) = 328.00

$E_f = 82000.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.009$

From (11-11), ACI 440: $V_s + V_f \leq 797164.595$

$b_w = 750.00$

displacement ductility demand is calculated as δ / y

- Calculation of δ / y for END B -

for rotation axis 2 and integ. section (b)

From analysis, chord rotation $\theta = 3.7334643E-021$

$y = (M_y \cdot L_s / 3) / E_{eff} = 0.00203441$ ((4.29), Biskinis Phd)

$M_y = 1.3183E+008$

$L_s = M/V$ (with $L_s > 0.1 \cdot L$ and $L_s < 2 \cdot L$) = 1500.00

From table 10.5, ASCE 41_17: $E_{eff} = \text{factor} \cdot E_c \cdot I_g = 3.2399E+013$

factor = 0.30

$A_g = 300000.00$

Mean concrete strength: $f_c' = (f_c'_{jacket} \cdot \text{Area}_{jacket} + f_c'_{core} \cdot \text{Area}_{core}) / \text{Area}_{section} = 33.00$

$N = 15947.314$

$E_c \cdot I_g = E_{c,jacket} \cdot I_{g,jacket} + E_{c,core} \cdot I_{g,core} = 1.0800E+014$

Calculation of Yielding Moment M_y

Calculation of δ / y and M_y according to Annex 7 -

$y = \text{Min}(y_{ten}, y_{com})$

$y_{ten} = 5.6388532E-006$

with ((10.1), ASCE 41-17) $f_y = \text{Min}(f_y, 1.25 \cdot f_y \cdot (I_b / I_d)^{2/3}) = 311.2112$

$d = 357.00$

$y = 0.22702375$

$A = 0.01018816$

$B = 0.00579182$

with $p_t = 0.00424746$

$p_c = 0.00424746$

$p_v = 0.00150186$

$N = 15947.314$

$b = 750.00$

$\alpha = 0.12044818$

$y_{comp} = 2.7794074E-005$

with f_c^* (12.3, (ACI 440)) = 33.35499

$f_c = 33.00$

$f_l = 0.77176471$

$b = 750.00$

$h = 400.00$

$A_g = 300000.00$

From (12.9), ACI 440: $k_a = 0.14671972$

$g = p_t + p_c + p_v = 0.00999678$

$r_c = 40.00$

$A_e / A_c = 0.51581153$

Effective FRP thickness, $t_f = NL \cdot t \cdot \text{Cos}(b_1) = 1.00$

effective strain from (12.5) and (12.12), $e_{fe} = 0.004$

$f_u = 0.009$

$E_f = 82000.00$

$E_c = 26999.444$

$y = 0.22410833$

$A = 0.00986286$

$B = 0.00560044$
with $E_s = 200000.00$

Calculation of ratio I_b/I_d

Inadequate Lap Length with $I_b/I_d = 0.30$

End Of Calculation of Shear Capacity for element: column JC1 of floor 1

At local axis: 3

Integration Section: (b)

Calculation No. 16

column C1, Floor 1

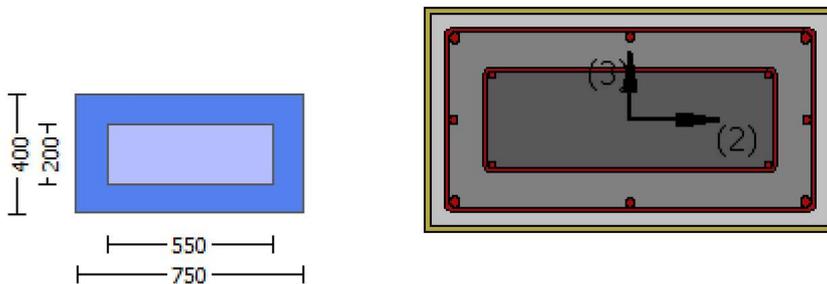
Limit State: Life Safety (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Chord rotation capacity (θ)

Edge: End

Local Axis: (3)



Start Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At Shear local axis: 3

(Bending local axis: 2)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$

Mean strength values are used for both shear and moment calculations.

Consequently:

Jacket

New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

```

Existing Column
New material of Primary Member: Concrete Strength,  $f_c = f_{cm} = 33.00$ 
New material of Primary Member: Steel Strength,  $f_s = f_{sm} = 555.56$ 
Concrete Elasticity,  $E_c = 26999.444$ 
Steel Elasticity,  $E_s = 200000.00$ 
#####
Note: Especially for the calculation of moment strengths,
the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14
Jacket
New material: Steel Strength,  $f_s = 1.25 \cdot f_{sm} = 694.45$ 
Existing Column
New material: Steel Strength,  $f_s = 1.25 \cdot f_{sm} = 694.45$ 
#####
External Height,  $H = 400.00$ 
External Width,  $W = 750.00$ 
Internal Height,  $H = 200.00$ 
Internal Width,  $W = 550.00$ 
Cover Thickness,  $c = 25.00$ 
Mean Confinement Factor overall section = 1.0435
Element Length,  $L = 3000.00$ 
Primary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Inadequate Lap Length with  $l_o/l_{ou,min} = 0.30$ 
FRP Wrapping Data
Type: Carbon
Dry properties (design values)
Thickness,  $t = 1.00$ 
Tensile Strength,  $f_{fu} = 840.00$ 
Tensile Modulus,  $E_f = 82000.00$ 
Elongation,  $e_{fu} = 0.009$ 
Number of directions,  $NoDir = 1$ 
Fiber orientations,  $bi: 0.00^\circ$ 
Number of layers,  $NL = 1$ 
Radius of rounding corners,  $R = 40.00$ 
-----

Stepwise Properties
-----
At local axis: 3
EDGE -A-
Shear Force,  $V_a = 3.4586346E-031$ 
EDGE -B-
Shear Force,  $V_b = -3.4586346E-031$ 
BOTH EDGES
Axial Force,  $F = -11016.808$ 
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension:  $A_{sl,t} = 0.00$ 
-Compression:  $A_{sl,c} = 2676.637$ 
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension:  $A_{sl,ten} = 1137.257$ 
-Compression:  $A_{sl,com} = 1137.257$ 
-Middle:  $A_{sl,mid} = 402.1239$ 
-----
-----

Calculation of Shear Capacity ratio ,  $V_e/V_r = 0.14265244$ 
Member Controlled by Flexure ( $V_e/V_r < 1$ )
Calculation of Shear Demand from fig. R18.6.5, ACI 318-14  $V_e = (M_{pr1} + M_{pr2})/l_n = 121055.918$ 
with
 $M_{pr1} = \text{Max}(M_{u1+} , M_{u1-}) = 1.8158E+008$ 
 $M_{u1+} = 1.8158E+008$ , is the ultimate moment strength at the edge 1 of the member in the actual moment direction
which is defined for the static loading combination
 $M_{u1-} = 1.8158E+008$ , is the ultimate moment strength at the edge 1 of the member in the opposite moment

```

direction which is defined for the static loading combination

$$M_{pr2} = \text{Max}(M_{u2+}, M_{u2-}) = 1.8158E+008$$

$M_{u2+} = 1.8158E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the the static loading combination

$M_{u2-} = 1.8158E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the the static loading combination

Calculation of M_{u1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$$\phi_u = 1.7255512E-005$$

$$M_u = 1.8158E+008$$

with full section properties:

$$b = 750.00$$

$$d = 357.00$$

$$d' = 43.00$$

$$v = 0.00124684$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$\omega (5A.5, \text{TBDY}) = 0.002$$

$$\text{Final value of } \phi_{cu}: \phi_{cu}^* = \text{shear_factor} * \text{Max}(\phi_{cu}, \phi_{cc}) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \phi_{cu} = 0.01006403$$

$$\phi_{we} ((5.4c), \text{TBDY}) = a_{se} * \phi_{sh, \min} * f_{ywe} / f_{ce} + \text{Min}(\phi_{fx}, \phi_{fy}) = 0.02692906$$

where $\phi_f = a_f * \rho_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\phi_{fx} = 0.02504965$$

$$a_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } \rho_f = 2t_f / b_w = 0.00266667$$

$$b_w = 750.00$$

$$\text{effective stress from (A.35), } f_{fe} = 800.0873$$

$$\phi_{fy} = 0.04491794$$

$$a_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } \rho_f = 2t_f / b_w = 0.005$$

$$b_w = 400.00$$

$$\text{effective stress from (A.35), } f_{fe} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L * t * \text{Cos}(b_1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{,f} = 0.015$$

$$a_{se} ((5.4d), \text{TBDY}) = (a_{se1} * A_{ext} + a_{se2} * A_{int}) / A_{sec} = 0.12601038$$

$$a_{se1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i2,1} = 1.1834E+006$$

$$a_{se2} = \text{Max}(a_{se1}, a_{se2}) = 0.12601038$$

$$b_{o,2} = 542.00$$

$$h_{o,2} = 192.00$$

$$b_{i2,2} = 661256.00$$

$$\phi_{psh, \min} * f_{ywe} = \text{Min}(\phi_{psh, x} * f_{ywe}, \phi_{psh, y} * f_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $\phi_{psh, \min} * f_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\phi_{psh, x} * f_{ywe} = \phi_{psh1} * f_{ywe1} + \phi_{ps2} * f_{ywe2} = 1.64062$$

$$\phi_{ps1} (\text{external}) = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$$

$$A_{sh1} = A_{stir,1} * n_{s,1} = 157.0796$$

No stirups, ns₁ = 2.00
h₁ = 400.00
ps₂ (internal) = (Ash₂*h₂/s₂)/Asec = 0.00026808
Ash₂ = Astir₂*ns₂ = 100.531
No stirups, ns₂ = 2.00
h₂ = 200.00

psh_y*Fywe = psh₁*Fywe₁+ps₂*Fywe₂ = 3.23907
ps₁ (external) = (Ash₁*h₁/s₁)/Asec = 0.00392699
Ash₁ = Astir₁*ns₁ = 157.0796
No stirups, ns₁ = 2.00
h₁ = 750.00
ps₂ (internal) = (Ash₂*h₂/s₂)/Asec = 0.00073723
Ash₂ = Astir₂*ns₂ = 100.531
No stirups, ns₂ = 2.00
h₂ = 550.00

Asec = 300000.00
s₁ = 100.00
s₂ = 250.00
fywe₁ = 694.45
fywe₂ = 694.45
fce = 33.00

From ((5.A.5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y₁ = 0.00140044
sh₁ = 0.0044814
ft₁ = 466.8167
fy₁ = 389.0139
su₁ = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/l_d = 0.30

su₁ = 0.4*esu₁_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu₁_nominal = 0.08,

For calculation of esu₁_nominal and y₁, sh₁,ft₁,fy₁, it is considered
characteristic value fsy₁ = fs₁/1.2, from table 5.1, TBDY.

y₁, sh₁,ft₁,fy₁, are also multiplied by Min(1,1.25*(lb/l_d)^{2/3}), from 10.3.5, ASCE 41-17.

with fs₁ = (fs_{jacket}*As_{l,ten,jacket} + fs_{core}*As_{l,ten,core})/As_{l,ten} = 389.0139

with Es₁ = (Es_{jacket}*As_{l,ten,jacket} + Es_{core}*As_{l,ten,core})/As_{l,ten} = 200000.00

y₂ = 0.00140044
sh₂ = 0.0044814
ft₂ = 466.8167
fy₂ = 389.0139
su₂ = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/l_{b,min} = 0.30

su₂ = 0.4*esu₂_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu₂_nominal = 0.08,

For calculation of esu₂_nominal and y₂, sh₂,ft₂,fy₂, it is considered
characteristic value fsy₂ = fs₂/1.2, from table 5.1, TBDY.

y₂, sh₂,ft₂,fy₂, are also multiplied by Min(1,1.25*(lb/l_d)^{2/3}), from 10.3.5, ASCE 41-17.

with fs₂ = (fs_{jacket}*As_{l,com,jacket} + fs_{core}*As_{l,com,core})/As_{l,com} = 389.0139

with Es₂ = (Es_{jacket}*As_{l,com,jacket} + Es_{core}*As_{l,com,core})/As_{l,com} = 200000.00

y_v = 0.00140044
sh_v = 0.0044814
ft_v = 466.8167
fy_v = 389.0139
su_v = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/l_d = 0.30

su_v = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: $esuv_nominal = 0.08$,
considering characteristic value $fsv = fsv/1.2$, from table 5.1, TBDY
For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered
characteristic value $fsv = fsv/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $Min(1, 1.25*(lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fsv = (fs_{jacket} * Asl_{mid,jacket} + fs_{mid} * Asl_{mid,core}) / Asl_{mid} = 389.0139$

with $Esv = (Es_{jacket} * Asl_{mid,jacket} + Es_{mid} * Asl_{mid,core}) / Asl_{mid} = 200000.00$

$1 = Asl_{ten} / (b * d) * (fs1 / fc) = 0.0500703$

$2 = Asl_{com} / (b * d) * (fs2 / fc) = 0.0500703$

$v = Asl_{mid} / (b * d) * (fsv / fc) = 0.01770442$

and confined core properties:

$b = 690.00$

$d = 327.00$

$d' = 13.00$

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

$c =$ confinement factor = 1.0435

$1 = Asl_{ten} / (b * d) * (fs1 / fc) = 0.05941729$

$2 = Asl_{com} / (b * d) * (fs2 / fc) = 0.05941729$

$v = Asl_{mid} / (b * d) * (fsv / fc) = 0.02100943$

Case/Assumption: Unconfined full section - Steel rupture
' satisfies Eq. (4.3)

---->

$v < vs_{y2}$ - LHS eq.(4.5) is satisfied

---->

su (4.9) = 0.16886055

$Mu = MRc$ (4.14) = 1.8158E+008

$u = su$ (4.1) = 1.7255512E-005

Calculation of ratio lb/ld

Inadequate Lap Length with $lb/ld = 0.30$

Calculation of $Mu1$ -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 1.7255512E-005$

$Mu = 1.8158E+008$

with full section properties:

$b = 750.00$

$d = 357.00$

$d' = 43.00$

$v = 0.00124684$

$N = 11016.808$

$fc = 33.00$

co (5A.5, TBDY) = 0.002

Final value of cu : $cu^* = shear_factor * Max(cu, cc) = 0.01006403$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $cu = 0.01006403$

where ((5.4c), TBDY) = $ase * sh_{min} * fy_{we} / f_{ce} + Min(fx, fy) = 0.02692906$

where $f = af * pf * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

 $fx = 0.02504965$

$af = 0.38744444$

$b = 750.00$

$h = 400.00$

From EC8 A4.4.3(6), $pf = 2tf/bw = 0.00266667$

$bw = 750.00$

effective stress from (A.35), $f_{fe} = 800.0873$

$fy = 0.04491794$

af = 0.38744444
b = 400.00
h = 750.00
From EC8 A.4.4.3(6), pf = 2tf/bw = 0.005
bw = 400.00
effective stress from (A.35), ff,e = 765.1636

R = 40.00
Effective FRP thickness, tf = NL*t*Cos(b1) = 1.00
fu,f = 840.00
Ef = 82000.00
u,f = 0.015
ase ((5.4d), TBDY) = (ase1*Aext+ase2*Aint)/Asec = 0.12601038
ase1 = 0.12601038
bo_1 = 690.00
ho_1 = 340.00
bi2_1 = 1.1834E+006
ase2 = Max(ase1,ase2) = 0.12601038
bo_2 = 542.00
ho_2 = 192.00
bi2_2 = 661256.00
psh,min*Fywe = Min(psh,x*Fywe , psh,y*Fywe) = 1.64062

Expression ((5.4d), TBDY) for psh,min*Fywe has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh_x*Fywe = psh1*Fywe1+ps2*Fywe2 = 1.64062
ps1 (external) = (Ash1*h1/s1)/Asec = 0.0020944
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 400.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00026808
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 200.00

psh_y*Fywe = psh1*Fywe1+ps2*Fywe2 = 3.23907
ps1 (external) = (Ash1*h1/s1)/Asec = 0.00392699
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 750.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00073723
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/d = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^2/3), from 10.3.5, ASCE 41-17.

with $f_{s1} = (f_{s,jacket} \cdot A_{s,ten,jacket} + f_{s,core} \cdot A_{s,ten,core}) / A_{s,ten} = 389.0139$
 with $E_{s1} = (E_{s,jacket} \cdot A_{s,ten,jacket} + E_{s,core} \cdot A_{s,ten,core}) / A_{s,ten} = 200000.00$
 $y_2 = 0.00140044$
 $sh_2 = 0.0044814$
 $ft_2 = 466.8167$
 $fy_2 = 389.0139$
 $su_2 = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_0/l_{ou,min} = l_b/l_{b,min} = 0.30$
 $su_2 = 0.4 \cdot esu_{2,nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu_{2,nominal} = 0.08$,
 For calculation of $esu_{2,nominal}$ and y_2, sh_2, ft_2, fy_2 , it is considered
 characteristic value $f_{sy2} = f_{s2}/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $f_{s2} = (f_{s,jacket} \cdot A_{s,com,jacket} + f_{s,core} \cdot A_{s,com,core}) / A_{s,com} = 389.0139$
 with $E_{s2} = (E_{s,jacket} \cdot A_{s,com,jacket} + E_{s,core} \cdot A_{s,com,core}) / A_{s,com} = 200000.00$
 $y_v = 0.00140044$
 $sh_v = 0.0044814$
 $ft_v = 466.8167$
 $fy_v = 389.0139$
 $su_v = 0.00512$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_0/l_{ou,min} = l_b/l_d = 0.30$
 $su_v = 0.4 \cdot esu_{v,nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu_{v,nominal} = 0.08$,
 considering characteristic value $f_{syv} = f_{sv}/1.2$, from table 5.1, TBDY
 For calculation of $esu_{v,nominal}$ and y_v, sh_v, ft_v, fy_v , it is considered
 characteristic value $f_{syv} = f_{sv}/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $f_{sv} = (f_{s,jacket} \cdot A_{s,mid,jacket} + f_{s,mid} \cdot A_{s,mid,core}) / A_{s,mid} = 389.0139$
 with $E_{sv} = (E_{s,jacket} \cdot A_{s,mid,jacket} + E_{s,mid} \cdot A_{s,mid,core}) / A_{s,mid} = 200000.00$
 $1 = A_{s,ten} / (b \cdot d) \cdot (f_{s1} / f_c) = 0.0500703$
 $2 = A_{s,com} / (b \cdot d) \cdot (f_{s2} / f_c) = 0.0500703$
 $v = A_{s,mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.01770442$
 and confined core properties:
 $b = 690.00$
 $d = 327.00$
 $d' = 13.00$
 $f_{cc} (5A.2, TBDY) = 34.43547$
 $cc (5A.5, TBDY) = 0.00243499$
 $c = \text{confinement factor} = 1.0435$
 $1 = A_{s,ten} / (b \cdot d) \cdot (f_{s1} / f_c) = 0.05941729$
 $2 = A_{s,com} / (b \cdot d) \cdot (f_{s2} / f_c) = 0.05941729$
 $v = A_{s,mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.02100943$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)
 --->
 $v < v_{s,y2}$ - LHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.16886055$
 $Mu = MRc (4.14) = 1.8158E+008$
 $u = su (4.1) = 1.7255512E-005$

 Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of Mu_{2+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$$u = 1.7255512E-005$$

$$Mu = 1.8158E+008$$

with full section properties:

$$b = 750.00$$

$$d = 357.00$$

$$d' = 43.00$$

$$v = 0.00124684$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$\omega (5A.5, TBDY) = 0.002$$

$$\text{Final value of } \phi_u: \phi_u^* = \text{shear_factor} * \text{Max}(\phi_u, \phi_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \phi_u = 0.01006403$$

$$\omega_e (5.4c, TBDY) = a_{se} * \text{sh}_{, \min} * f_{ywe} / f_{ce} + \text{Min}(\phi_x, \phi_y) = 0.02692906$$

where $\phi = a_f * \rho_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\phi_x = 0.02504965$$

$$a_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A 4.4.3(6), } \rho_f = 2t_f / bw = 0.00266667$$

$$bw = 750.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 800.0873$$

$$\phi_y = 0.04491794$$

$$a_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A 4.4.3(6), } \rho_f = 2t_f / bw = 0.005$$

$$bw = 400.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL * t * \text{Cos}(b1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{,f} = 0.015$$

$$a_{se} ((5.4d), TBDY) = (a_{se1} * A_{ext} + a_{se2} * A_{int}) / A_{sec} = 0.12601038$$

$$a_{se1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i2,1} = 1.1834E+006$$

$$a_{se2} = \text{Max}(a_{se1}, a_{se2}) = 0.12601038$$

$$b_{o,2} = 542.00$$

$$h_{o,2} = 192.00$$

$$b_{i2,2} = 661256.00$$

$$\text{psh}_{, \min} * F_{ywe} = \text{Min}(\text{psh}_{,x} * F_{ywe}, \text{psh}_{,y} * F_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $\text{psh}_{, \min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\text{psh}_{,x} * F_{ywe} = \text{psh}_1 * F_{ywe1} + \text{ps}_2 * F_{ywe2} = 1.64062$$

$$\text{ps}_1 (\text{external}) = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$$

$$A_{sh1} = A_{\text{stir}_1} * n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

$$h_1 = 400.00$$

$$\text{ps}_2 (\text{internal}) = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00026808$$

$$A_{sh2} = A_{\text{stir}_2} * n_{s,2} = 100.531$$

$$\text{No stirrups, } n_{s,2} = 2.00$$

$$h_2 = 200.00$$

$$\text{psh}_{,y} * F_{ywe} = \text{psh}_1 * F_{ywe1} + \text{ps}_2 * F_{ywe2} = 3.23907$$

$$\text{ps}_1 (\text{external}) = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.00392699$$

$$A_{sh1} = A_{\text{stir}_1} * n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

h1 = 750.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00073723
Ash2 = Astir_2*ns_2 = 100.531
No stirups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/d = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044
sh2 = 0.0044814
ft2 = 466.8167
fy2 = 389.0139
su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044
shv = 0.0044814
ftv = 466.8167
fyv = 389.0139
suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/d = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.0500703
2 = Asl,com/(b*d)*(fs2/fc) = 0.0500703

v = Asl,mid/(b*d)*(fsv/fc) = 0.01770442

and confined core properties:

$b = 690.00$
 $d = 327.00$
 $d' = 13.00$
 $f_{cc} (5A.2, TBDY) = 34.43547$
 $cc (5A.5, TBDY) = 0.00243499$
 $c = \text{confinement factor} = 1.0435$
 $1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.05941729$
 $2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.05941729$
 $v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.02100943$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)

--->
 $v < v_{s,y2}$ - LHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.16886055$
 $Mu = MRc (4.14) = 1.8158E+008$
 $u = su (4.1) = 1.7255512E-005$

 Calculation of ratio l_b/l_d

 Inadequate Lap Length with $l_b/l_d = 0.30$

 Calculation of Mu_2 -

 Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 1.7255512E-005$
 $Mu = 1.8158E+008$

 with full section properties:

$b = 750.00$
 $d = 357.00$
 $d' = 43.00$
 $v = 0.00124684$
 $N = 11016.808$
 $f_c = 33.00$
 $co (5A.5, TBDY) = 0.002$
 Final value of cu : $cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.01006403$
 The Shear_factor is considered equal to 1 (pure moment strength)
 From (5.4b), TBDY: $cu = 0.01006403$
 $w_e ((5.4c), TBDY) = a_{se} * sh_{,min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$
 where $f = a_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

 $f_x = 0.02504965$
 $a_f = 0.38744444$
 $b = 750.00$
 $h = 400.00$
 From EC8 A.4.4.3(6), $p_f = 2t_f/b_w = 0.00266667$
 $b_w = 750.00$
 effective stress from (A.35), $f_{f,e} = 800.0873$

 $f_y = 0.04491794$
 $a_f = 0.38744444$
 $b = 400.00$
 $h = 750.00$
 From EC8 A.4.4.3(6), $p_f = 2t_f/b_w = 0.005$
 $b_w = 400.00$
 effective stress from (A.35), $f_{f,e} = 765.1636$

 $R = 40.00$
 Effective FRP thickness, $t_f = NL * t * \text{Cos}(b_1) = 1.00$
 $f_{u,f} = 840.00$
 $E_f = 82000.00$

$$u,f = 0.015$$

$$ase((5.4d), TBDY) = (ase1 \cdot A_{ext} + ase2 \cdot A_{int}) / A_{sec} = 0.12601038$$

$$ase1 = 0.12601038$$

$$bo_1 = 690.00$$

$$ho_1 = 340.00$$

$$bi2_1 = 1.1834E+006$$

$$ase2 = \text{Max}(ase1, ase2) = 0.12601038$$

$$bo_2 = 542.00$$

$$ho_2 = 192.00$$

$$bi2_2 = 661256.00$$

$$psh_{min} \cdot F_{ywe} = \text{Min}(psh_x \cdot F_{ywe}, psh_y \cdot F_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $psh_{min} \cdot F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$psh_x \cdot F_{ywe} = psh1 \cdot F_{ywe1} + ps2 \cdot F_{ywe2} = 1.64062$$

$$ps1 \text{ (external)} = (Ash1 \cdot h1 / s1) / A_{sec} = 0.0020944$$

$$Ash1 = Astir_1 \cdot ns_1 = 157.0796$$

$$\text{No stirrups, } ns_1 = 2.00$$

$$h1 = 400.00$$

$$ps2 \text{ (internal)} = (Ash2 \cdot h2 / s2) / A_{sec} = 0.00026808$$

$$Ash2 = Astir_2 \cdot ns_2 = 100.531$$

$$\text{No stirrups, } ns_2 = 2.00$$

$$h2 = 200.00$$

$$psh_y \cdot F_{ywe} = psh1 \cdot F_{ywe1} + ps2 \cdot F_{ywe2} = 3.23907$$

$$ps1 \text{ (external)} = (Ash1 \cdot h1 / s1) / A_{sec} = 0.00392699$$

$$Ash1 = Astir_1 \cdot ns_1 = 157.0796$$

$$\text{No stirrups, } ns_1 = 2.00$$

$$h1 = 750.00$$

$$ps2 \text{ (internal)} = (Ash2 \cdot h2 / s2) / A_{sec} = 0.00073723$$

$$Ash2 = Astir_2 \cdot ns_2 = 100.531$$

$$\text{No stirrups, } ns_2 = 2.00$$

$$h2 = 550.00$$

$$A_{sec} = 300000.00$$

$$s1 = 100.00$$

$$s2 = 250.00$$

$$fywe1 = 694.45$$

$$fywe2 = 694.45$$

$$fce = 33.00$$

$$\text{From ((5.A5), TBDY), TBDY: } cc = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$y1 = 0.00140044$$

$$sh1 = 0.0044814$$

$$ft1 = 466.8167$$

$$fy1 = 389.0139$$

$$su1 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$$lo/lo_{min} = lb/ld = 0.30$$

$$su1 = 0.4 \cdot esu1_{nominal}((5.5), TBDY) = 0.032$$

$$\text{From table 5A.1, TBDY: } esu1_{nominal} = 0.08,$$

For calculation of $esu1_{nominal}$ and $y1, sh1, ft1, fy1$, it is considered characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$$y1, sh1, ft1, fy1, \text{ are also multiplied by } \text{Min}(1, 1.25 \cdot (lb/ld)^{2/3}), \text{ from 10.3.5, ASCE 41-17.}$$

$$\text{with } fs1 = (fs_{jacket} \cdot A_{sl,ten,jacket} + fs_{core} \cdot A_{sl,ten,core}) / A_{sl,ten} = 389.0139$$

$$\text{with } Es1 = (Es_{jacket} \cdot A_{sl,ten,jacket} + Es_{core} \cdot A_{sl,ten,core}) / A_{sl,ten} = 200000.00$$

$$y2 = 0.00140044$$

$$sh2 = 0.0044814$$

$$ft2 = 466.8167$$

$$fy2 = 389.0139$$

$$su2 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$$lo/lo_{min} = lb/lb_{min} = 0.30$$

$$su_2 = 0.4 * esu_2_nominal ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $esu_2_nominal = 0.08$,

For calculation of $esu_2_nominal$ and y_2, sh_2, ft_2, fy_2 , it is considered characteristic value $fsy_2 = fs_2/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fs_2 = (fs_jacket * Asl_com_jacket + fs_core * Asl_com_core) / Asl_com = 389.0139$$

$$\text{with } Es_2 = (Es_jacket * Asl_com_jacket + Es_core * Asl_com_core) / Asl_com = 200000.00$$

$$yv = 0.00140044$$

$$shv = 0.0044814$$

$$ftv = 466.8167$$

$$fyv = 389.0139$$

$$suv = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$$lo/lo_{u,min} = lb/ld = 0.30$$

$$suv = 0.4 * esuv_nominal ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $esuv_nominal = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered

characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fsv = (fs_jacket * Asl_mid_jacket + fs_mid * Asl_mid_core) / Asl_mid = 389.0139$$

$$\text{with } Esv = (Es_jacket * Asl_mid_jacket + Es_mid * Asl_mid_core) / Asl_mid = 200000.00$$

$$1 = Asl_ten / (b * d) * (fs_1 / fc) = 0.0500703$$

$$2 = Asl_com / (b * d) * (fs_2 / fc) = 0.0500703$$

$$v = Asl_mid / (b * d) * (fsv / fc) = 0.01770442$$

and confined core properties:

$$b = 690.00$$

$$d = 327.00$$

$$d' = 13.00$$

$$fcc (5A.2, TBDY) = 34.43547$$

$$cc (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = Asl_ten / (b * d) * (fs_1 / fc) = 0.05941729$$

$$2 = Asl_com / (b * d) * (fs_2 / fc) = 0.05941729$$

$$v = Asl_mid / (b * d) * (fsv / fc) = 0.02100943$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y_2}$ - LHS eq.(4.5) is satisfied

--->

$$su (4.9) = 0.16886055$$

$$Mu = MRc (4.14) = 1.8158E+008$$

$$u = su (4.1) = 1.7255512E-005$$

Calculation of ratio lb/ld

Inadequate Lap Length with $lb/ld = 0.30$

Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 848607.437$

Calculation of Shear Strength at edge 1, $V_{r1} = 848607.437$

$$V_{r1} = V_{CoI} ((10.3), ASCE 41-17) = knl * V_{CoI0}$$

$$V_{CoI0} = 848607.437$$

$$knl = 1 \text{ (zero step-static loading)}$$

NOTE: In expression (10-3) ' $V_s = A_v * fy * d / s$ ' is replaced by ' $V_s + f * V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

$$= 1 \text{ (normal-weight concrete)}$$

Mean concrete strength: $fc' = (fc'_jacket * Area_jacket + fc'_core * Area_core) / Area_section = 33.00$, but $fc'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$M/Vd = 2.00$
 $\mu_u = 3.5345895E-012$
 $\nu_u = 3.4586346E-031$
 $d = 0.8 \cdot h = 320.00$
 $N_u = 11016.808$
 $A_g = 300000.00$
 From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 279254.914$
 where:
 $V_{s1} = 279254.914$ is calculated for jacket, with:
 $d = 320.00$
 $A_v = 157079.633$
 $f_y = 555.56$
 $s = 100.00$
 V_{s1} is multiplied by $Col1 = 1.00$
 $s/d = 0.3125$
 $V_{s2} = 0.00$ is calculated for core, with:
 $d = 160.00$
 $A_v = 100530.965$
 $f_y = 555.56$
 $s = 250.00$
 V_{s2} is multiplied by $Col2 = 0.00$
 $s/d = 1.5625$
 $V_f ((11-3)-(11.4), ACI 440) = 234192.00$
 $f = 0.95$, for fully-wrapped sections
 $w_f/s_f = 1$ (FRP strips adjacent to one another).
 In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,
 where a is the angle of the crack direction (see KANEPE).
 This later relation, considered as a function $V_f(,)$, is implemented for every different fiber orientation a_i ,
 as well as for 2 crack directions, $a = 45^\circ$ and $a = -45^\circ$ to take into consideration the cyclic seismic loading.
 orientation 1: $a_1 = b_1 + 90^\circ = 90.00$
 $V_f = \text{Min}(|V_f(45, a_1)|, |V_f(-45, a_1)|)$, with:
 total thickness per orientation, $t_{f1} = NL \cdot t / N_{oDir} = 1.00$
 $d_{fv} = d$ (figure 11.2, ACI 440) = 357.00
 $f_{fe} ((11-5), ACI 440) = 328.00$
 $E_f = 82000.00$
 $f_e = 0.004$, from (11.6a), ACI 440
 with $f_u = 0.009$
 From (11-11), ACI 440: $V_s + V_f \leq 915872.391$
 $b_w = 750.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 848607.437$
 $V_{r2} = V_{Col} ((10.3), ASCE 41-17) = knl \cdot V_{Col0}$
 $V_{Col0} = 848607.437$
 $knl = 1$ (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v \cdot f_y \cdot d / s$ ' is replaced by ' $V_s + f \cdot V_f$ '
 where V_f is the contribution of FRPs (11.3), ACI 440).

$\beta = 1$ (normal-weight concrete)
 Mean concrete strength: $f_c' = (f_c'_{jacket} \cdot Area_{jacket} + f_c'_{core} \cdot Area_{core}) / Area_{section} = 33.00$, but $f_c'^{0.5} \leq 8.3$
 MPa (22.5.3.1, ACI 318-14)

$M/Vd = 2.00$
 $\mu_u = 3.5345895E-012$
 $\nu_u = 3.4586346E-031$
 $d = 0.8 \cdot h = 320.00$
 $N_u = 11016.808$
 $A_g = 300000.00$
 From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 279254.914$
 where:
 $V_{s1} = 279254.914$ is calculated for jacket, with:
 $d = 320.00$
 $A_v = 157079.633$
 $f_y = 555.56$
 $s = 100.00$
 V_{s1} is multiplied by $Col1 = 1.00$
 $s/d = 0.3125$

Vs2 = 0.00 is calculated for core, with:

d = 160.00

Av = 100530.965

fy = 555.56

s = 250.00

Vs2 is multiplied by Col2 = 0.00

s/d = 1.5625

Vf ((11-3)-(11.4), ACI 440) = 234192.00

f = 0.95, for fully-wrapped sections

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression,

where θ is the angle of the crack direction (see KANEPE).

This later relation, considered as a function Vf(θ), is implemented for every different fiber orientation ai, as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta_1 = b1 + 90^\circ = 90.00$

Vf = Min(|Vf(45, θ_1)|, |Vf(-45, a1)|), with:

total thickness per orientation, tf1 = NL*t/NoDir = 1.00

dfv = d (figure 11.2, ACI 440) = 357.00

ffe ((11-5), ACI 440) = 328.00

Ef = 82000.00

fe = 0.004, from (11.6a), ACI 440

with fu = 0.009

From (11-11), ACI 440: Vs + Vf <= 915872.391

bw = 750.00

End Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At local axis: 3

Start Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1

At Shear local axis: 2

(Bending local axis: 3)

Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$

Mean strength values are used for both shear and moment calculations.

Consequently:

Jacket

New material of Primary Member: Concrete Strength, fc = fcm = 33.00

New material of Primary Member: Steel Strength, fs = fsm = 555.56

Concrete Elasticity, Ec = 26999.444

Steel Elasticity, Es = 200000.00

Existing Column

New material of Primary Member: Concrete Strength, fc = fcm = 33.00

New material of Primary Member: Steel Strength, fs = fsm = 555.56

Concrete Elasticity, Ec = 26999.444

Steel Elasticity, Es = 200000.00

#####

Note: Especially for the calculation of moment strengths,

the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

Jacket

New material: Steel Strength, fs = 1.25*fsm = 694.45

Existing Column

New material: Steel Strength, fs = 1.25*fsm = 694.45

#####

External Height, H = 400.00

External Width, W = 750.00

Internal Height, H = 200.00

Internal Width, W = 550.00

Cover Thickness, c = 25.00

Mean Confinement Factor overall section = 1.0435

Element Length, L = 3000.00

Primary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Inadequate Lap Length with $l_o/l_{o,min} = 0.30$
FRP Wrapping Data
Type: Carbon
Dry properties (design values)
Thickness, $t = 1.00$
Tensile Strength, $f_{fu} = 840.00$
Tensile Modulus, $E_f = 82000.00$
Elongation, $e_{fu} = 0.009$
Number of directions, $NoDir = 1$
Fiber orientations, $bi = 0.00^\circ$
Number of layers, $NL = 1$
Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 2
EDGE -A-
Shear Force, $V_a = 2.9582284E-031$
EDGE -B-
Shear Force, $V_b = -2.9582284E-031$
BOTH EDGES
Axial Force, $F = -11016.808$
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: $A_{st} = 0.00$
-Compression: $A_{sc} = 2676.637$
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: $A_{st,ten} = 1137.257$
-Compression: $A_{st,com} = 1137.257$
-Middle: $A_{st,mid} = 402.1239$

Calculation of Shear Capacity ratio , $V_e/V_r = 0.19671905$
Member Controlled by Flexure ($V_e/V_r < 1$)
Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 248405.514$
with

$M_{pr1} = \text{Max}(Mu_{1+} , Mu_{1-}) = 3.7261E+008$

$Mu_{1+} = 3.7261E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination

$Mu_{1-} = 3.7261E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination

$M_{pr2} = \text{Max}(Mu_{2+} , Mu_{2-}) = 3.7261E+008$

$Mu_{2+} = 3.7261E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the the static loading combination

$Mu_{2-} = 3.7261E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the the static loading combination

Calculation of Mu_{1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$\phi_u = 8.5888248E-006$

$M_u = 3.7261E+008$

with full section properties:

$b = 400.00$

$d = 707.00$

$d' = 43.00$

$$v = 0.00118049$$

$$N = 11016.808$$

$$fc = 33.00$$

$$co \text{ (5A.5, TBDY)} = 0.002$$

$$\text{Final value of } cu: cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } cu = 0.01006403$$

$$\text{we ((5.4c), TBDY)} = ase^* sh_{,min} * fy_{we} / f_{ce} + \text{Min}(fx, fy) = 0.02692906$$

where $f = af * pf * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$fx = 0.02504965$$

$$af = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } pf = 2tf/bw = 0.00266667$$

$$bw = 750.00$$

$$\text{effective stress from (A.35), } ff,e = 800.0873$$

$$fy = 0.04491794$$

$$af = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } pf = 2tf/bw = 0.005$$

$$bw = 400.00$$

$$\text{effective stress from (A.35), } ff,e = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } tf = NL * t * \text{Cos}(b1) = 1.00$$

$$fu,f = 840.00$$

$$Ef = 82000.00$$

$$u,f = 0.015$$

$$ase \text{ ((5.4d), TBDY)} = (ase1 * A_{ext} + ase2 * A_{int}) / A_{sec} = 0.12601038$$

$$ase1 = 0.12601038$$

$$bo_1 = 690.00$$

$$ho_1 = 340.00$$

$$bi2_1 = 1.1834E+006$$

$$ase2 = \text{Max}(ase1, ase2) = 0.12601038$$

$$bo_2 = 542.00$$

$$ho_2 = 192.00$$

$$bi2_2 = 661256.00$$

$$psh_{,min} * Fy_{we} = \text{Min}(psh_{,x} * Fy_{we}, psh_{,y} * Fy_{we}) = 1.64062$$

Expression ((5.4d), TBDY) for $psh_{,min} * Fy_{we}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$psh_{,x} * Fy_{we} = psh1 * Fy_{we1} + ps2 * Fy_{we2} = 1.64062$$

$$ps1 \text{ (external)} = (Ash1 * h1 / s1) / A_{sec} = 0.0020944$$

$$Ash1 = Astir_1 * ns_1 = 157.0796$$

$$\text{No stirrups, } ns_1 = 2.00$$

$$h1 = 400.00$$

$$ps2 \text{ (internal)} = (Ash2 * h2 / s2) / A_{sec} = 0.00026808$$

$$Ash2 = Astir_2 * ns_2 = 100.531$$

$$\text{No stirrups, } ns_2 = 2.00$$

$$h2 = 200.00$$

$$psh_{,y} * Fy_{we} = psh1 * Fy_{we1} + ps2 * Fy_{we2} = 3.23907$$

$$ps1 \text{ (external)} = (Ash1 * h1 / s1) / A_{sec} = 0.00392699$$

$$Ash1 = Astir_1 * ns_1 = 157.0796$$

$$\text{No stirrups, } ns_1 = 2.00$$

$$h1 = 750.00$$

$$ps2 \text{ (internal)} = (Ash2 * h2 / s2) / A_{sec} = 0.00073723$$

$$Ash2 = Astir_2 * ns_2 = 100.531$$

$$\text{No stirrups, } ns_2 = 2.00$$

$$h2 = 550.00$$

$$A_{sec} = 300000.00$$

$$s1 = 100.00$$

$$s2 = 250.00$$

fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A.5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044
sh2 = 0.0044814
ft2 = 466.8167
fy2 = 389.0139
su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044
shv = 0.0044814
ftv = 466.8167
fyv = 389.0139
suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.04740567

2 = Asl,com/(b*d)*(fs2/fc) = 0.04740567

v = Asl,mid/(b*d)*(fsv/fc) = 0.01676222

and confined core properties:

b = 340.00
d = 677.00
d' = 13.00

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

1 = Asl,ten/(b*d)*(fs1/fc) = 0.05824278

2 = Asl,com/(b*d)*(fs2/fc) = 0.05824278

v = Asl,mid/(b*d)*(fsv/fc) = 0.02059413

Case/Assumption: Unconfined full section - Steel rupture
' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

s_u (4.9) = 0.15682678

μ_u = MRc (4.14) = 3.7261E+008

u = s_u (4.1) = 8.5888248E-006

Calculation of ratio l_b/l_d

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of μ_{u1} -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

u = 8.5888248E-006

μ_u = 3.7261E+008

with full section properties:

b = 400.00

d = 707.00

d' = 43.00

v = 0.00118049

N = 11016.808

f_c = 33.00

α (5A.5, TBDY) = 0.002

Final value of α : $\alpha^* = \text{shear_factor} * \text{Max}(\alpha, \alpha_c) = 0.01006403$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $\alpha_c = 0.01006403$

α_{we} ((5.4c), TBDY) = $\alpha_{se} * \text{sh_min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$

where $f = \alpha^* \text{pf} * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

 f_x = 0.02504965

α_f = 0.38744444

b = 750.00

h = 400.00

From EC8 A4.4.3(6), $\text{pf} = 2t_f/bw = 0.00266667$

bw = 750.00

effective stress from (A.35), $f_{fe} = 800.0873$

 f_y = 0.04491794

α_f = 0.38744444

b = 400.00

h = 750.00

From EC8 A4.4.3(6), $\text{pf} = 2t_f/bw = 0.005$

bw = 400.00

effective stress from (A.35), $f_{fe} = 765.1636$

 R = 40.00

Effective FRP thickness, $t_f = NL * t * \text{Cos}(b1) = 1.00$

$f_{u,f}$ = 840.00

E_f = 82000.00

$u_{,f}$ = 0.015

α_{se} ((5.4d), TBDY) = $(\alpha_{se1} * A_{ext} + \alpha_{se2} * A_{int}) / A_{sec} = 0.12601038$

α_{se1} = 0.12601038

$bo_{,1}$ = 690.00

$ho_{,1}$ = 340.00

$bi_{2,1}$ = 1.1834E+006

α_{se2} = $\text{Max}(\alpha_{se1}, \alpha_{se2}) = 0.12601038$

$bo_{,2}$ = 542.00

$ho_{,2}$ = 192.00

$$bi2_2 = 661256.00$$

$$psh,min*Fywe = \text{Min}(psh,x*Fywe, psh,y*Fywe) = 1.64062$$

Expression ((5.4d), TBDY) for $psh,min*Fywe$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$psh_x*Fywe = psh1*Fywe1 + ps2*Fywe2 = 1.64062$$

$$ps1 \text{ (external)} = (Ash1*h1/s1)/Asec = 0.0020944$$

$$Ash1 = Astir_1*ns_1 = 157.0796$$

$$\text{No stirrups, } ns_1 = 2.00$$

$$h1 = 400.00$$

$$ps2 \text{ (internal)} = (Ash2*h2/s2)/Asec = 0.00026808$$

$$Ash2 = Astir_2*ns_2 = 100.531$$

$$\text{No stirrups, } ns_2 = 2.00$$

$$h2 = 200.00$$

$$psh_y*Fywe = psh1*Fywe1 + ps2*Fywe2 = 3.23907$$

$$ps1 \text{ (external)} = (Ash1*h1/s1)/Asec = 0.00392699$$

$$Ash1 = Astir_1*ns_1 = 157.0796$$

$$\text{No stirrups, } ns_1 = 2.00$$

$$h1 = 750.00$$

$$ps2 \text{ (internal)} = (Ash2*h2/s2)/Asec = 0.00073723$$

$$Ash2 = Astir_2*ns_2 = 100.531$$

$$\text{No stirrups, } ns_2 = 2.00$$

$$h2 = 550.00$$

$$Asec = 300000.00$$

$$s1 = 100.00$$

$$s2 = 250.00$$

$$fywe1 = 694.45$$

$$fywe2 = 694.45$$

$$fce = 33.00$$

$$\text{From ((5.A5), TBDY), TBDY: } cc = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$y1 = 0.00140044$$

$$sh1 = 0.0044814$$

$$ft1 = 466.8167$$

$$fy1 = 389.0139$$

$$su1 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$$lo/lou,min = lb/l_d = 0.30$$

$$su1 = 0.4*esu1_nominal \text{ ((5.5), TBDY)} = 0.032$$

$$\text{From table 5A.1, TBDY: } esu1_nominal = 0.08,$$

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$$y1, sh1, ft1, fy1, \text{ are also multiplied by } \text{Min}(1, 1.25*(lb/l_d)^{2/3}), \text{ from 10.3.5, ASCE 41-17.}$$

$$\text{with } fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139$$

$$\text{with } Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00$$

$$y2 = 0.00140044$$

$$sh2 = 0.0044814$$

$$ft2 = 466.8167$$

$$fy2 = 389.0139$$

$$su2 = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$$lo/lou,min = lb/l_b,min = 0.30$$

$$su2 = 0.4*esu2_nominal \text{ ((5.5), TBDY)} = 0.032$$

$$\text{From table 5A.1, TBDY: } esu2_nominal = 0.08,$$

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$$y1, sh1, ft1, fy1, \text{ are also multiplied by } \text{Min}(1, 1.25*(lb/l_d)^{2/3}), \text{ from 10.3.5, ASCE 41-17.}$$

$$\text{with } fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139$$

$$\text{with } Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00$$

$$yv = 0.00140044$$

$$shv = 0.0044814$$

$$ftv = 466.8167$$

$$fyv = 389.0139$$

$$suv = 0.00512$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lo_{u,min} = lb/ld = 0.30$$

$$suv = 0.4 * esuv_{nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: $esuv_{nominal} = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY

For calculation of $esuv_{nominal}$ and yv , shv, ftv, fyv , it is considered

characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.

$y1$, $sh1, ft1, fy1$, are also multiplied by $Min(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fsv = (fs_{jacket} * Asl_{mid,jacket} + fs_{mid} * Asl_{mid,core}) / Asl_{mid} = 389.0139$$

$$\text{with } Esv = (Es_{jacket} * Asl_{mid,jacket} + Es_{mid} * Asl_{mid,core}) / Asl_{mid} = 200000.00$$

$$1 = Asl_{ten} / (b * d) * (fs1 / fc) = 0.04740567$$

$$2 = Asl_{com} / (b * d) * (fs2 / fc) = 0.04740567$$

$$v = Asl_{mid} / (b * d) * (fsv / fc) = 0.01676222$$

and confined core properties:

$$b = 340.00$$

$$d = 677.00$$

$$d' = 13.00$$

$$fcc (5A.2, TBDY) = 34.43547$$

$$cc (5A.5, TBDY) = 0.00243499$$

$$c = \text{confinement factor} = 1.0435$$

$$1 = Asl_{ten} / (b * d) * (fs1 / fc) = 0.05824278$$

$$2 = Asl_{com} / (b * d) * (fs2 / fc) = 0.05824278$$

$$v = Asl_{mid} / (b * d) * (fsv / fc) = 0.02059413$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < vs, y2$ - LHS eq.(4.5) is satisfied

--->

$$su (4.9) = 0.15682678$$

$$Mu = MRc (4.14) = 3.7261E+008$$

$$u = su (4.1) = 8.5888248E-006$$

Calculation of ratio lb/ld

Inadequate Lap Length with $lb/ld = 0.30$

Calculation of $Mu2+$

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$$u = 8.5888248E-006$$

$$Mu = 3.7261E+008$$

with full section properties:

$$b = 400.00$$

$$d = 707.00$$

$$d' = 43.00$$

$$v = 0.00118049$$

$$N = 11016.808$$

$$fc = 33.00$$

$$co (5A.5, TBDY) = 0.002$$

$$\text{Final value of } cu: cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } cu = 0.01006403$$

$$we ((5.4c), TBDY) = ase * sh_{min} * fy_{we} / f_{ce} + \text{Min}(fx, fy) = 0.02692906$$

where $f = af * pf * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$fx = 0.02504965$$

af = 0.38744444
b = 750.00
h = 400.00
From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.00266667$
bw = 750.00
effective stress from (A.35), $ff,e = 800.0873$

fy = 0.04491794
af = 0.38744444
b = 400.00
h = 750.00
From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.005$
bw = 400.00
effective stress from (A.35), $ff,e = 765.1636$

R = 40.00
Effective FRP thickness, $tf = NL*t*Cos(b1) = 1.00$
fu,f = 840.00
Ef = 82000.00
u,f = 0.015
ase ((5.4d), TBDY) = $(ase1*Aext+ase2*Aint)/Asec = 0.12601038$
ase1 = 0.12601038
bo_1 = 690.00
ho_1 = 340.00
bi2_1 = 1.1834E+006
ase2 = Max(ase1,ase2) = 0.12601038
bo_2 = 542.00
ho_2 = 192.00
bi2_2 = 661256.00

psh,min*Fywe = Min(psh,x*Fywe , psh,y*Fywe) = 1.64062
Expression ((5.4d), TBDY) for psh,min*Fywe has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh_x*Fywe = psh1*Fywe1+ps2*Fywe2 = 1.64062
ps1 (external) = $(Ash1*h1/s1)/Asec = 0.0020944$
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 400.00
ps2 (internal) = $(Ash2*h2/s2)/Asec = 0.00026808$
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 200.00

psh_y*Fywe = psh1*Fywe1+ps2*Fywe2 = 3.23907
ps1 (external) = $(Ash1*h1/s1)/Asec = 0.00392699$
Ash1 = Astir_1*ns_1 = 157.0796
No stirrups, ns_1 = 2.00
h1 = 750.00
ps2 (internal) = $(Ash2*h2/s2)/Asec = 0.00073723$
Ash2 = Astir_2*ns_2 = 100.531
No stirrups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00
s1 = 100.00
s2 = 250.00
fywe1 = 694.45
fywe2 = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499
c = confinement factor = 1.0435

y1 = 0.00140044
sh1 = 0.0044814
ft1 = 466.8167
fy1 = 389.0139
su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044

sh2 = 0.0044814

ft2 = 466.8167

fy2 = 389.0139

su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044

shv = 0.0044814

ftv = 466.8167

fyv = 389.0139

suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

lo/lou,min = lb/ld = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = (fs,jacket*Asl,mid,jacket + fs,mid*Asl,mid,core)/Asl,mid = 389.0139

with Esv = (Es,jacket*Asl,mid,jacket + Es,mid*Asl,mid,core)/Asl,mid = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.04740567

2 = Asl,com/(b*d)*(fs2/fc) = 0.04740567

v = Asl,mid/(b*d)*(fsv/fc) = 0.01676222

and confined core properties:

b = 340.00

d = 677.00

d' = 13.00

fcc (5A.2, TBDY) = 34.43547

cc (5A.5, TBDY) = 0.00243499

c = confinement factor = 1.0435

1 = Asl,ten/(b*d)*(fs1/fc) = 0.05824278

2 = Asl,com/(b*d)*(fs2/fc) = 0.05824278

v = Asl,mid/(b*d)*(fsv/fc) = 0.02059413

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

v < vs,y2 - LHS eq.(4.5) is satisfied

--->

su (4.9) = 0.15682678

Mu = MRc (4.14) = 3.7261E+008

u = su (4.1) = 8.5888248E-006

Calculation of ratio lb/ld

Inadequate Lap Length with $l_b/l_d = 0.30$

Calculation of μ_2 -

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:

$$\mu = 8.5888248E-006$$

$$\mu_2 = 3.7261E+008$$

with full section properties:

$$b = 400.00$$

$$d = 707.00$$

$$d' = 43.00$$

$$v = 0.00118049$$

$$N = 11016.808$$

$$f_c = 33.00$$

$$c_o \text{ (5A.5, TBDY)} = 0.002$$

$$\text{Final value of } \mu_2: \mu_2^* = \text{shear_factor} * \text{Max}(\mu_2, \mu_c) = 0.01006403$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \mu_2 = 0.01006403$$

$$\mu_{we} \text{ ((5.4c), TBDY)} = a_{se} * \text{sh}_{,min} * f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.02692906$$

where $f = a_f * p_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.02504965$$

$$a_f = 0.38744444$$

$$b = 750.00$$

$$h = 400.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.00266667$$

$$b_w = 750.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 800.0873$$

$$f_y = 0.04491794$$

$$a_f = 0.38744444$$

$$b = 400.00$$

$$h = 750.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/b_w = 0.005$$

$$b_w = 400.00$$

$$\text{effective stress from (A.35), } f_{f,e} = 765.1636$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L * t * \text{Cos}(b_1) = 1.00$$

$$f_{u,f} = 840.00$$

$$E_f = 82000.00$$

$$u_{,f} = 0.015$$

$$a_{se} \text{ ((5.4d), TBDY)} = (a_{se1} * A_{ext} + a_{se2} * A_{int}) / A_{sec} = 0.12601038$$

$$a_{se1} = 0.12601038$$

$$b_{o,1} = 690.00$$

$$h_{o,1} = 340.00$$

$$b_{i,1} = 1.1834E+006$$

$$a_{se2} = \text{Max}(a_{se1}, a_{se2}) = 0.12601038$$

$$b_{o,2} = 542.00$$

$$h_{o,2} = 192.00$$

$$b_{i,2} = 661256.00$$

$$p_{sh,min} * F_{ywe} = \text{Min}(p_{sh,x} * F_{ywe}, p_{sh,y} * F_{ywe}) = 1.64062$$

Expression ((5.4d), TBDY) for $p_{sh,min} * F_{ywe}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$p_{sh,x} * F_{ywe} = p_{sh1} * F_{ywe1} + p_{sh2} * F_{ywe2} = 1.64062$$

$$p_{s1} \text{ (external)} = (A_{sh1} * h_1 / s_1) / A_{sec} = 0.0020944$$

$$A_{sh1} = A_{stir,1} * n_{s,1} = 157.0796$$

$$\text{No stirrups, } n_{s,1} = 2.00$$

$$h_1 = 400.00$$

$$p_{s2} \text{ (internal)} = (A_{sh2} * h_2 / s_2) / A_{sec} = 0.00026808$$

Ash2 = Astir_2*ns_2 = 100.531
No stirups, ns_2 = 2.00
h2 = 200.00

psh_y*Fywe = psh1*Fywe1+ps2*Fywe2 = 3.23907
ps1 (external) = (Ash1*h1/s1)/Asec = 0.00392699
Ash1 = Astir_1*ns_1 = 157.0796
No stirups, ns_1 = 2.00
h1 = 750.00
ps2 (internal) = (Ash2*h2/s2)/Asec = 0.00073723
Ash2 = Astir_2*ns_2 = 100.531
No stirups, ns_2 = 2.00
h2 = 550.00

Asec = 300000.00

s1 = 100.00

s2 = 250.00

fywe1 = 694.45

fywe2 = 694.45

fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00243499

c = confinement factor = 1.0435

y1 = 0.00140044

sh1 = 0.0044814

ft1 = 466.8167

fy1 = 389.0139

su1 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb = 0.30

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = (fs,jacket*Asl,ten,jacket + fs,core*Asl,ten,core)/Asl,ten = 389.0139

with Es1 = (Es,jacket*Asl,ten,jacket + Es,core*Asl,ten,core)/Asl,ten = 200000.00

y2 = 0.00140044

sh2 = 0.0044814

ft2 = 466.8167

fy2 = 389.0139

su2 = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 0.30

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (\text{lb}/\text{ld})^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = (fs,jacket*Asl,com,jacket + fs,core*Asl,com,core)/Asl,com = 389.0139

with Es2 = (Es,jacket*Asl,com,jacket + Es,core*Asl,com,core)/Asl,com = 200000.00

yv = 0.00140044

shv = 0.0044814

ftv = 466.8167

fyv = 389.0139

suv = 0.00512

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb = 0.30

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered

characteristic value $f_{sv} = f_{sv}/1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $f_{sv} = (f_{s,jacket} \cdot A_{s,mid,jacket} + f_{s,mid} \cdot A_{s,mid,core}) / A_{s,mid} = 389.0139$

with $E_{sv} = (E_{s,jacket} \cdot A_{s,mid,jacket} + E_{s,mid} \cdot A_{s,mid,core}) / A_{s,mid} = 200000.00$

1 = $A_{s,ten} / (b \cdot d) \cdot (f_{s1} / f_c) = 0.04740567$

2 = $A_{s,com} / (b \cdot d) \cdot (f_{s2} / f_c) = 0.04740567$

$v = A_{s,mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.01676222$

and confined core properties:

$b = 340.00$

$d = 677.00$

$d' = 13.00$

$f_{cc} (5A.2, TBDY) = 34.43547$

$cc (5A.5, TBDY) = 0.00243499$

$c = \text{confinement factor} = 1.0435$

1 = $A_{s,ten} / (b \cdot d) \cdot (f_{s1} / f_c) = 0.05824278$

2 = $A_{s,com} / (b \cdot d) \cdot (f_{s2} / f_c) = 0.05824278$

$v = A_{s,mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.02059413$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

---->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

---->

$\mu (4.9) = 0.15682678$

$\mu_u = M_{Rc} (4.14) = 3.7261E+008$

$u = \mu (4.1) = 8.5888248E-006$

Calculation of ratio lb/ld

Inadequate Lap Length with $lb/ld = 0.30$

Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 1.2627E+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 1.2627E+006$

$V_{r1} = V_{Col} ((10.3), ASCE 41-17) = k_{nl} \cdot V_{Col0}$

$V_{Col0} = 1.2627E+006$

$k_{nl} = 1$ (zero step-static loading)

NOTE: In expression (10-3) ' $V_s = A_v \cdot f_y \cdot d / s$ ' is replaced by ' $V_s + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: $f'_c = (f'_{c,jacket} \cdot Area_{jacket} + f'_{c,core} \cdot Area_{core}) / Area_{section} = 33.00$, but $f_c^{0.5} \leq 8.3$
MPa (22.5.3.1, ACI 318-14)

$M/Vd = 2.00$

$\mu_u = 2.3795529E-012$

$V_u = 2.9582284E-031$

$d = 0.8 \cdot h = 600.00$

$N_u = 11016.808$

$A_g = 300000.00$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} = 621900.694$

where:

$V_{s1} = 523602.964$ is calculated for jacket, with:

$d = 600.00$

$A_v = 157079.633$

$f_y = 555.56$

$s = 100.00$

V_{s1} is multiplied by $Col1 = 1.00$

$s/d = 0.16666667$

$V_{s2} = 98297.73$ is calculated for core, with:

$d = 440.00$

$A_v = 100530.965$

$f_y = 555.56$

$s = 250.00$

Vs2 is multiplied by Col2 = 1.00

$$s/d = 0.56818182$$

$$Vf((11-3)-(11.4), ACI 440) = 463792.00$$

f = 0.95, for fully-wrapped sections

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) $\sin \theta + \cos \theta$ is replaced with $(\cot \theta + \csc \theta) \sin \alpha$ which is more a generalised expression, where θ is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $Vf(\theta, \alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$$Vf = \text{Min}(|Vf(45, \theta)|, |Vf(-45, \theta)|), \text{ with:}$$

total thickness per orientation, $tf1 = NL * t / \text{NoDir} = 1.00$

$$dfv = d \text{ (figure 11.2, ACI 440)} = 707.00$$

$$ffe((11-5), ACI 440) = 328.00$$

$$Ef = 82000.00$$

$$fe = 0.004, \text{ from (11.6a), ACI 440}$$

$$\text{with } fu = 0.009$$

$$\text{From (11-11), ACI 440: } Vs + Vf \leq 915872.391$$

$$bw = 400.00$$

Calculation of Shear Strength at edge 2, $Vr2 = 1.2627E+006$

$$Vr2 = VCol((10.3), ASCE 41-17) = knl * VCol0$$

$$VCol0 = 1.2627E+006$$

knl = 1 (zero step-static loading)

NOTE: In expression (10-3) ' $Vs = Av * fy * d / s$ ' is replaced by ' $Vs + f * Vf$ ' where Vf is the contribution of FRPs (11.3), ACI 440).

= 1 (normal-weight concrete)

Mean concrete strength: $fc' = (fc'_{\text{jacket}} * \text{Area}_{\text{jacket}} + fc'_{\text{core}} * \text{Area}_{\text{core}}) / \text{Area}_{\text{section}} = 33.00$, but $fc'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$$M/Vd = 2.00$$

$$Mu = 2.3795529E-012$$

$$Vu = 2.9582284E-031$$

$$d = 0.8 * h = 600.00$$

$$Nu = 11016.808$$

$$Ag = 300000.00$$

$$\text{From (11.5.4.8), ACI 318-14: } Vs = Vs1 + Vs2 = 621900.694$$

where:

Vs1 = 523602.964 is calculated for jacket, with:

$$d = 600.00$$

$$Av = 157079.633$$

$$fy = 555.56$$

$$s = 100.00$$

Vs1 is multiplied by Col1 = 1.00

$$s/d = 0.16666667$$

Vs2 = 98297.73 is calculated for core, with:

$$d = 440.00$$

$$Av = 100530.965$$

$$fy = 555.56$$

$$s = 250.00$$

Vs2 is multiplied by Col2 = 1.00

$$s/d = 0.56818182$$

$$Vf((11-3)-(11.4), ACI 440) = 463792.00$$

f = 0.95, for fully-wrapped sections

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) $\sin \theta + \cos \theta$ is replaced with $(\cot \theta + \csc \theta) \sin \alpha$ which is more a generalised expression, where θ is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $Vf(\theta, \alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$$Vf = \text{Min}(|Vf(45, \theta)|, |Vf(-45, \theta)|), \text{ with:}$$

total thickness per orientation, $tf1 = NL * t / \text{NoDir} = 1.00$

$$dfv = d \text{ (figure 11.2, ACI 440)} = 707.00$$

$$ffe((11-5), ACI 440) = 328.00$$

$$Ef = 82000.00$$

fe = 0.004, from (11.6a), ACI 440
with fu = 0.009
From (11-11), ACI 440: Vs + Vf <= 915872.391
bw = 400.00

End Of Calculation of Shear Capacity ratio for element: column JC1 of floor 1
At local axis: 2

Start Of Calculation of Chord Rotation Capacity for element: column JC1 of floor 1
At local axis: 3
Integration Section: (b)
Section Type: rcjrs

Constant Properties

Knowledge Factor, $\gamma = 0.90$
Chord Rotation is generally considered as Deformation-Controlled Action according to Table C7-1, ASCE 41-17.
Mean strengths are used for Deformation-Controlled Actions according to 7.5.1.3, ASCE 41-17
Consequently:
Jacket
New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$
New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$
Concrete Elasticity, $E_c = 26999.444$
Steel Elasticity, $E_s = 200000.00$
Existing Column
New material of Primary Member: Concrete Strength, $f_c = f_{cm} = 33.00$
New material of Primary Member: Steel Strength, $f_s = f_{sm} = 555.56$
Concrete Elasticity, $E_c = 26999.444$
Steel Elasticity, $E_s = 200000.00$
External Height, $H = 400.00$
External Width, $W = 750.00$
Internal Height, $H = 200.00$
Internal Width, $W = 550.00$
Cover Thickness, $c = 25.00$
Element Length, $L = 3000.00$
Primary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Inadequate Lap Length with $l_b/l_d = 0.30$
FRP Wrapping Data
Type: Carbon
Dry properties (design values)
Thickness, $t = 1.00$
Tensile Strength, $f_{fu} = 840.00$
Tensile Modulus, $E_f = 82000.00$
Elongation, $e_{fu} = 0.009$
Number of directions, $NoDir = 1$
Fiber orientations, $b_i: 0.00^\circ$
Number of layers, $NL = 1$
Radius of rounding corners, $R = 40.00$

Stepwise Properties

Bending Moment, $M = 1.6361E+006$
Shear Force, $V_2 = 3112.69$
Shear Force, $V_3 = 2.2719624E-013$
Axial Force, $F = -15947.314$
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: $As_t = 0.00$
-Compression: $As_c = 2676.637$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $Asl_{ten} = 1137.257$

-Compression: $Asl_{com} = 1137.257$

-Middle: $Asl_{mid} = 402.1239$

Longitudinal External Reinforcement Area Distribution (in 3 divisions)

-Tension: $Asl_{ten,jacket} = 829.3805$

-Compression: $Asl_{com,jacket} = 829.3805$

-Middle: $Asl_{mid,jacket} = 402.1239$

Longitudinal Internal Reinforcement Area Distribution (in 3 divisions)

-Tension: $Asl_{ten,core} = 307.8761$

-Compression: $Asl_{com,core} = 307.8761$

-Middle: $Asl_{mid,core} = 0.00$

Mean Diameter of Tension Reinforcement, $DbL = 16.80$

New component: From table 7-7, ASCE 41_17: Final chord rotation Capacity $u_R = 1.0^*$ $u = 0.0304039$

$u = y + p = 0.0304039$

- Calculation of y -

$y = (My * Ls / 3) / E_{eff} = 0.0004039$ ((4.29), Biskinis Phd))

$My = 2.6257E+008$

$Ls = M/V$ (with $Ls > 0.1 * L$ and $Ls < 2 * L$) = 525.6338

From table 10.5, ASCE 41_17: $E_{eff} = factor * E_c * I_g = 1.1390E+014$

factor = 0.30

$Ag = 300000.00$

Mean concrete strength: $fc' = (fc'_{jacket} * Area_{jacket} + fc'_{core} * Area_{core}) / Area_{section} = 33.00$

$N = 15947.314$

$E_c * I_g = E_{c,jacket} * I_{g,jacket} + E_{c,core} * I_{g,core} = 3.7968E+014$

Calculation of Yielding Moment My

Calculation of y and My according to Annex 7 -

$y = \text{Min}(y_{ten}, y_{com})$

$y_{ten} = 2.8044129E-006$

with ((10.1), ASCE 41-17) $f_y = \text{Min}(f_y, 1.25 * f_y * (I_b / I_d)^{2/3}) = 311.2112$

$d = 707.00$

$y = 0.21519134$

$A = 0.00964597$

$B = 0.00520141$

with $pt = 0.00402142$

$pc = 0.00402142$

$pv = 0.00142194$

$N = 15947.314$

$b = 400.00$

" = 0.06082037

$y_{comp} = 1.4823586E-005$

with fc^* (12.3, (ACI 440)) = 33.35516

$fc = 33.00$

$fl = 0.77176471$

$b = 400.00$

$h = 750.00$

$Ag = 300000.00$

From (12.9), ACI 440: $ka = 0.14679369$

$g = pt + pc + pv = 0.00946477$

$rc = 40.00$

$Ae / Ac = 0.51607158$

Effective FRP thickness, $tf = NL * t * \text{Cos}(b1) = 1.00$

effective strain from (12.5) and (12.12), $efe = 0.004$

$fu = 0.009$

$Ef = 82000.00$

Ec = 26999.444
y = 0.21218177
A = 0.00933798
B = 0.00502021
with Es = 200000.00

Calculation of ratio lb/l_d

Inadequate Lap Length with lb/l_d = 0.30

- Calculation of p -

From table 10-8: p = 0.03

with:

- Columns controlled by inadequate development or splicing along the clear height because lb/l_d < 1

shear control ratio $V_y E / V_{Col} O E = 0.19671905$

d = d_{external} = 707.00

s = s_{external} = 0.00

t = s₁ + s₂ + 2*tf/bw*(ffe/fs) = 0.00761619

jacket: s₁ = Av₁*h₁/(s₁*Ag) = 0.00392699

Av₁ = 157.0796, is the total area of all stirrups parallel to loading (shear) direction

h₁ = 750.00

s₁ = 100.00

core: s₂ = Av₂*h₂/(s₂*Ag) = 0.00073723

Av₂ = 100.531, is the total area of all stirrups parallel to loading (shear) direction

h₂ = 550.00

s₂ = 250.00

The term 2*tf/bw*(ffe/fs) is implemented to account for FRP contribution

where f = 2*tf/bw is FRP ratio (EC8 - 3, A.4.4.3(6)) and ffe/fs normalises f to steel strength

All these variables have already been given in Shear control ratio calculation.

For the normalisation fs of jacket is used.

NUD = 15947.314

Ag = 300000.00

f_{cE} = (f_{c_jacket}*Area_{jacket}+ f_{c_core}*Area_{core})/section_area = 33.00

f_{yIE} = (f_{y_ext_Long_Reinf}*Area_{ext_Long_Reinf}+ f_{y_int_Long_Reinf}*Area_{int_Long_Reinf})/Area_{Tot_Long_Rein} =

555.56

f_{yIE} = (f_{y_ext_Trans_Reinf}* s₁+ f_{y_int_Trans_Reinf}* s₂)/(s₁+ s₂) = 555.56

pl = Area_{Tot_Long_Rein}/(b*d) = 0.00946477

b = 400.00

d = 707.00

f_{cE} = 33.00

End Of Calculation of Chord Rotation Capacity for element: column JC1 of floor 1

At local axis: 3

Integration Section: (b)